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STAMP:

PROJECT NAME:

Snow College Library

**150 College Avenue
Ephraim, Utah 84627**

REVISIONS:

100% CD, September 15, 2008

100% CD Review, August 4, 2008

ISSUE DATE:

SEPTEMBER 15 2008, 100% CD

ARCHITECT'S PROJECT NUMBER:

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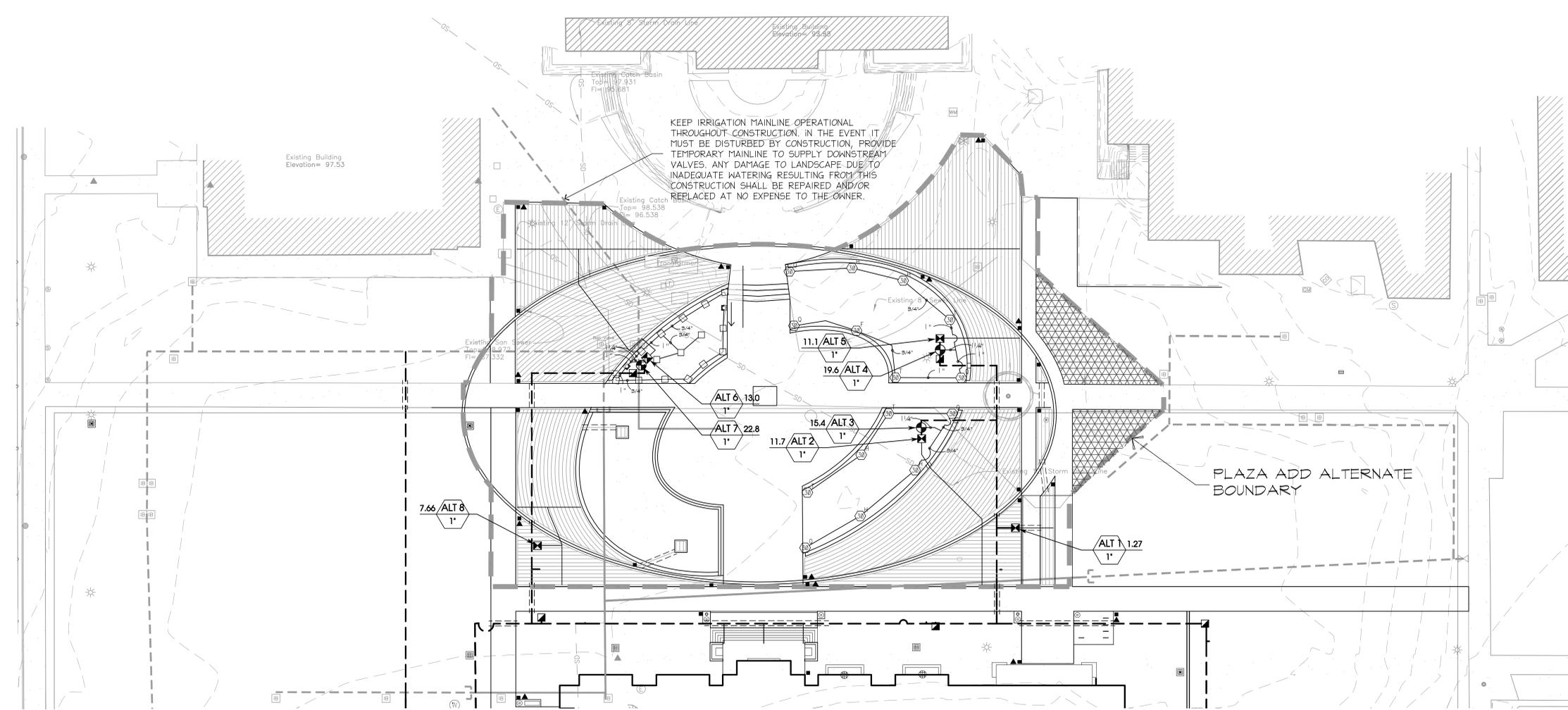
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SHEET TITLE:

**PLAZA ADD ALTERNATE
IRRIGATION PLAN**

SHEET NUMBER:

LR-102



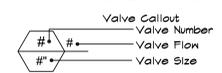
KEEP IRRIGATION MAINLINE OPERATIONAL THROUGHOUT CONSTRUCTION. IN THE EVENT IT MUST BE DISTURBED BY CONSTRUCTION, PROVIDE TEMPORARY MAINLINE TO SUPPLY DOWNSTREAM VALVES. ANY DAMAGE TO LANDSCAPE DUE TO INADEQUATE WATERING RESULTING FROM THIS CONSTRUCTION SHALL BE REPAIRED AND/OR REPLACED AT NO EXPENSE TO THE OWNER.

PLAZA ADD ALTERNATE BOUNDARY

IRRIGATION LEGEND

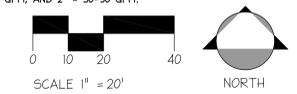
SYMBOL	MANUFACTURER	MODEL NUMBER	DESCRIPTION	RAD.	P.S.I.	G.P.M.	DETAIL
[Symbol]	RAINBIRD	1804-PRS-10H	POP-UP SPRAY HEAD	9"	30	.79	SHT. LR-501 DTL. J
[Symbol]	RAINBIRD	1804-PRS-12-Q,T,H,F	POP-UP SPRAY HEAD	10"	30	.65, .87, 1.30, 2.60	SHT. LR-501 DTL. J
[Symbol]	RAINBIRD	5004 PL-NPR-30-Q,T,H,F	POP-UP ROTOR HEAD	27"	45	1.40, 1.85, 2.96, 5.78	SHT. LR-501 DTL. G
[Symbol]	RAINBIRD	XCZ-100-B-COM	DRIP CONTROL VALVE ASSEMBLY INCLUDES PEB VALVE, FILTER, AND PRESSURE REGULATOR RESPONSIBLE TO ADJUST PRESSURE AT EACH VALVE.				SHT. LR-502 DTL. A
[Symbol]	RAINBIRD	PEB-PRS-D	ELECTRIC REMOTE CONTROL VALVE WITH PRESSURE REGULATING MODULE (SIZED AS NOTED). CONTRACTOR RESPONSIBLE TO ADJUST PRESSURE AT EACH VALVE.				SHT. LR-501 DTL. A
[Symbol]	RAINBIRD	44RC	QUICK COUPLING VALVE				SHT. LR-501 DTL. E
[Symbol]			EXISTING MAINLINE TO REMAIN				
[Symbol]			EXISTING MAINLINE TO BE ABANDONED				
[Symbol]	APPROVED	PVC SCH 40	2" POTABLE PRESSURE SUPPLY LINE				SHT. LR-501 DTL. C
[Symbol]	APPROVED	PVC SCH 40	NON-PRESSURE LATERAL LINE (SIZE NOTED ON PLANS)				SHT. LR-501 DTL. C
[Symbol]	APPROVED	PVC SCH 40	IRRIGATION SLEEVE				SHT. LR-501 DTL. I
[Symbol]	NETAFIM	TLGV4-18	TECHLINE INLINE EMITTER TUBING W/CHECK VALVE AND .4 GPH EMITTERS SPACED 18" APART.				SHT. LR-502 DTL. B,C,H
[Symbol]	NETAFIM	#TLFV-1 AND #TL50V	DRIP AUTOMATIC AND MANUAL FLUSH VALVE				SHT. LR-502 DTL. D
[Symbol]	NETAFIM	#10-CV-01	DRIP OPERATION INDICATOR STAKE				SHT. LR-502 DTL. I
[Symbol]			ADD IRRIGATION FOR NEW TURF AREA (SEE NOTE #22)				

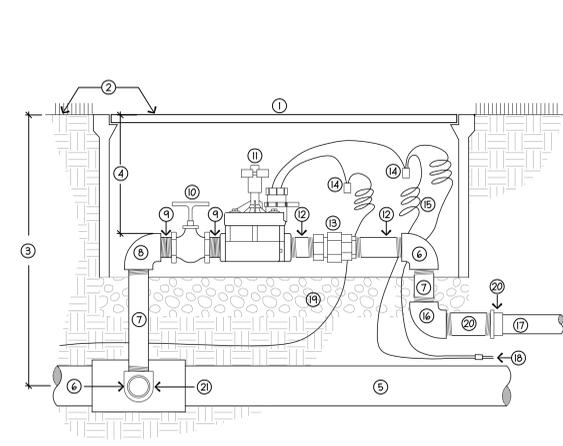
NOTE: IRRIGATION HEADS SHALL BE INSTALLED WITH 5/8" CHECK VALVES AS NECESSARY TO PREVENT LOW HEAD DRAINAGE. INSTALL SPRING CHECK VALVES AS NECESSARY ON DRIP IRRIGATION LINES TO PREVENT LOW HEAD DRAINAGE.



IRRIGATION NOTES

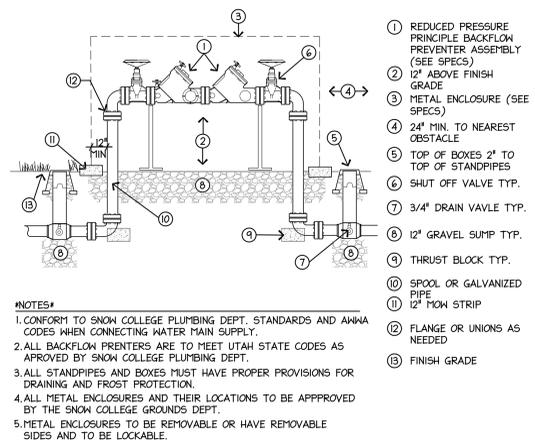
- MIKE DUNCAN IS THE SNOW COLLEGE GROUNDS AND IRRIGATION DEPARTMENT CONTACT PERSON. HE CAN BE REACHED BY TELEPHONE AT (485) 340-0268.
- EXISTING LANDSCAPE TO REMAIN SHALL BE PROTECTED, MAINTAINED, AND SHALL BE IRRIGATED THROUGHOUT THE GROWING SEASON. ANY LANDSCAPE AREAS TO REMAIN THAT WILL HAVE IRRIGATION SHUT OFF DUE TO CONSTRUCTION SHALL BE HAND WATERED. ANY LANDSCAPE TO REMAIN THAT IS DAMAGED DUE TO LACK OF IRRIGATION SHALL BE REPLACED AT NO EXPENSE TO THE OWNER. COORDINATE WITH SNOW COLLEGE IRRIGATION DEPT. TO VERIFY LOCATIONS OF EXISTING IRRIGATION AND THE LANDSCAPE AREAS THEY WATER.
- THE IRRIGATION CONTRACTOR MUST EXAMINE THE SITE CONDITIONS UNDER WHICH THE WORK IS TO BE PERFORMED AND NOTIFY THE LANDSCAPE ARCHITECT IN WRITING OF UNSATISFACTORY CONDITIONS. DO NOT PROCEED UNTIL CONDITIONS HAVE BEEN CORRECTED.
- IT IS THE RESPONSIBILITY OF THE IRRIGATION CONTRACTOR TO FAMILIARIZE HIMSELF WITH ALL GRADE DIFFERENCES, LOCATION OF WALKS, RETAINING WALLS, STRUCTURES, AND UTILITIES. THE IRRIGATION CONTRACTOR SHALL REPAIR OR REPLACE ALL ITEMS DAMAGED BY HIS WORK. HE SHALL COORDINATE HIS WORK WITH OTHER CONTRACTORS FOR THE LOCATION, INSTALLATION AND MARKING OF PIPE SLEEVES UNDER WALKS AND THROUGH WALLS.
- BEFORE ANY TRENCHING, EXCAVATION OR DIGGING FOR ANY REASON, THE IRRIGATION CONTRACTOR SHALL OBTAIN A "DIGGING PERMIT" AND HAVE THE AREA "BLUE STAKED" IN ORDER TO DETERMINE THE LOCATION OF UNDERGROUND UTILITIES. REQUESTS FOR DIGGING PERMITS MUST BE RECEIVED FIVE (5) WORKING DAYS BEFORE DIGGING BEGINS. THE CONTRACTOR WILL CONDUCT HIS WORK IN SUCH A MANNER TO PROTECT ALL UTILITIES FROM DAMAGE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO REPAIR OR REPLACE ANY DAMAGE CAUSED BY HIM OR HIS WORKMEN AT NO EXPENSE TO THE OWNER.
- THE IRRIGATION SYSTEM IS BASED ON A MINIMUM OPERATING PRESSURE AT THE MAIN CONNECTION OF 75 PSI. THE IRRIGATION CONTRACTOR SHALL VERIFY WATER PRESSURE PRIOR TO CONSTRUCTION. REPORT ANY DIFFERENCES BETWEEN THE WATER PRESSURE INDICATED ON THE DRAWINGS AND THE ACTUAL PRESSURE READING AT THE POINT OF CONNECTION TO THE LANDSCAPE ARCHITECT.
- DO NOT HILLFULLY INSTALL THE IRRIGATION SYSTEM AS SHOWN ON THE DRAWINGS WHEN IT IS OBVIOUS IN THE FIELD THAT UNKNOWN OBSTRUCTIONS, GRADE DIFFERENCES OR DIFFERENCES IN THE AREA DIMENSIONS EXIST THAT MIGHT NOT HAVE BEEN CONSIDERED IN THE ENGINEERING. SUCH OBSTRUCTIONS OR DIFFERENCES SHOULD BE BROUGHT TO THE ATTENTION OF THE LANDSCAPE ARCHITECT. IN THE EVENT THIS NOTIFICATION IS NOT PERFORMED, THE IRRIGATION CONTRACTOR SHALL ASSUME FULL RESPONSIBILITY FOR ANY REVISIONS NECESSARY.
- ALL IRRIGATION EQUIPMENT NOT OTHERWISE DETAILED SHALL BE INSTALLED AS PER MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS.
- THE IRRIGATION CONTRACTOR SHALL FLUSH AND ADJUST ALL THROTTLE CONTROLS ON ALL SPRINKLER HEADS AND ADJUST VALVES FOR OPTIMUM COVERAGE WITH MINIMAL OVER SPRAY ONTO WALKS, STREETS, WALLS, ETC.
- LOCATE ALL HEADS NEXT TO WALKS A MINIMUM OF 3" FROM EDGE OF WALKS, PAVEMENTS, CURBS, ETC. AND SET 1" BELOW EDGE OF PAVEMENT.
- ALL SPRINKLER HEADS SHALL BE SET PERPENDICULAR TO FINISH GRADE.
- IRRIGATION VALVES AND MAIN AND LATERAL LINES SHOWN ON PLANS ARE DIAGRAMMATIC ONLY AND ARE SHOWN IN HARDSCAPE AREAS FOR CLARITY ONLY. ALL VALVES AND PIPES SHALL BE PLACED IN LANDSCAPE AREAS. PIPE SHALL BE LAIN IN COMMON TRENCHES WHEN FEASIBLE.
- EXACT LOCATION OF IRRIGATION MAINLINE ROUTING SHALL BE COORDINATED WITH THE PLANTING (SEE SHEET LP101) AND PLACED TO ELIMINATE CONFLICTS WITH TREE PLANTING.
- PIPE SIZES SHALL CONFORM TO THOSE SHOWN ON THE DRAWINGS. NO SUBSTITUTIONS OR SMALLER PIPE SIZES SHALL BE APPROVED. ALL DAMAGED AND REJECTED PIPE SHALL BE REMOVED FROM THE SITE IMMEDIATELY.
- ALL MAIN LINES, LATERAL LINES, AND CONTROL WIRES UNDER PAVING SHALL BE INSTALLED IN SEPARATE SLEEVES. CONTROL WIRE SLEEVES SHALL BE OF SUFFICIENT SIZE FOR THE REQUIRED NUMBER OF WIRES UNDER PAVING.
- ALL SALVAGED IRRIGATION EQUIPMENT (HEADS, VALVES, ETC.) WITH THE EXCEPTION OF PIPE, SHALL BE DELIVERED TO SNOW COLLEGE GROUNDS DEPARTMENT.
- THE IRRIGATION CONTRACTOR SHALL MAKE THE ELECTRICAL CONNECTIONS FROM THE NEW VALVES TO THE CONTROLLERS. IRRIGATION TIRES SHALL BE SET BY THE CONTRACTOR UNDER THE DIRECTION OF SNOW COLLEGE GROUNDS MAINTENANCE SUPERVISOR.
- EXISTING TREES ON THE SITE MAY REQUIRE SLIGHT MODIFICATIONS TO THE LINES AS THEY ARE SHOWN ON PLANS. CONTRACTOR TO HAND TRENCH UNDER EXISTING TREE TRUNKS.
- THE CONTRACTOR SHALL PROVIDE ALL LABOR, PARTS AND MATERIALS REQUIRED TO COMPLETE THE IRRIGATION SYSTEM.
- AREAS SHOWN TO BE REPAIRED AND ADJUSTED ARE ESTIMATED. CONTRACTOR SHALL REPAIR AND ADJUST IRRIGATION IN ALL AREAS THAT HAVE BEEN DISTURBED BY THIS CONSTRUCTION.
- ALL LINES SHALL SLOPE TO MANUAL DRAIN (SEE DETAILS). IF FIELD CONDITIONS NECESSITATE ADDITIONAL DRAINS, THESE DRAINS SHALL BE INSTALLED FOR COMPLETE DRAINAGE OF THE ENTIRE SYSTEM. REFER TO DETAILS AND SPECIFICATIONS FOR DRAIN INSTALLATION, SUMP REQUIREMENTS AND DEPTH OF BOTH DRAIN AND SUMP.
- CONNECT NEW IRRIGATION TO ADJACENT IRRIGATION ZONE(S) IF PIPE SIZING IS ADEQUATE TO HANDLE THE ADDITIONAL FLOW DEMAND. ALL IRRIGATION HEADS AND EQUIPMENT SHALL MATCH THAT OF THE EXISTING ZONE. IF PIPE SIZING IS NOT ADEQUATE, UPSIZE PIPING AS NECESSARY OR INSTALL NEW IRRIGATION ZONE. FLOW DEMAND FOR LATERAL PIPE SHALL NOT EXCEED THE FOLLOWING PARAMETERS: 3/4" = 8 GPM OR LESS; 1" = 8-12GPM; 1 1/4" = 12-22 GPM; 1 1/2" = 22-30 GPM; AND 2" = 30-50 GPM.





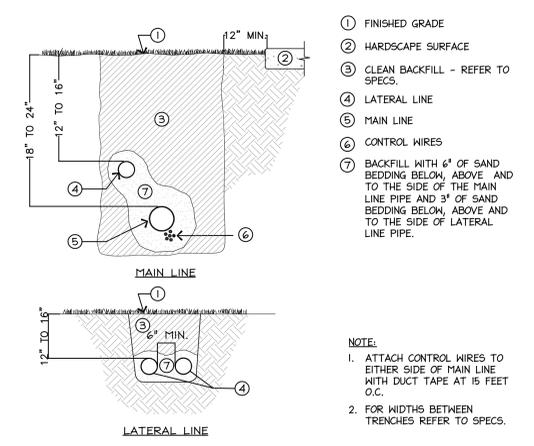
- 1 STANDARD OR JUMBO VALVE BOX
 - 2 INSTALL BOX AT FINISH GRADE IN TURF AREAS AND LEVEL WITH TOP OF BARK MULCH IN PLANTING AREAS
 - 3 18"-24" MAX. DEPTH
 - 4 MAX. DEPTH 12"
 - 5 IRRIGATION MAIN LINE, SIZE AS PER PLANS
 - 6 6" MIN. DEPTH- 1/2" WASHED AGGREGATE
 - 7 THREADED NIPPLE WITH BUSHING
 - 8 PVC SCH. 80 T X T ELBOW SAME SIZE AS VALVE
 - 9 SCH. 80 NIPPLE ON EITHER SIDE OF BALL VALVE
 - 10 BRASS BALL VALVE (LINE SIZE). INSTALL ONE PER VALVE CLUSTER. MAX. 3 CONTROL VALVES PER CLUSTER.
 - 11 ELECTRIC REMOTE CONTROL VALVE - SEE IRRIGATION LEGEND
 - 12 SCH. 80 NIPPLE ON EITHER SIDE OF UNION
 - 13 PVC SCH. 80 UNION
 - 14 WATER TIGHT CONNECTORS (3M DBY ONLY)
 - 15 PROVIDE 24" EXPANSION LOOP AT EACH WIRE CONNECTOR IN BOX.
 - 16 SCH. 80 ELBOW WITH SCHED. 80 NIPPLE INTO T X T BUSHING TO LATERAL
 - 17 IRRIGATION LATERAL LINE, SIZE AS PER PLANS
 - 18 CONTROL WIRES
 - 19 6" MIN. DEPTH- 1/2" WASHED AGGREGATE
 - 20 THREADED NIPPLE WITH BUSHING
 - 21 PVC SCH. 80 TEE 5/8x5/8 WITH SCH. 80 5/8T BUSHING.
- NOTES:
 1. ONE REMOTE CONTROL VALVE PER STANDARD SIZE VALVE BOX.
 2. TWO REMOTE CONTROL VALVES PER JUMBO SIZE VALVE BOX.
 3. ALL FITTINGS AND NIPPLES IN MANIFOLD SHALL BE SCH. 80 THREADED PVC USING TEFLON TAPE PASTE IF LARGER THAN 1".
 4. VALVE MANIFOLD TO BE INSTALLED GOING AWAY FROM MAIN LINE. MINIMUM OF 1 FOOT BEFORE CHANGE IN DIRECTION.

A ELECTRIC REMOTE-CONTROL VALVE ASSEMBLY
SCALE: NTS SECTION



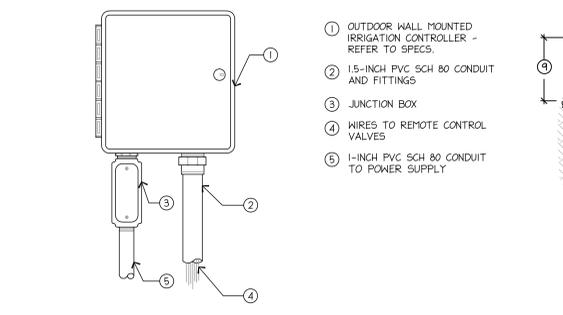
- NOTES:
 1. CONFORM TO SNOW COLLEGE PLUMBING DEPT. STANDARDS AND ANNA CODES WHEN CONNECTING WATER MAIN SUPPLY.
 2. ALL BACKFLOW PREVENTERS ARE TO MEET UTAH STATE CODES AS APPROVED BY SNOW COLLEGE PLUMBING DEPT.
 3. ALL STANDPIPES AND BOXES MUST HAVE PROPER PROVISIONS FOR DRAINING AND FROST PROTECTION.
 4. ALL METAL ENCLOSURES AND THEIR LOCATIONS TO BE APPROVED BY THE SNOW COLLEGE GROUNDS DEPT.
 5. METAL ENCLOSURES TO BE REMOVABLE OR HAVE REMOVABLE SIDES AND TO BE LOCKABLE.
- 1 REDUCED PRESSURE PRINCIPLE BACKFLOW PREVENTER ASSEMBLY (SEE SPECS)
 - 2 12" ABOVE FINISH GRADE
 - 3 METAL ENCLOSURE (SEE SPECS)
 - 4 24" MIN. TO NEAREST OBSTACLE
 - 5 TOP OF BOXES 2" TO TOP OF STANDPIPES
 - 6 SHUT OFF VALVE TYP.
 - 7 3/4" DRAIN VALVE TYP.
 - 8 12" GRAVEL SUMP TYP.
 - 9 THRUST BLOCK TYP.
 - 10 SPOOL OR GALVANIZED PIPE
 - 11 12" HIGH STRIP
 - 12 FLANGE OR UNIONS AS NEEDED
 - 13 FINISH GRADE

B BACKFLOW PREVENTER
SCALE: NTS SECTION

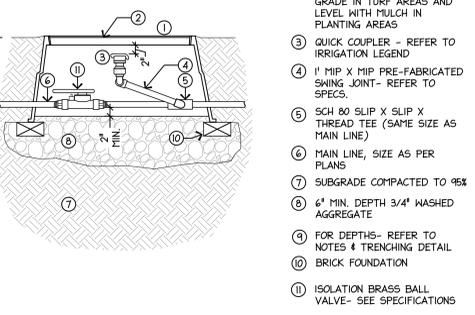


- 1 FINISHED GRADE
 - 2 HARDSCAPE SURFACE
 - 3 CLEAN BACKFILL - REFER TO SPECS.
 - 4 LATERAL LINE
 - 5 MAIN LINE
 - 6 CONTROL WIRES
 - 7 BACKFILL WITH 6" OF SAND BEDDING BELOW, ABOVE AND TO THE SIDE OF THE MAIN LINE PIPE AND 3" OF SAND BEDDING BELOW, ABOVE AND TO THE SIDE OF LATERAL LINE PIPE.
- NOTE:
 1. ATTACH CONTROL WIRES TO EITHER SIDE OF MAIN LINE WITH DUCT TAPE AT 15 FEET O.C.
 2. FOR WIDTHS BETWEEN TRENCHES REFER TO SPECS.

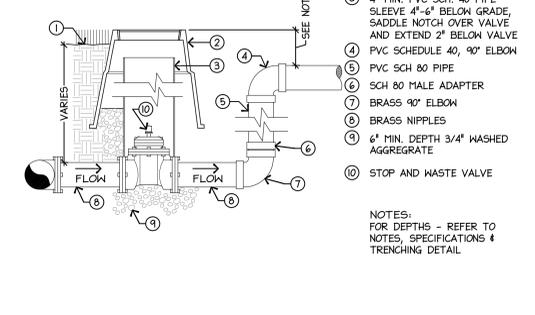
C TRENCHING
SCALE: NTS SECTION



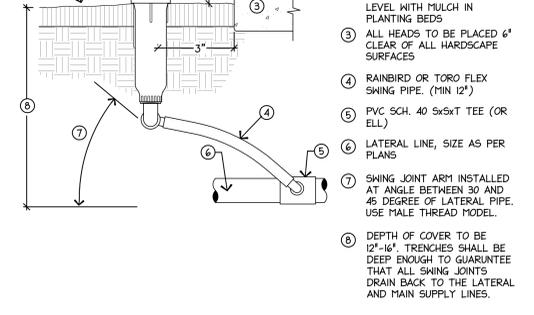
D IRRIGATION CONTROLLER-WALL MOUNTED
SCALE: NTS SECTION



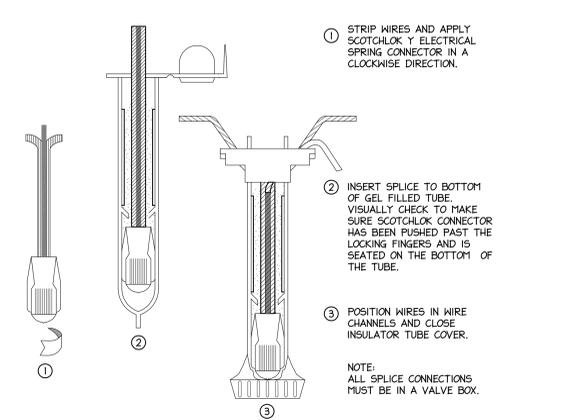
E ISOLATION/QUICK COUPLING ASSEMBLY
SCALE: NTS SECTION



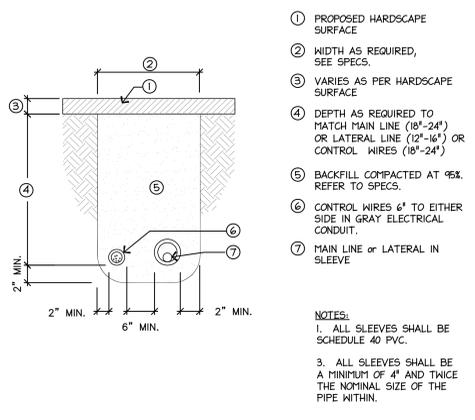
F STOP AND WASTE VALVE
SCALE: SCALE TYPE OF DETAIL SECTION



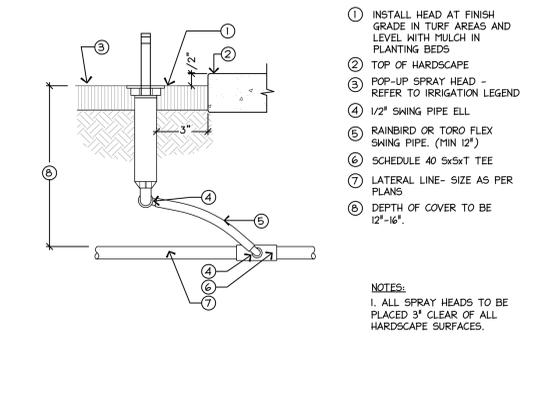
G POP-UP ROTOR
SCALE: NTS SECTION



H 3M DBY DIRECT BURY SPLICE KIT
SCALE: NTS SECTION



I SLEEVING DETAIL
SCALE: NTS SECTION



J POP-UP SPRAY HEAD
SCALE: NTS SECTION



K POP-UP SPRAY HEAD
SCALE: NTS SECTION

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SNOW
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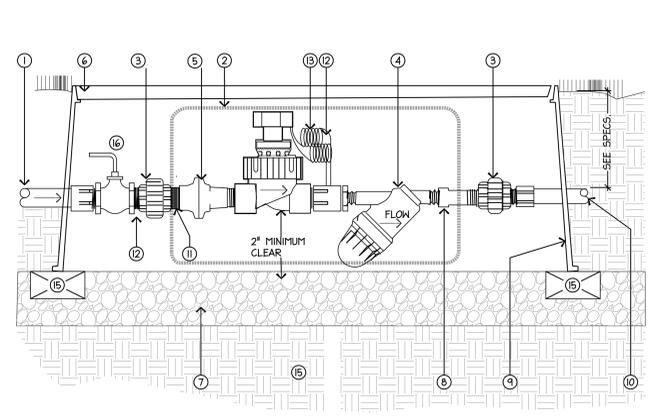
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SHEET TITLE:

IRRIGATION DETAILS

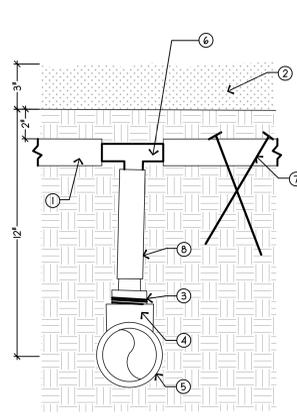
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LR-501



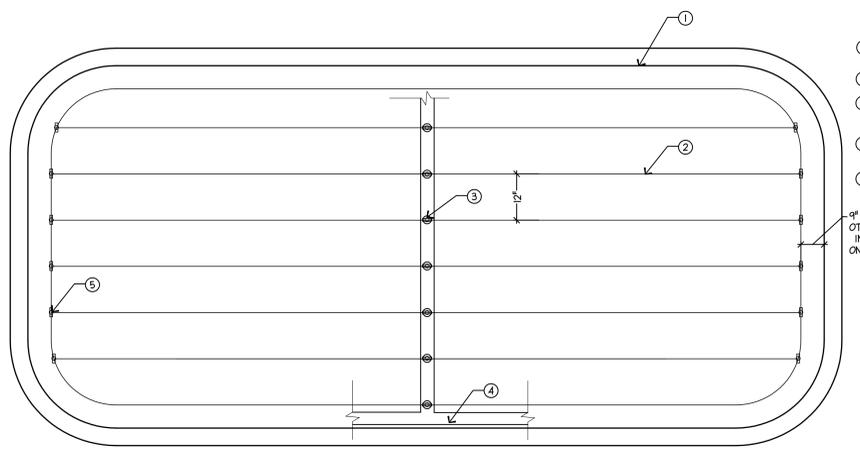
A DRIP CONTROL VALVE ASSEMBLY
SCALE: NTS SECTION

- ① IRRIGATION MAIN SUPPLY LINE, SIZE AS PER PLANS
 - ② DRIP CONTROL VALVE ASSEMBLY (CONTROL VALVE, FILTER, AND PRESSURE REGULATOR)
 - ③ SCH 80 UNION
 - ④ INLINE FILTER (SEE SPECS.)
 - ⑤ INLINE PRESSURE REGULATOR VALVE (SEE SPECS.)
 - ⑥ INSTALL BOX AT FINISH GRADE (TOP OF MULCH OR TURF)
 - ⑦ 6" MIN. DEPTH- 1/2" WASHED AGGREGATE
 - ⑧ SCH 80 M x F I.P.S. COUPLING
 - ⑨ CARSON BROOKS VALVE BOX WITH BOLT DOWN LID, SIZE AS NECESSARY TO CONTAIN VALVE, FILTER, AND PRESSURE REGULATOR IN SAME BOX.
 - ⑩ IRRIGATION LATERAL LINE, SIZE AS PER PLANS
 - ⑪ PVC SCH. 80 NIPPLE, LENGTH AS REQUIRED (TYP.)
 - ⑫ WATER TIGHT CONNECTORS (3M DBY ONLY)
 - ⑬ PROVIDE 24" EXPANSION LOOP AT EACH WIRE CONNECTOR IN BOX.
 - ⑭ 90% COMPACTED SUB-GRADE
 - ⑮ BRICK FOUNDATION OR PRESSURE TREATED WOOD FRAME
 - ⑯ BRASS BALL VALVE (LINE SIZE).
- NOTES:
1. ALL FITTINGS AND NIPPLES IN MANIFOLD SHALL BE SCH. 80 THREADED PVC USING TEFLON TAPE.
2. VALVE MANIFOLD TO BE INSTALLED GOING AWAY FROM MAIN LINE. MINIMUM OF 1 FOOT BEFORE CHANGE IN DIRECTION.
3. LOCATE ALL VALVE BOXES IN LANDSCAPE AREAS UNLESS OTHERWISE INDICATED.



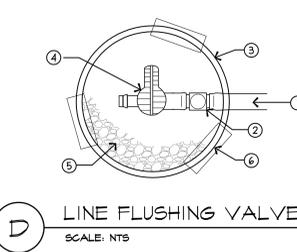
B DRIP TUBING CONNECTION
SCALE: NTS SECTION

- ① DRIP TUBING
 - ② MULCH, AS SPECIFIED
 - ③ THREADED MALE DRIP TUBING ADAPTER
 - ④ SCH 40 PVC TEE
 - ⑤ SCH 40 PVC PIPE, SIZE AS NOTED ON THE PLANS.
 - ⑥ COMPATIBLE DRIP TUBING FITTING
 - ⑦ 2 WIRE STAKES 2" O.C. MAX. ANGLED AND CROSSED ALL TUBING
 - ⑧ BLANK TUBING
- NOTES:
DRIP TUBING SHALL NOT BE PLACED ON THE SURFACE. IT SHALL BE PLACED 2" BELOW GRADE AS DETAILED. NO EXCEPTIONS.



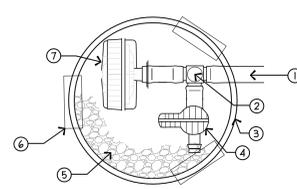
C CENTER FEED DRIP TUBING LAYOUT
SCALE: NTS PLAN

- ① EDGE OF PLANTING BED/CURB
 - ② MULCH
 - ③ DRIP TUBING CONNECTION (SEE DETAIL B, SHEET LR-502)
 - ④ PVC LATERAL IRRIGATION LINE
 - ⑤ COMPATIBLE DRIP TUBING ADAPTER
- 9" TYP. UNLESS OTHERWISE INDICATED ON PLAN
- NOTE:
NO RUN OF INLINE EMITTER TUBING SHALL EXCEED 300'



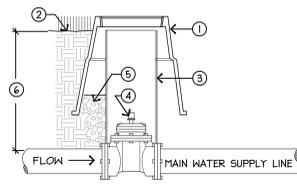
D LINE FLUSHING VALVE
SCALE: NTS PLAN

- ① BLANK TUBING TYP.
- ② TEE 12" X 18" VALVE
- ③ BOX (INSTALL PER SPECS)
- ④ SHUT-OFF VALVE (NETAFIM #TL50V)
- ⑤ 3" GRAVEL SUMP (FILLING 10" WIDE X 2" DEEP HOLE) WITH FILTER FABRIC ON TOP AND SIDES.
- ⑥ BRICK FOUNDATION OR PRESSURE TREATED WOOD FRAME



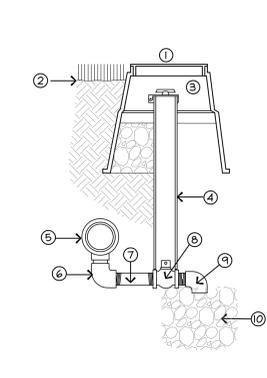
E AUTOMATIC FLUSH VALVE
SCALE: NTS PLAN

- ① BLANK TUBING TYP.
- ② TECHLINE TEE
- ③ BOX (INSTALL PER SPECS)
- ④ SHUT-OFF VALVE (NETAFIM #TL50V)
- ⑤ 3" GRAVEL SUMP (FILLING 10" WIDE X 2" DEEP HOLE) WITH FILTER FABRIC ON TOP AND SIDES.
- ⑥ BRICK FOUNDATION OR PRESSURE TREATED WOOD FRAME
- ⑦ LINE FLUSHING VALVE #LF-TLV-1



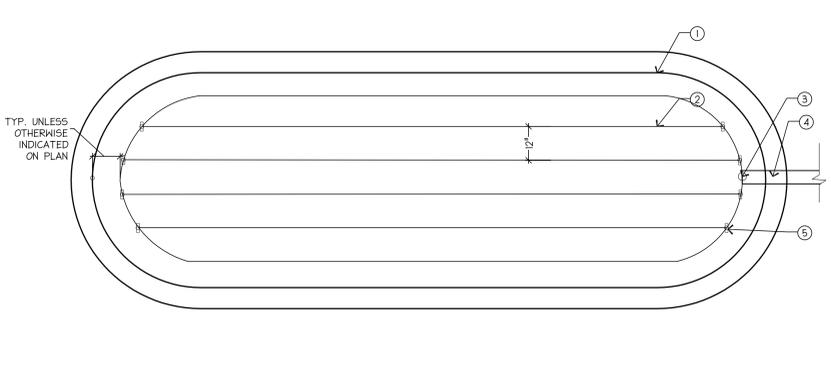
F MAINLINE ISOLATION VALVE
SCALE: NTS SECTION

- ① 10" ROUND CARSON-BROOKS 910 VALVE BOX
- ② INSTALL BOX AT FINISH GRADE IN TURF AREAS AND LEVEL WITH MULCH IN PLANTING BEDS
- ③ 6" PVC SCH. 40 PIPE SLEEVE, 4" - 6" BELOW GRADE
- ④ RESILIENT WEDGE GATE VALVE - REFER TO IRRIGATION LEGEND
- ⑤ 6" MIN. DEPTH 3/4" WASHED AGGREGATE
- ⑥ FOR DEPTHS - REFER TO NOTES, SPECS. & TRENCHING DETAIL



G MANUAL DRAIN VALVE ASSEMBLY
SCALE: NTS SECTION

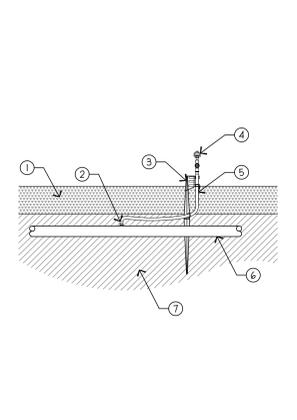
- ① 10" ROUND VALVE BOX W/ LOCK BOLT, CARSON-BROOKS 910 OR EQUAL
 - ② FINISH GRADE
 - ③ 90% WEATHERTIGHT LOCKING VALVE CAP PERMANENTLY ATTACHED TO SLEEVE. TOP OF MARKER 4" - 6" BELOW GRADE
 - ④ 2" PVC SCH. 40 SLEEVE NOTCHED OVER VALVE
 - ⑤ PVC SCH. 80 TEE 4" MAIN LINE
 - ⑥ 3/4" PVC SCH. 80 ELBOW
 - ⑦ 3/4" PVC SCH. 80 NIPPLE (LENGTH AS REQUIRED)
 - ⑧ 3/4" FORD B1133 BALL VALVE
 - ⑨ 3/4" MARLEX STREET ELBOW
 - ⑩ 1/2" WASHED AGGREGATE SUMP - 18" X 18" X 12" MIN. SIZE
 - ⑪ 6" MIN. CLEAN GRAVEL
- NOTE:
1) ALL PVC NIPPLES TO BE SCH. 80
2) PROVIDE VALVE KEY TO OWNER



H END FEED DRIP TUBING LAYOUT
SCALE: NTS PLAN

- ① EDGE OF PLANTING BED/CURB
 - ② MULCH
 - ③ DRIP TUBING CONNECTION (SEE DETAIL B, SHEET LR-502)
 - ④ PVC LATERAL IRRIGATION LINE SIZE AS PER PLAN
 - ⑤ COMPATIBLE DRIP TUBING FITTING (SEE DRIP TUBING CONNECTION DETAIL B, SHEET LR-502)
- 9" TYP. UNLESS OTHERWISE INDICATED ON PLAN
- NOTE:
END CONFIGURATIONS SHALL ONLY BE USED IN BEDS REQUIRING LESS THAN 300' OF IN-LINE EMITTER TUBING.

- ① EDGE OF PLANTING BED/CURB
 - ② MULCH AS SPECIFIED
 - ③ BARB X BARB CONNECTOR
 - ④ STAKE
 - ⑤ DRIP OPERATION INDICATOR (SEE IRRIGATION SCHEDULE)
 - ⑥ SUPER FLEX UV WHITE TUBING
 - ⑦ TECHLINE CV 17M1 TUBING
 - ⑧ PLANTING SOIL MIX
- NOTES:
LOCATIONS SHOWN ON PLAN ARE APPROXIMATE. LOCATE INDICATOR STAKE AT THE FARTHEST AND/OR HIGHEST POINT ON THE ZONE AND IN LOCATIONS APPROVED BY SNOW COLLEGE IRRIGATION MAINTENANCE REPRESENTATIVE.



I DRIP OPERATION INDICATOR STAKE
SCALE: NTS SECTION

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STAMP:

PROJECT NAME:
Snow College Library
150 College Avenue
Ephraim, Utah 84627

REVISIONS:

100% CD, September 15, 2008
100% CD Review, August 4, 2008

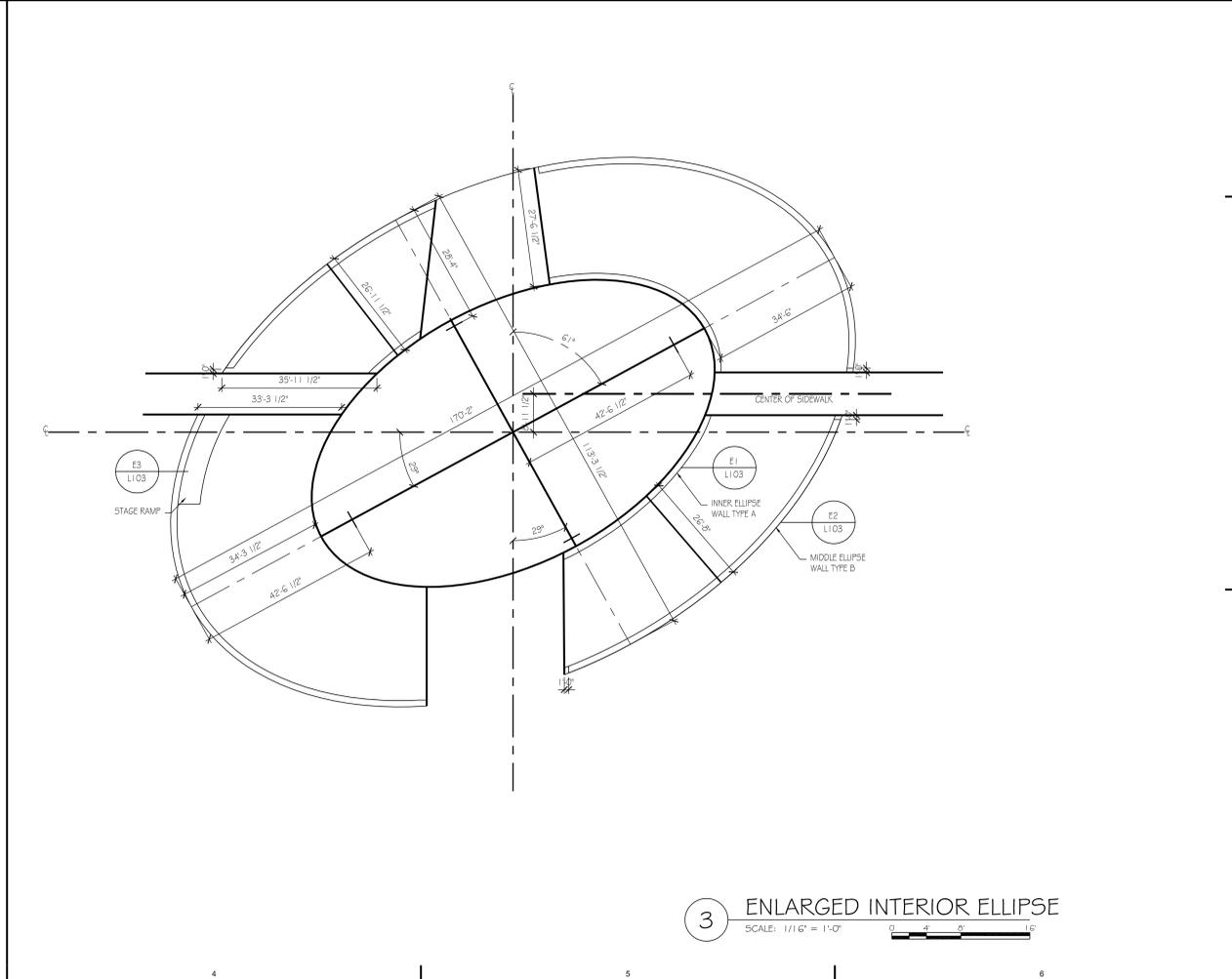
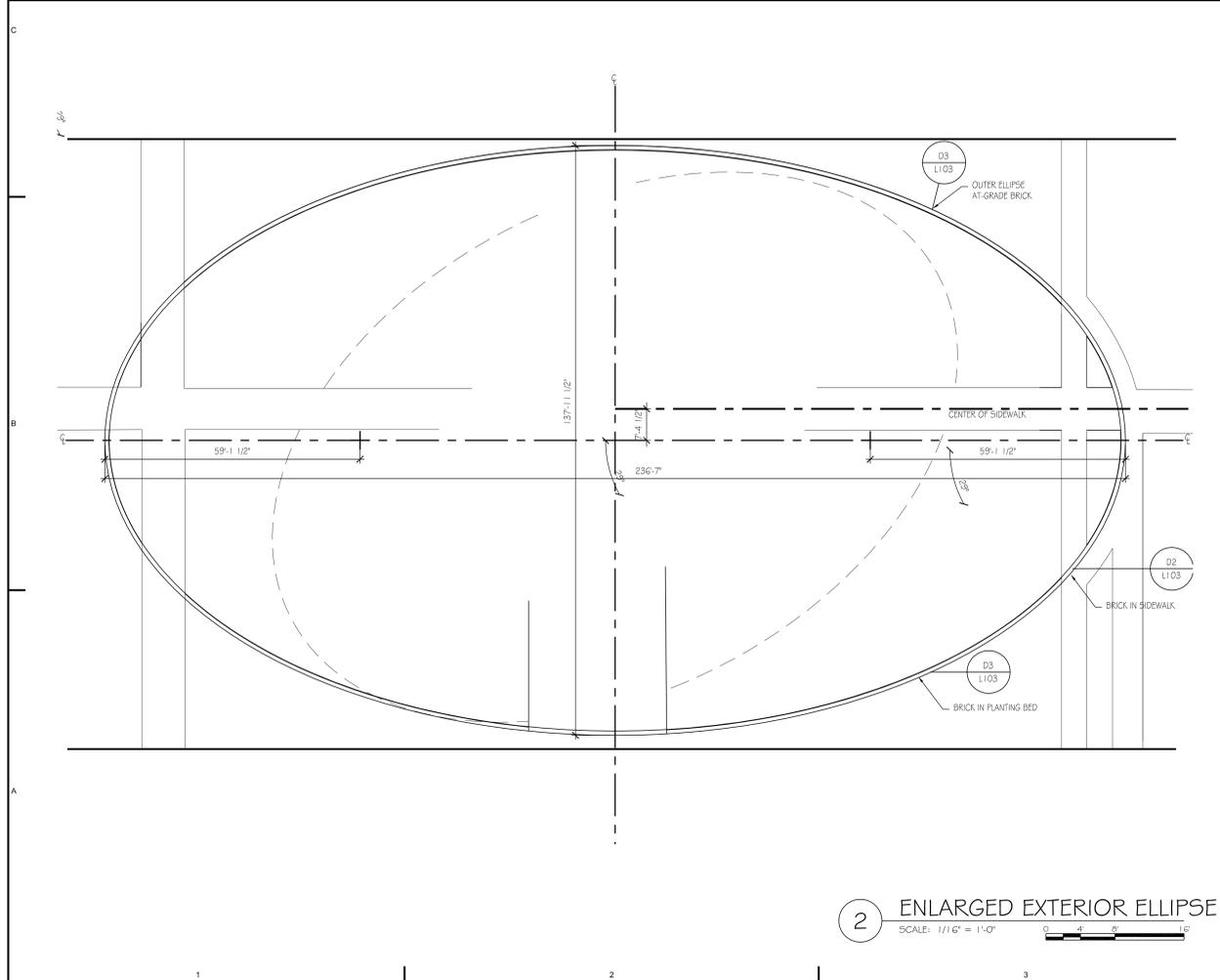
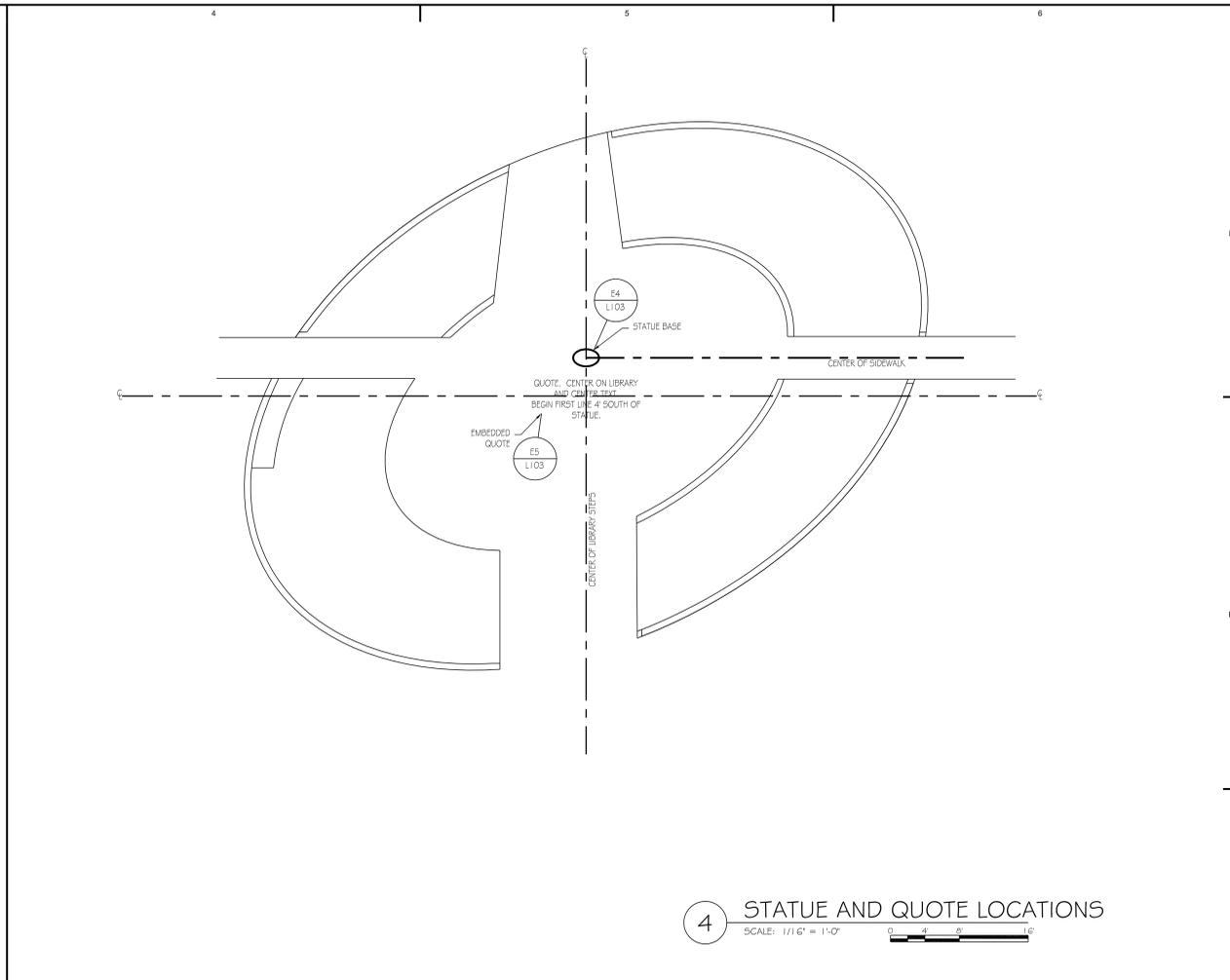
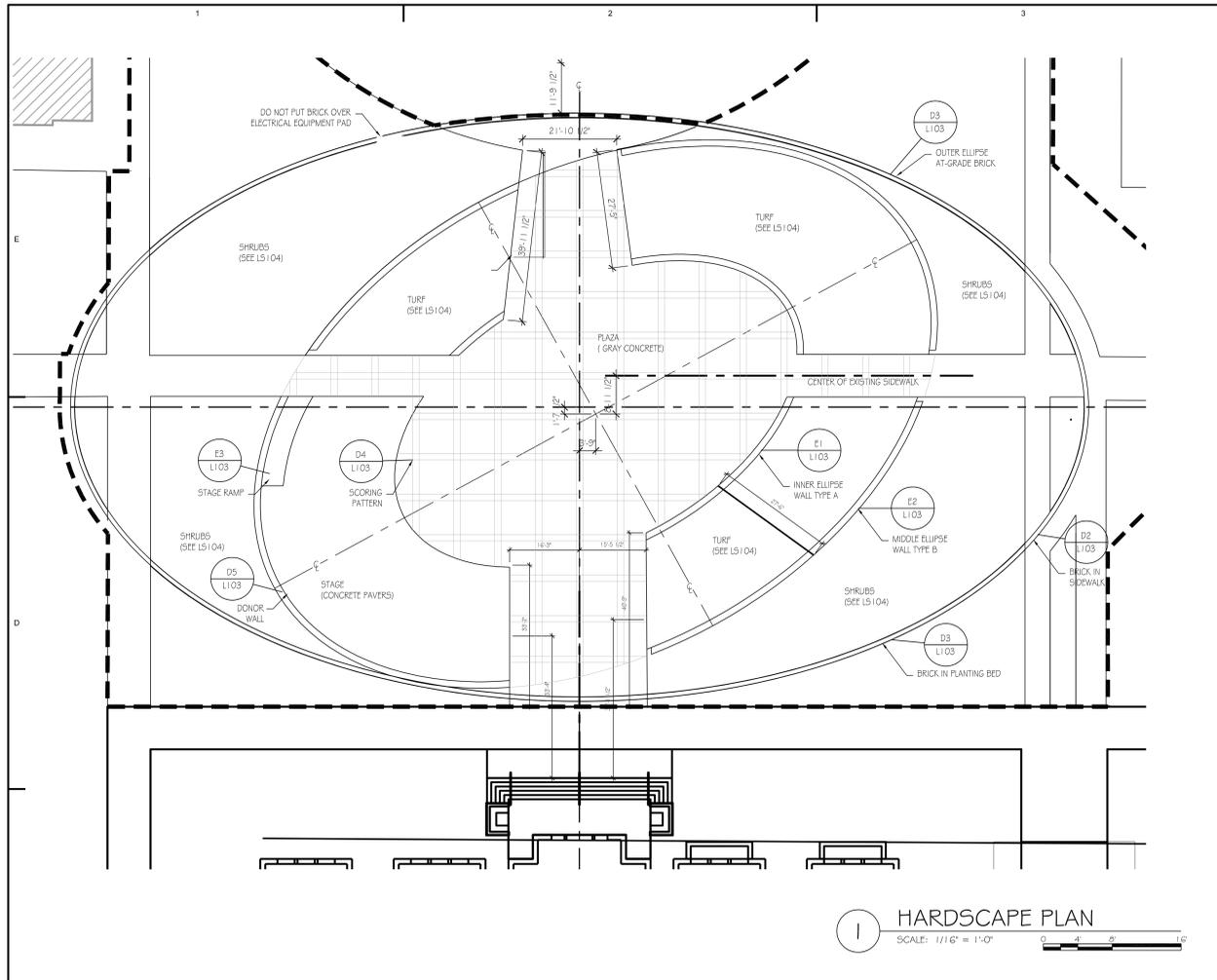
ISSUE DATE:
SEPTEMBER 15 2008, 100% CD

ARCHITECT'S PROJECT NUMBER:
B07-051

DFCM PROJECT NUMBER:
07258700

SHEET TITLE:
IRRIGATION DETAILS

SHEET NUMBER:
LR-502



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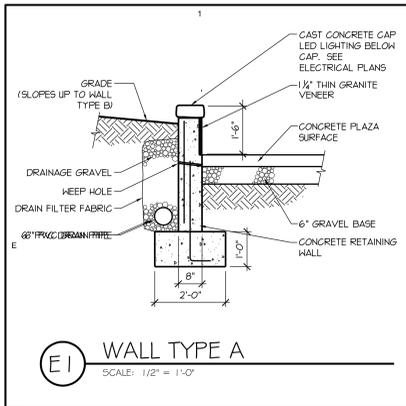
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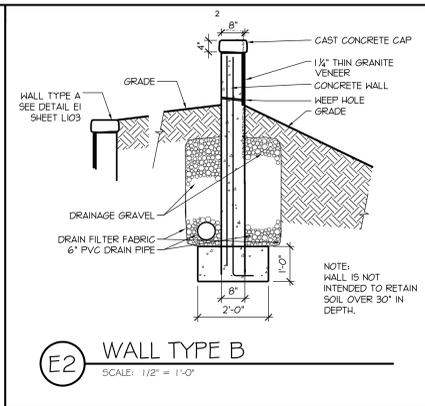
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150 College Avenue
Ephraim, Utah 84627

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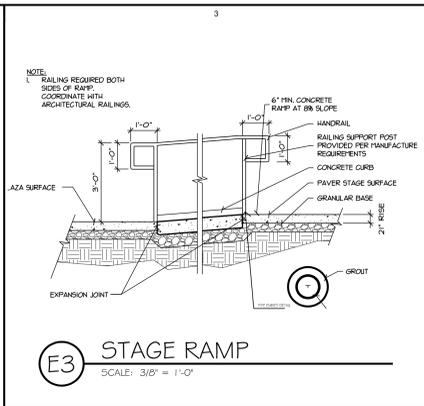
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ARCHITECT'S PROJECT NUMBER:
B07-051
DFCM PROJECT NUMBER:
07258700
SHEET TITLE:
HARDSCAPE PLAN
ADD ALT 1 SCHEME B
SHEET NUMBER:
LS102



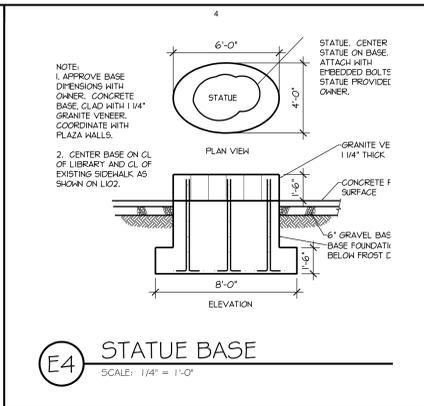
E1 WALL TYPE A
SCALE: 1/2" = 1'-0"



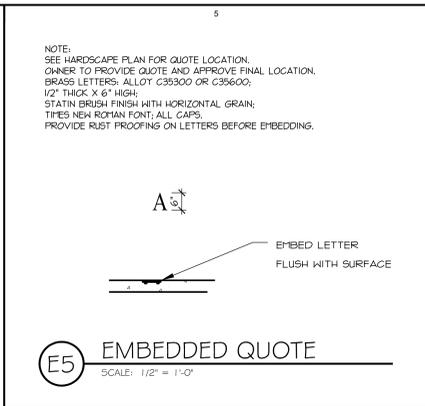
E2 WALL TYPE B
SCALE: 1/2" = 1'-0"



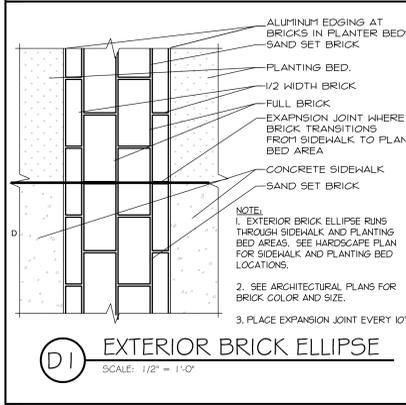
E3 STAGE RAMP
SCALE: 3/8" = 1'-0"



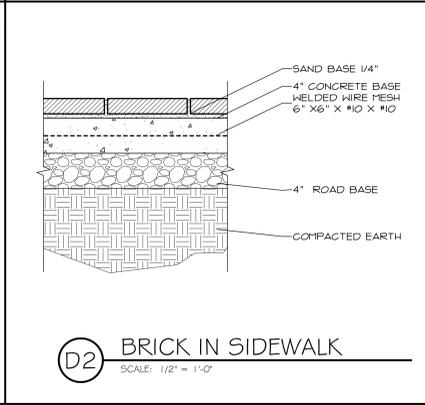
E4 STATUE BASE
SCALE: 1/4" = 1'-0"



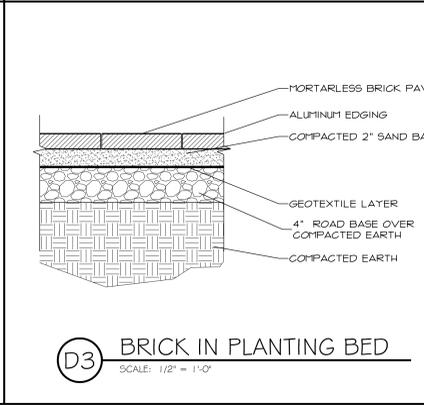
E5 EMBEDDED QUOTE
SCALE: 1/2" = 1'-0"



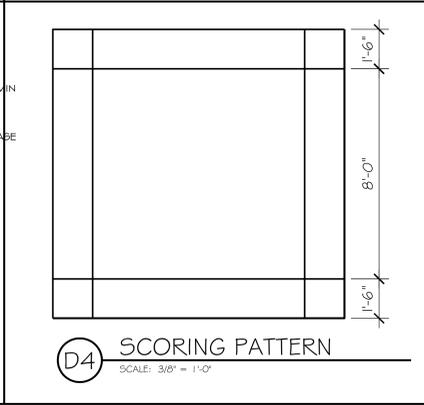
D1 EXTERIOR BRICK ELLIPSE
SCALE: 1/2" = 1'-0"



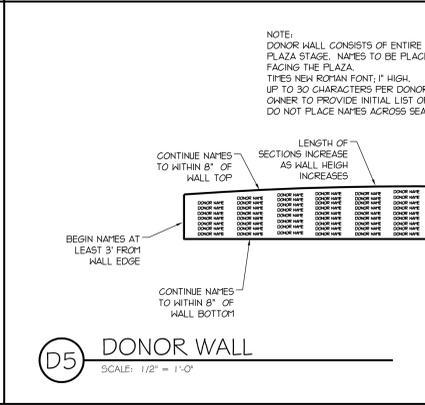
D2 BRICK IN SIDEWALK
SCALE: 1/2" = 1'-0"



D3 BRICK IN PLANTING BED
SCALE: 1/2" = 1'-0"



D4 SCORING PATTERN
SCALE: 3/8" = 1'-0"



D5 DONOR WALL
SCALE: 1/2" = 1'-0"

C
B
A

2
1

3

4

5

6

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HARDSCAPE DETAILS
ADD ALT 1 SCHEME B
SHEET NUMBER:
LS103

I. Design Criteria

- A. Governing Building Code: 2006 International Building Code (IBC)
B. Floor Live Loading: Office, 80 psf Live Load + 20 psf Partition Load; Exit Facilities & Corridors, 105 psf Live Load; Mechanical Rooms, 125 psf Live Load or actual weights, if larger; Library Stacks, 150 psf Live Load; High Density Shelving, 300 psf Live Load; Exterior patio, 100 psf Live Load
C. Roof Live Loading: Roof Live Load, 20 psf; Roof Snow Load, 36 psf + Drift per IBC; Ground Snow Load, Fg, 47 psf; Snow Exposure Factor, Ce, 1.0; Importance Factor, Is, 1.1; Thermal Factor, Ct, 1.0; Roof Slope Factor, Cs, 0.8
D. Earthquake: Occupancy Category, III; Seismic Design Category, D; Spectral Response Accelerations, Sa = 0.65 g, S1 = 0.23 g, S0.1 = 0.30 g; Soil Site Class, D; Basic Seismic-Force-Resisting System, Special Concrete Shear Wall; R = 5.0, Cd = 5.0, Qp = 2.5; Importance Factor, Ie, 1.25; Design Base Shear, 556 kips; Design Story Drift, Delta, 1.5; Analysis Procedure, Equivalent Lateral Force (Static)

- E. Wind: Basic Wind Speed (3-second gust), 90 mph; Importance Factor, Iw, 1.15; Exposure, B; Internal Pressure Coefficient, GCp, 0.18; Topographic Factor, Kzt, 1.0
F. Foundation: Subsurface Conditions: Soils report and log of borings was obtained by the Owner for the Engineer's use in the design of the foundation, and is not a part of the Contract Documents. This report and log of borings is available for the Contractor's information, but is not a warranty of the subsurface conditions. The Contractor may use the report at his own risk.
Soils Report by Gordon Spilker Huber Geotechnical Consultants, dated 4/22/08
Soil Bearing Pressure: use deep foundation system
Lateral Soil Pressure Fluid Equivalent Density
a. Active: 35 pcf (retaining walls)
b. At Rest: 56 pcf (rigid foundation walls)
c. Passive: 300 pcf
d. Increase for Seismic: 20 pcf
Coefficient of Friction: 0.4

- G. Classification for Fire Rated Construction: For the purpose of determining fire-resistive assemblies, open-web steel framing members shall be considered unrestrained. All other steel floor and roof framing members shall be considered restrained.

II. Earthwork

- A. Clearing: The entire building area shall be scraped to remove the top 4 inches of soil, including all vegetation and debris.
B. All moisture sensitive soils shall be removed and replaced with compacted structural fill. The depth of this material is approximately 22 feet. Do not place any footing on this material. Contractor is to coordinate with the Geotechnical Engineer to determine which soils require removal.
C. Proof rolling: The natural undisturbed soil below all footings shall be proof rolled prior to placing concrete. Remove all soft spots and replace with compacted structural fill.
D. Compacted structural fill: All fill material shall be a well-graded granular material with a maximum size less than 4 inches and with 20 percent passing No. 200 sieve. It shall be compacted to 95 percent of the maximum laboratory density as determined by ASTM D1557. All fill shall be tested (See Specifications and the Quality Assurance section of the GSI).

III. Concrete

- A. Materials shall comply with the Standards specified in American Concrete Institute (ACI) 318-05, "Building Code Requirements for Structural Concrete."
1. Compressive strengths of concrete at 28 days shall be as follows:
a. Footings: 3000 psi
b. Slabs on Grade: 3000 psi
c. Reinforced 8" slab on grade: 4000 psi
d. Walls: 4000 psi
e. Columns: 4000 psi
f. Joists, Beams and Suspended Slabs: 4000 psi
g. Normal weight concrete over Steel Deck: 3000 psi
h. All other Site Cast Concrete: 4000 psi
2. Concrete Density (Maximum Air Content):
a. Normal weight concrete shall be approximately 145 to 155 pounds per cubic foot.
b. Lightweight concrete shall not exceed 110 pounds per cubic foot and shall be made of lightweight coarse aggregates and a blend of lightweight and normal weight fines.
3. Reinforcement steel:
a. ASTM A615 Grade 60, fy = 60,000 psi min. unless noted otherwise.
4. Admixtures:
a. Air-entraining admixtures, comply with ASTM C 260 (when used).
(1) When air content of a trowel finished floor slab exceeds 3%, there is an increased risk for delaminations and blistering to occur. When this situation is present, the contractor shall pay special attention to the finishing procedures to help minimize such risks. Refer to ACI 302.1R-96 "Guide for Concrete Floor and Slab Construction" for proper finishing guidelines.
b. Calcium chloride shall not be added to the concrete mix.
5. Only one grade or type of concrete shall be poured on the site at any given time.
B. Formwork shall comply with ACI Standards Publication 347 and the project specifications. The contractor shall be responsible for the design, detailing, care, placement and removal of the formwork and shores.
1. Precamber forms and sores with a camber of 1/4" per every 10'-0" of span to compensate for dead load deflection, unless noted otherwise.
C. Concrete cover requirements for deformed bar reinforcing steel shall comply with ACI 318, "Building Code Requirements for Structural Concrete".
1. Cast-in-place Concrete: Clear Cover
a. Cast against and permanently exposed to earth: 3"
b. Formed concrete exposed to earth or weather:
#6 thru #18 bars: 2"
#5 and smaller bars: 1.12"
c. Concrete not exposed to weather or in contact with ground:
Slabs, Walls, #11 bars and smaller: 3/4"
Beams, Columns, Primary Reinf., Ties, Stirrups, Spirals: 1.12"
D. Construction Joints and Control Joints:
1. Provide a continuous 2 X 4 keyway or a surface intentionally roughened to a full amplitude of approximately 1/4" in all wall footings. Adjust the keyway as necessary to provide for proper bar placement. A continuous keyway shall not be used for concrete shear wall to footing connections, unless specifically indicated. Refer to project plans, schedules and details for the shear wall to footing connection requirements.
2. All horizontal and vertical construction joints shall have a continuous 2 X 4 keyway along the joint or joints that be intentionally roughened to a full amplitude of approximately 1/4", unless noted otherwise.

- 3. Provide reinforcement dowels to match the member reinforcement across the joint except for shear walls, unless noted otherwise. For dowels across construction joints and wall to footing connections of concrete shear walls, refer to specific project plans, schedules, and details.
4. Construction joints in suspended concrete pours shall be made at the center of spans.
5. Slabs on grade shall have construction or control joints spaced not to exceed 30 times the slab thickness in any direction. All discontinuous control or construction joints shall be reinforced with 2 - #4 x 48". See structural details. Construction joints shall not exceed a distance of 125'-0" o.c. in any direction.
6. Control joints shall be installed in slabs on grade so the length to width ratio of the slab is no more than 1.25:1. Control joints shall be completed within 12 hours of concrete placement. Control joints may be installed by:
a. Saw cut a depth of 1/4 the thickness of the slab
b. Tooled joints a depth of 1/4 the thickness of the slab
7. Control joints in visually exposed walls, unless noted otherwise: (Joints shall line up with masonry and architectural joints, see drawings.)
a. Vertical control joints at 10'-0" on center.
b. Reinforcing shall be continuous through control and construction joints, unless noted otherwise.
c. Control joints in concrete foundation walls shall line up with masonry control joints.
8. Control joints shall be installed in suspended slabs over steel decking by sawcutting along all interior grid lines. Joints centered above the purlins shall have #4 X 5'-0" at 16" o.c. reinforcing, placed perpendicular (and centered) to the purlin. Joints centered above the girders shall be 3/4" deep and shall have #4 X 10'-0" at 16" o.c. reinforcing placed perpendicular (and centered) to the girder. The #4 bar reinforcing centered above the grid lines is in addition to the specified WWF continuous throughout the suspended slabs over steel decking. Reinforcing shall be placed 1" below the top of the slab.

- E. Detailing: All reinforcing, including WWF, shall be detailed, bolstered & supported to comply with ACI 315, "Details and Detailing of Concrete Reinforcement" and the Concrete Reinforcing Steel Institute (CRSI) recommendations. Reinforcing bars shall not be welded unless specifically shown on drawings.

- 1. Lap splice lengths shall be detailed to comply with the "Reinforcing Bar Lap Splice Schedule" contained within the contract drawings. Splices may be made with mechanical splices capable of 125% tension capacity of the bar being spliced. Mechanical splices shall be the positive connecting type coupler. They shall be covered by a current ICC Code Evaluation Report. Use "Cadweld" splice sleeves with ferrous filler, "Lentor" taper threaded rebar splices, "Bar-Lock" lockshear bolt coupling sleeves, or approved equivalent. If mechanical splices are used, splices or couplers on adjacent bars shall be staggered a minimum of 24" apart along the longitudinal axis of the reinforcing bars.
2. All embedments and dowels shall be securely tied to formwork or to adjacent reinforcing prior to the placement of concrete.
3. Use chairs or other support devices recommended by the CRSI to continuously support at 36" o.c. maximum.
4. Provide corner bars at intersecting wall corners using the same bar size and spacing as the horizontal wall reinforcing. Unless noted otherwise, corner bar lengths shall conform with reinforcing bar lap splice lengths as noted above.
5. All vertical reinforcing shall be dowelled to footings, or to the structure below. Dowels shall be the same size and at the same spacing as the vertical reinforcing scheduled (or detailed) for the element above. Lap splice lengths shall comply as noted above or as shown in the drawings. Dowels extending into footings shall terminate with a 90 degree standard ACI hook and shall extend to within 4" of the bottom of the footing. Footing dowels (#6 bars and smaller) with hooks need not extend more than 20" into footings.
6. Horizontal wall reinforcing shall terminate at ends of walls and openings into the far end of the jamb column with a 90-degree standard ACI hook, unless shown otherwise. Lap horizontal bar splices as noted above or as shown in the drawings. Horizontal wall reinforcing shall be continuous through construction and control joints. Splices in horizontal reinforcing shall be staggered, so the splice laps will not overlap. Splices in two curtains where used shall not occur in the same location, splice laps shall not overlap.
7. Wall Openings 36" wide and wider: Place 2 - #5 bars (or 1 - #7 bar in 10" walls and thinner) around all openings 8" or larger in any direction, and extend the reinforcing bars a minimum of 24" beyond the corner of the openings, unless noted otherwise. Where 24" is not available, extend bars as far beyond the opening as possible and terminate them with a 90 degree standard ACI hook.
8. Wall Openings 36" wide and wider: Provide reinforced concrete lintels per Concrete Lintel Schedule over the top of and 2-#5 bars (or 1 - #7 bar in 10" walls and thinner) and on all sides and below every unscheduled opening, unless noted otherwise. Bars for all openings shall extend a minimum of 24" beyond the corners of the opening. Vertical bars shall extend from floor level below the floor, or roof, level above. Where 24" extension is not possible, extend bars as far beyond the opening as possible and terminate them with a 90 degree standard ACI hook.
9. Provide 2-#5 X 4'-0" diagonal bars (or 1 - #7 x 4'-0" bar in 10" walls and thinner) at the corners of all openings. Diagonal bars shall be centered on the corner of the opening. All recesses in concrete walls that interrupt reinforcing steel shall be reinforced the same as an opening.
10. Contractor shall coordinate placement of all openings, cuts, dowels, sleeves, conduits, bolts, inserts and other embedded items prior to concrete placement.
11. All tied columns shall have ties spaced at one-half the required tie spacing for a distance of one-sixth of the column height above and below all floor (or beam) and roof (or beam) levels or any other point of lateral support, unless noted or detailed otherwise on the structural drawings.
12. Column cross-ties shall have a 135 degree hook at one end and a 90 degree hook at the other. The hooks shall engage the vertical column reinforcement. The 135 degree hooks of consecutive cross-ties engaging the same vertical bars shall engage alternate vertical bars.
13. Splices in vertical column reinforcing will be permitted at floor levels only, unless shown otherwise. Where changes in the cross section of the column occur, the longitudinal bars shall be offset in a region where lateral support is afforded. Where offset, the slope of the inclined portion of the bar shall not exceed 1 to 6 (horizontal to vertical). In the case of tied columns, the ties shall be spaced not over three inches on center for a distance of one foot above and one foot below the point of offset.
14. All reinforcement shall be bent cold, and shall be bent only once at the same location. All reinforcement shall be shop bent, unless otherwise permitted by the engineer.

Table with 3 columns: Wall Thickness, Horizontal Reinf., Vertical Reinf.
6" #4 @ 13" o.c. #4 @ 18" o.c.
8" #5 @ 15" o.c. #4 @ 16" o.c.
10" #5 @ 12" o.c. #4 @ 13" o.c.
12" #4 @ 13" o.c. Each Face #4 @ 18" o.c. Each Face
Others 0.25% of Wall Area 0.15% of Wall Area

Place steel in the center of the wall (except in walls thicker than 10" and where shown otherwise). Walls thicker than 10" shall have two curtains of reinforcing (placed near each face of the wall), unless otherwise shown on the structural drawings. Spacing shall not exceed three times the wall thickness nor 18". In addition to the above reinforcing, 2 - #5 (or 1 - #7 in 10" walls and thinner) x continuous horizontal bars shall be placed at the bottom of the wall (near the footing) and at each floor level, at the roof level and at the top of wall.

- G. No aluminum conduit or product containing aluminum or any other material injurious to concrete shall be embedded in concrete.
H. Unless otherwise noted, all slabs on grade shall be 4" thick.
I. Expansion anchors noted in the drawings shall be Hilti Kwik Bolt TZ, Simpson Strong-Bolt Wedge Anchor, or equivalent approved by the Structural Engineer of Record.

IV. Masonry

- A. Materials, unless noted otherwise:
1. Concrete Masonry Units: Lightweight Grade N, Type 1 (minimum unit strength of 1900 psi) or better (fm = 1500 psi)
2. Hollow Clay Units: Hollow Brick, Grade 1 (minimum unit strength of 6400 psi average or better. (fm = 2000 psi)
3. Solid Clay Units: Grade SW (minimum unit strength of 4400 psi average or better. (fm = 1500 psi)
4. Mortar: Use Type "S" according to IBC Section 2103.7, and tested according to ASTM C270. Admixtures shall not be added to the mortar mix. (1500 psi minimum compressive strength)
5. Grout: Conform to IBC Table 2103.10 or ASTM C476. Proportioned according to IBC Section 2103.10 and tested according to ASTM C1019. Grout shall attain a minimum compressive strength of 28 days.
6. Reinforcing: Grade 60 reinforcing steel shall comply with ASTM A615. Wire joint reinforcing shall comply with ASTM A951.
7. Deformed Bar Anchors (DBA): All DBAs shall comply with ASTM A496
8. Anchor Bolts (AB): ASTM A307 with ASTM A563 heavy hex nuts and hardened washers, Grade A, unless noted otherwise.
9. Headed Stud Anchors (HSA): Manufacture all HSAs in conformance with ASTM A108 with dimensions complying with AISC specifications.

- B. Construction Requirements:
1. Mortar Joints: Joints shall be "concave", "V-joint" or "weathered raked" for structural members unless noted otherwise on architectural drawings.
2. Masonry walls, beams and columns shall be constructed with running bond, unless noted otherwise.
3. Grouting Requirements: Comply with IBC Section 2104 and ACI 530.1/ASCE 6/TMS 602. Grout shall be mechanically consolidated and mechanically reconsolidated according to ACI 530.1/ASCE 6/TMS 602 Section 3.6 E.
4. Reinforcing Bars shall not be welded unless specifically shown on drawings. In such cases, use only AWS standards. Do not substitute reinforcing bars for DBAs or HSAs.
5. Control Joints: Spacing shall not exceed 40'-0". See architectural drawings for locations.
6. Grout all beam and joint pockets solid after installation of beams and joists.
7. Masonry Veneer Attachment and Reinforcing:
a. To steel stud and wood stud walls: Veneer shall be attached to the studs with Dur-O-Wal D/A 213 S seismic veneer anchors or Hohnmann & Barnard DW-10 or DW-10HS seismic veneer anchors (or equal) spaced at 16" o.c. Veneer anchors shall be attached to studs with #10 corrosion resistant self drilling screws. Attach the veneer to the anchors with Dur-O-Wal Seismic Steel Pintles or Hohnmann & Barnard 3/16" Byna-Tie with Seismiclips (or equal) spaced at a maximum of 16" o.c. in both directions. Anchor ties shall engage to a galvanized No. 9 gauge horizontal joint reinforcement wire in the veneer which shall be continuous and shall be placed at 16" o.c. maximum at the center of the veneer.
b. To concrete walls: 22 gauge galvanized dovetail slots shall be installed vertically in concrete at 16" o.c. Attach the veneer to dovetail slots with Dur-O-Wal D/A 16 gauge seismic dovetail anchor ties or Hohnmann & Barnard 3/16" Byna-Tie with Seismiclips (or equal) spaced at a maximum of 16" o.c. in both vertical and horizontal directions. Anchor ties shall engage to a galvanized No. 9 gauge horizontal joint reinforcement wire in the veneer which shall be continuous and shall be placed at 16" o.c. maximum at the center of the veneer. Dovetail slots and anchor ties shall be galvanized.
c. To reinforced masonry walls: Veneer shall be attached with tri-rod ladder type reinforcement spaced at a maximum of 16" o.c. vertically consisting of 3 - #9 galvanized, galvanized, corrugated, wires. Veneer may also be attached with Dur-O-Wal D/A 360 S Seismic Ladder-Eye spaced at 16" o.c. maximum in both vertical and horizontal directions. Anchor ties shall engage to a galvanized No. 9 gauge horizontal joint reinforcement wire in the veneer which shall be continuous and shall be placed at 16" o.c. maximum at the center of the veneer. Anchors shall extend to the galvanized ladder type (2 - #9 wires) joint reinforcing in the masonry wall spaced 16" o.c. maximum.
d. Other methods of attachment may be used after written acceptance by the architect and structural engineer.

- e. Steel Lintels: Provide steel angle lintels at all openings through the masonry veneer. Provide one inch of bearing for each foot of width of opening, with a minimum bearing of six inches. See the Steel Angle Lintel Schedule for size.
C. Detailing Requirements:
1. Standards: Reinforcing detailing shall comply with American Concrete Institute (ACI) Standard 315, "Details and Detailing of Concrete Reinforcement."
2. Reinforcement Protection (cover):
a. Joint reinforcement shall have not less than 5/8" mortar coverage from the exposed face.
b. Other reinforcement shall have a minimum coverage of one bar diameter over all the bars, but not less than 3/4". When masonry is exposed to soil, minimum coverage shall be 1 1/2".
3. Vertical steel reinforcement shall be placed and secured against displacement prior to grouting by wire positioners or other suitable devices, at intervals not exceeding 112 bar diameters, at the grid line heights, or at bar splice locations, whichever is less. Vertical reinforcing shall be located at the center of the wall, unless noted otherwise.
4. Lap Splice Lengths: Lap all masonry reinforcing bars per the "Masonry Reinforcing Bar Lap Splice Schedule." Joint reinforcement shall lap a minimum of 6".
5. Corner Bars: Horizontal reinforcing shall be continuous at all corners and at intersecting walls. Provide corner bars with the required lap splice length.
6. Dowels: All vertical reinforcing shall be dowelled to the foundation wall, footing (structure below) and to the structure above with the same size dowel, spacing (and in the same core) as the vertical wall reinforcing unless noted otherwise.
7. Wall Openings 24" wide and wider: Provide reinforced masonry lintels per Masonry Lintel Schedule over the top of, and 2 - #5 bars, in grouted spaces, on all sides and adjacent to every unscheduled opening, unless noted otherwise. Bars for all openings shall extend a minimum of 48 bar diameters beyond the bottom chord of the opening. Vertical bars shall extend from floor level below to the floor, or roof, level above. Where a 48 bar diameter extension is not possible, extend bars as far beyond the opening as possible and terminate them with a 90 degree standard ACI hook.
8. Horizontal wall reinforcing shall be continuous through joining concrete walls, masonry walls, columns, and pilasters. Provide a key between the wall and the column or pilaster. Horizontal wall reinforcing shall be placed inside the column vertical reinforcing.
9. Anchor bolts and headed stud anchors shall be set in a grouted cell. Anchor bolts and headed stud anchors shall have 1" grout surrounding the shank at its penetration. Grout shall be flush with the face or top of the masonry.
10. All masonry column ties shall terminate with 135 degree hooks plus a 6 bar diameter extension (4" minimum).
11. The exposed face of all embed plates shall be set flush with the face of masonry wall or column.

- D. Minimum Reinforcing: All masonry walls shall be reinforced as follows, unless shown otherwise on the drawings. Reinforcing shall be placed in grouted cells.
Table with 3 columns: Wall Thickness, Horizontal Reinf., Vertical Reinf.
6" #4 @ 48" o.c. #5 @ 32" o.c.
8" #5 @ 48" o.c. #5 @ 32" o.c.
10" #5 @ 48" o.c. #6 @ 32" o.c.
12" 2 - #5 @ 48" o.c. #6 @ 32" o.c.
E. Structural Steel
A. Material:
1. W-Shapes: ASTM A992, (Fy = 50 ksi), except as noted otherwise.
2. All Other Shapes and Plates: ASTM A572 (Fy = 36 ksi), except as noted otherwise.
3. Rectangular and Square Hollow Structural Sections (HSS): ASTM A500, Grade B (Fy = 46 ksi).
4. Round HSS: ASTM A500, Grade B (Fy = 42 ksi).
5. Steel Pipe: ASTM A53, Grade B (Fy = 36 ksi).
6. Deformed Bar Anchors (DBA): ASTM A496.
7. Headed Stud Anchors (HSA): ASTM A108, with dimensions complying with AISC specifications.
8. Anchor Rods: ASTM F1554, Grade 36 with ASTM A563 heavy hex nuts and ASTM F436 hardened washers, unless noted otherwise.
B. Fabrication and construction shall comply with the following Codes and Standards:
1. American Institute of Steel Construction (AISC) 360-05, "Specification for Structural Steel Buildings," dated March 9, 2005.
2. AISC 344-05, "Seismic Provisions for Structural Steel Buildings" dated March 9, 2005, including Supplement No. 1, dated November 16, 2005.
3. AISC 303-05, "Code of Standard Practice for Steel Buildings and Bridges" excluding the following Section 3.3 (last sentence of first paragraph), Section 4.4, Section 4.1, Section 4.4.2, Section 4.5, and Section 7.13.3.
a. The architectural drawings are the prime contract drawings. Consultants' drawings by other disciplines are supplementary to the architectural drawings. The structural drawings shall be used in conjunction with the architectural drawings. Detailing and shop drawing production for structural elements will require information (including dimensions) contained in architectural, structural, and/or other consultants' drawings. Refer to VII. Special Instructions, notes VII B and VII C on this sheet.
4. AISI/CISC 2004, "Specification for Structural Joists Using ASTM A325 or A490 Bolts".
5. American Welding Society (AWS) D1.1-04, "Standard Welding Code - Steel" (specific items do not apply when they conflict with the AISC requirements).
6. Steel Joist Institute (SJI) 2005, "Standard Specification, Load Tables and Weight Tables for Steel Joists and Joist Girders," and Recommended Code of Standard Practice.
7. American Iron and Steel Institute (AISI) 2001, "North American Specification for the Design of Cold-Formed Steel Structural Members."
C. Structural shapes and plates shall be fabricated from newly rolled (milled) one-piece sections without splices, unless specifically noted otherwise on the structural drawings. Connections for structural steel shall comply with the structural drawings, unless written approval is given by the structural engineer.
D. Welding:
1. Certification of Welders: All shop and field welding shall be executed by AWS certified welders who have been specifically certified for the type of work to be performed. Certification shall be considered current if dated within the past 12 months. Welders will be considered certified if they have been certified under AWS and their work records are current within every six-month period thereafter as required by AWS. Certification and records must comply with AWS Standards. Certification and appropriate records must be provided to the architect prior to beginning work.
2. Electrodes: E-70 XX or as noted otherwise. E80 XX may be used for welding steel floor and roof decks.
3. Minimum Welds: All intersecting steel shapes that are not bolted shall be connected by a fillet weld all around, unless noted otherwise. Fillet weld sizes that are not shown shall be 1/16" less than the thinnest of the connected parts for thicknesses 1/4" and larger. Fillet welds on plates less than 1/4" shall be of the same size as the thinnest of the connected parts.

- 4. Reinforcing Bars: Do not weld rebar except as specifically detailed in the drawings. In such cases, use only AWS standards. Do not substitute reinforcing bars for deformed bar anchors (DBAs), machine bolts, or headed stud anchors (HSAs).
5. Bolts: Do not apply any welds, including "back" welds to bolts, including anchor bolts, except as specifically detailed in the drawings.
6. It is recommended the steel erection contractor and steel fabricator contact the Quality Assurance Agency prior to beginning any of the above welds. A program of joint preparation and welding procedures should be worked out between the two parties before the welding is started so that correct weld will be made from the beginning.
7. Headed Stud Anchor (HSA) welding and Deformed Bar Anchor (DBA) welding shall conform to the manufacturer's specifications. Welding shall comply with AWS D1.1 Section 7.8 through 7.8 and Annex IX.

- E. Bolted Connections:
1. Use ASTM A325N bolts for all steel to steel connections, unless noted otherwise. Tighten bolts by the turn of the nut, calibrated wrench, or direct tension indicator method. Alternate fastener designs as defined by AISC shall be submitted to the engineer for review and acceptability prior to installation. Provide hardened washers beneath turned element.
2. Use ASTM A325SC bolts for all steel to steel connections in moment frames and connections subject to vibrations, unless noted otherwise. Tighten bolts by the turn of the nut, calibrated wrench, or direct tension indicator method. Alternate fastener designs as defined by AISC shall be submitted to the engineer for review and acceptability prior to installation.
3. Provide hardened washers beneath the turned element of all bolts or nuts. Provide hardened beveled washers, to compensate for the lack of parallelism, where the outer face of the bolted parts has a slope greater than one in twenty with respect to the plane normal to the bolt axis. Hardened washers or plates installed over oversized holes or slotted holes shall be at least 5/16" thick and shall conform to ASTM F436. Plates or bars installed at slotted holes shall have a size sufficient to completely cover the slot after installation.
4. Where a steel to steel connection is not detailed in the drawings, provide a standard AISC framed connection with the capacity to support one half of the total uniform load capacity of the given shape for the span and for the steel specified.
5. Bolts, nuts and washers shall not be reused.

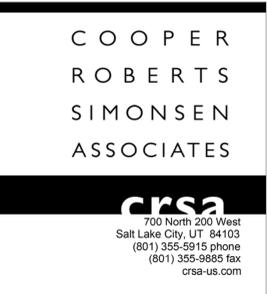
- F. Steel Lintels: Provide steel angle lintels at all openings through the masonry veneer. Provide one inch of bearing for each foot of width of opening, with a minimum bearing of six inches. See the Steel Angle Lintel Schedule for size.

Table with 3 columns: Beam Web Stiffener Plates, Beam Web Stiffener Thickness, Weld Size.
1/4 inch thick: Greater than 0' and less than 8 1/4" 3/16"
3/8 inch thick: Greater than 8 1/4" and less than 12 1/4" 1/4"
1/2 inch thick: Greater than 12 1/4" and less than 16 1/2" 5/16"
5/8 inch thick: Greater than 16 1/2" and less than 20 3/4" 3/8"

- H. Composite Beams:
1. Composite beams are indicated on the framing plans with the suffix (). The number inside the brackets indicates number of studs for this beam. Beams shall have the studs spaced uniformly over the entire beam length.
2. Beams which have more than one number inside the symbol (, ,), such as (W, X, Y, Z), shall have "W" number of studs spaced evenly over the first "section of beam", "X" number of studs spaced evenly over the second "section of beam", "Y" number of studs spaced evenly over the third "section of beam", and "Z" number of studs spaced evenly over the fourth "section of beam". A "section of beam" is defined as that portion of beam located between the column and the nearest intersecting framing member or that portion of beam located between two adjacent intersecting framing members.
3. All headed stud type shear connectors shall be in conformance with ASTM A108. Dimensions shall comply with AWS. Use 3/4" diameter studs. Headed studs shall extend 1 3/4" minimum above the top of the steel deck after welding. Headed studs shall be applied through the metal deck to the top flange of the steel section, or welded directly to the steel section.
4. The minimum center-to-center spacing of stud connectors shall be six (6) diameters along the longitudinal axis of the supporting composite beam and four (4) diameters transverse to the longitudinal axis of the supporting composite beam. The maximum center to center spacing shall not exceed 32".
5. C = 0" on the plans denotes precamber dimension (upward) in inches.

- I. Open Web Steel Joists and Girders:
1. The steel joist supplier shall be responsible for the design of all open web steel joists and girders. Joists or girders with slopes greater than 1/2 inch per foot shall be designed to meet or exceed the load capacities, listed in the SJI load tables, of the joist or girder sizes indicated on the framing plan, as if the joists or girders were installed level.
2. Open web joists and girders shall be designed with deflection limits of L/240 for total load and L/360 for live load, where L is the joist span.
3. Where uplift loads due to wind are indicated, rows of bottom chord bridging shall be provided at the first bottom chord panel points per SJI Specifications. This is in addition to the bridging shown on the framing plans.
4. Provide special bearing ends to accommodate slopes from sloped joists, sloped girders or sloped bearing conditions.
5. Camber: All joists shall be cambered as specified in the SJI specifications, unless noted otherwise.
6. Field Modifications: Do not modify any joist or girder, including holes through the top and bottom chords, without the written consent and direction from the manufacturer.
7. Shop Drawings and Design Calculations: Shop drawings for all joists and girders used in the project shall be submitted for review. Prior to the fabrication of joists and girders, the open web steel joist and girder manufacturer shall prepare complete joist and girder calculations under the direct supervision of a professional engineer licensed in the State of Utah. Calculations shall be submitted for review for joists and girders designated as SPECIAL, or SP and for all joists or girders with axial loads or other loads (as noted on the drawings). Submitted calculations shall bear the seal of a professional engineer licensed in the State of Utah.
8. Stabilizer Plates: Provide steel joist and joist girder stabilizer plates as indicated. Stabilizer plates shall be 6" x 6" with a 13/16" diameter hole with 1 1/2" minimum edge distance, and shall extend 3" minimum below the bottom chord of the joist or girder. Plate thickness shall be equal to the chord gap minus 1/4", or 3/4", whichever is less.
9. Verify size, weight, location and configuration of all roof top equipment with architect and mechanical engineer. Provide steel frames for support of roof top equipment as indicated on structural details in the structural drawings. Coordinate openings with the mechanical and general contractor.
10. All concentrated loads greater than 100 pounds supported by open web steel joists and girders shall be located within 6 inches of joist or girder panel points or the joist or girder shall be reinforced with an additional web member. Refer to the "TYPICAL DETAIL AT ADDITIONAL CONCENTRATED POINT LOADS" in the structural drawings.
11. Concentrated point loads, single or multiple, totaling 100 pounds or less can be located at any point along the top or bottom chord of an open web steel joist or girder between adjacent panel points without meeting the requirements above. A limit of four concentrated 100# maximum

Structural Drawing List table with columns: Sheet Number, Sheet Name.
SE001 General Structural Notes
SE002 General Structural Notes
SB101 Basement - Footing & Foundation Plan
SB102 Basement - Loading Plan
SB103 Tunnel Plan & Section
SB301 Stair Sections
SB501 Typical Footing & Foundation Details
SB502 Typical Footing & Foundation Details
SB601 Structural Schedules
SB602 Structural Schedules
SF102 Level 1, Main Floor Framing Plan
SF103 Level 2 Floor Framing Plan
SF104 Level 3, Attic Framing Plan
SF105 Roof Well Framing Plan
SF106 Roof Framing Plan
SF201 Braced Frame Elevations & Details
SF401 Enlarged Stair Framing Plans
SF501 Typical Floor Framing Details
SF502 Typical Floor Framing Details
SF503 Typical Floor Framing Details
SF504 Typical Floor Framing Details
SF505 Typical Floor Framing Details
SF511 Typical Roof Framing Details
SF512 Typical Roof Framing Details
SF513 Typical Roof Framing Details
SF601 Structural Schedules
SF602 Structural Schedules



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STAMP:
Professional Engineer Seal for Dave Waldron, No. 154201, Arthur Parker Brown, Utah.

PROJECT NAME:
Snow College Library
150 College Avenue
Ephraim, Utah 84627

REVISIONS:
100% CD September 15, 2008
100% CD Review, August 4, 2008

ISSUE DATE:
SEPTEMBER 15 2008, 100% CD

ARCHITECT'S PROJECT NUMBER:
B07-051

DFCM PROJECT NUMBER:
07258700

SHEET TITLE:
General Structural Notes

SHEET NUMBER:
SE001

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point loads per joist or girder will be permitted on spans of 12' or greater, one concentrated 100# max. load on spans less than 12', unless specifically noted otherwise on the structural drawings.

12. Joist bridging shall never be used to support hanging loads.

13. Bracing of mechanical, electrical, plumbing, etc.) to the bottom chord of joists or girders will not be allowed in any instance. All lateral braces must connect to the top flange/top chord of the framing member above unless noted otherwise on the structural drawings.

14. Upon completion of fabrication, the steel joist manufacturer shall submit a certificate of compliance in accordance with section 1704.2.2 stating that the work was performed in accordance with approved construction documents and with the SJI standard specifications.

J. Steel Floor Deck:

- Steel floor deck shall comply with the latest requirements of the Steel Deck Institute (SDI), given to the contractor for review with load and lateral shear capacities with shop drawings.
- Steel floor deck shall be 3" deep X 20 gauge minimum phosphatized/painted composite (deck shall be galvanized (G60) when used above or below mechanical equipment rooms), type "W" deck with interlocking side seams with the following properties:

Minimum S (in ² /ft)	20 Gauge	18 Gauge
	0.534	0.197
Minimum I (in ⁴ /ft)	0.896	1.203

 Painted deck shall be coated with special paint to receive sprayed-on fire proofing, where required.
- A 2 1/2" thick (5 1/2" overall) normal weight concrete (f'c = 3,000 psi @ 28 days unless noted otherwise) slab shall be poured over the steel deck. Reinforce slab with 6" x 6" - W2.1/W2.1 welded wire fabric (minimum) or reinforce slab with 2 pounds per cubic yard minimum polypropylene fibrillated fiber reinforcement, as per specifications. Welded Wire Fabric shall be placed 1" to 1 1/2" below the top of the slab.
- Steel deck with 2 1/2" thick (5 1/2" overall) normal weight concrete slab shall have a minimum diaphragm shear capacity of 2390 lbs/ft. for a 10'-0" deck span.
- Weld deck to supporting framing members with 3/4" diameter puddle welds at the following spacing (Closer spacings may be used to develop minimum shear requirements.)
 - 12" o.c. to supports perpendicular to deck corrugations (4 welds per 36" wide sheet).
 - 12" o.c. to all supports parallel to deck corrugations.
 - Stud welds may take the place of puddle welds where studs are welded through the deck to the top flange of beams.
- All welded surfaces shall be dry before welding deck or studs to supports.
- Attach interlocking seams with 3/16" button punch at 18" o.c. or 1 1/2" top seam weld at 36" o.c. between adjacent pieces of deck. Crimp seams before button punching or welding interlocking seams. Closer spacings may be used to develop minimum shear requirements.
- All deck shall be 3-span continuous minimum where possible. In areas where 3-span conditions are not possible, the deck shall meet the above loading criteria for the span condition. The contractor shall provide heavier gauge deck or provide shoring as required for one or two span conditions to meet the equivalent loading of the above deck under a three span condition.
- Deck shall have a minimum bearing length of 2".

K. Steel Roof Deck:

- Steel roof deck shall comply with the latest requirements of the Steel Deck Institute (SDI). Submit ICC Code Evaluation Report with load and lateral shear capacities with shop drawings.
- Steel roof deck shall be 1 1/2" deep X 20 Gauge minimum galvanized (G60), type "B" wide rib deck with interlocking side seams with the following properties:

Minimum S (in ² /ft)	20 Gauge	18 Gauge
	0.234	0.322
Minimum I (in ⁴ /ft)	0.213	0.296

 Painted roof deck shall be coated with special paint to receive sprayed-on fire proofing, where required.
- Minimum allowable deck diaphragm shear values shall be 890 lbs/ft. for a 6'-0" deck span.
- Weld steel roof deck to supporting framing members with 3/4" diameter puddle welds at the following spacings (Closer spacings may be used to develop minimum shear requirements.)
 - 6" o.c. to all supports perpendicular to deck corrugations (7 welds per 36" sheet).
 - 6" o.c. to the following supports parallel to deck corrugations: All perimeter supports, all concrete or masonry walls, all joists or joist girders with a "TKC" designation, and all steel beams which are on a grid line with braced frames or moment frames.
 - 12" o.c. to all other supports parallel to deck corrugations.
 - All welded surfaces shall be dry before welding deck to supports.
- Attach interlocking seams with 1 1/2" long top seam welds at 12" o.c. minimum between adjacent pieces of decking. Crimp side seams before welding. Closer spacings may be used to develop minimum shear requirements.
- Alternate means of deck attachment and fastening using screws, pins or other mechanical fastening methods are permitted with approval of the engineer. The contractor shall submit the proposed attachment system and the code evaluation report demonstrating the system has the strength to meet the required deck shears specified above (in item 3) and on the plans. The flexibility factor of the proposed deck attachment system shall be equal to or less than the flexibility factor for the specified welded deck attachment system. If the alternate attachment method is approved, it is the responsibility of the contractor to ensure that the deck type and profile is compatible with the fastening system being used.
- Provide a 2" minimum bearing and a 4-inch lap at the splice points of all pieces of deck.
- All deck shall be 3-span continuous minimum where possible. The contractor shall provide heavier gauge deck as required for one or two span conditions to meet equivalent loading of the above deck under a three span condition.
- Steel roof deck shall not be used to support loads from plumbing, HVAC ducts, light fixtures, architectural elements or equipment of any kind, unless specifically noted otherwise. Lightweight suspended acoustical ceilings with a total weight per wire not exceeding 50# may be hung from the steel roof deck. The hangers should be staggered to distribute the load over multiple deck futes.

L. Cold-Formed Steel

- Light Gauge Steel Framing:
 - Where steel framing size designators are used in the drawings, they follow the convention established by the Steel Stud Manufacturers' Association (SSMA) and the North American Steel Framing Alliance (NASFA). Framing members provided shall comply with the designations according to this convention.
 - All load bearing stud (and/or) joist framing members along with all runner, bridging, and end track shall be of the designation shown on the plans. All studs with base metal thickness of 54 mil and 68 mil, and joists with base metal thickness of 54 mil, 68 mil and 97 mil, shall be formed from steel meeting the requirements of ASTM A570 except that the steel shall have a 50,000 psi yield stress. All track and end closures with base metal thickness of 54 mil and 68 mil, bridging with base metal thickness of 54 mil, and studs and track with base metal thickness of 43 mil and 33 mil, shall be formed from steel with a minimum yield of 33,000 psi. All components shall be galvanized.
 - Follow all manufacturers' recommendations for the use of these products.
 - Unless noted otherwise, all welded connections shall be done using 1/8" AWS type 6013 or 7014 rod with a welding heat of 60-110 amperes depending on the gauge of material and the fit of the parts. Wire tying of framing components shall not be permitted.
 - All interior non-bearing steel stud walls that extend above the ceiling but do not attach to the floor or roof diaphragm (above) shall have diagonal braces at 45 degrees (+/-). The Kl/r ratio of the brace shall not exceed 200 and shall not be spaced further apart than 10'-0". Connect diagonal braces to the top of the steel stud walls and to the underside of the top flange of the steel beams, open web joists or girders, etc. with 1/8" fillet welds all around. Diagonal angle braces may be connected to 8" x 8" x 1/4" steel plates which shall be anchored to the floor or roof decks which have concrete fill above with 2 - 3/8" dia. expansion anchors for each plate. Connect angle to plate with a 1/4" fillet weld all around. Connect diagonal braces to roof decking which does not have concrete fill above per the non-bearing wall brace connection details shown on the structural drawings. Diagonal braces may be constructed from cold-formed light gauge steel studs but must conform to the Kl/r ratio of less than 200 requirement. When diagonal brace lengths exceed 10'-0", cold-formed box sections made from two 60S137-54 steel studs will likely be required.
- Prefabricated Systems: Submit complete shop drawings and calculations of all elements for review. Submittals shall bear the stamp of a Professional Engineer registered in the State of Utah.

O. Metal Bar Grating:

- Metal bar grating shall comply with the most recent requirements of the National Association of Architectural Metals Manufacturers, Metal Bar Grating Division (NAAMM MBG). Products shall conform to the latest edition of the Metal Bar Grating Manual, ANSI/NAAMM MBG 531, or the Heavy Duty Metal Bar Grating Manual, ANSI/NAAMM MBG 532, as appropriate.
 - Materials: Unless noted otherwise, metal bar grating of the following types shall conform with the following standards:
 - Steel: ASTM A563 (allowable fiber unit stress F = 18,000 psi)
 - Stainless Steel: ASTM A167, alloys 304/316 (allowable fiber unit stress F = 20,000 psi)
 - Aluminum: ASTM B221, alloys 6063-T6/6061-T6 (allowable fiber unit stress F = 12,000 psi)
- Metal bar grating shall be provided with mill finish, unless otherwise noted.
- Unless noted otherwise, provide W-19-4 (1 1/2" x 3/16") Steel grating in locations where metal bar grating is specified.
- All metal bar grating shall be firmly and positively anchored to supporting members. Unless noted otherwise, weld grating to supporting members with 3/16" fillet welds, 3/4" long. Locate welds at each end of bearing bars approximately 6 in from each side of grating panel. At intermediate supports in panel, locate one weld at middle of panel.

VII. Quality Assurance

Quality Assurance Agency Requirements.

- The owner shall engage a qualified Quality Assurance Agency (QAA) to provide all special inspection and quality assurance testing for the project. All quality assurance personnel assigned to the project shall demonstrate competence, to the satisfaction of the building official, for inspection of the particular type of construction or operation requiring special inspection.
- Prior to construction, the QAA shall prepare a written Quality Assurance Implementation Plan (QAIP) for the project. The QAIP shall include a list of personnel assigned to the project including management personnel, inspection procedures and frequency, proposed testing methods and frequency of testing, and reporting procedures. The QAIP shall also outline methods of documenting deficiencies and reporting corrections. A copy of the QAIP shall be given to the contractor for review and coordination with subcontractors.
- Prior to construction, the QAA shall submit the following information to the Architect and Engineer of Record for approval:
 - A copy of the Quality Assurance Implementation Plan for the project.
 - A copy of the appropriate certification and training records for each individual performing inspections or testing.
 - A list of the testing equipment designated for the project and recent calibration records for the equipment.
 - Sample inspection and testing reports and the distribution list for the reports.
- The special inspector shall inspect the work per Chapter 17 of the IBC for conformance with the contract documents. The special inspector shall send reports to the owner, building official, architect, engineer, and contractor. All discrepancies shall be brought to the immediate attention of the contractor for correction. The QAA shall submit a final signed report stating that the special inspection work was, to the best of their knowledge, in conformance with the plans, specifications and applicable workmanship provisions of the IBC.

B. Seismic Force Resisting Systems

- Elements that are a part of the Main Seismic Force Resisting System for the structure may require increased quality assurance inspection and testing. The Main Seismic Force Resisting system for the structure includes the following elements:
 - Masonry walls.
 - Concrete shear walls.
 - Footings and foundation systems that directly support walls referenced above.
 - Braced frames at dropped roof.
 - Roof decking.
 - Floor decking and/or slab systems.
 - All elements labeled as "drag struts" or "chords."
 - Connections between the elements referenced above.

C. Special Inspection: Special inspection shall be provided for the following elements per IBC sections 1704 and 1707:

- Concrete and elements embedded in concrete shall be special inspected prior to and during placement of concrete. Special inspection of concrete shall include the following:
 - Reinforcing steel size and placement.
 - Surface preparation at cold joints including placement of keyways.
 - Soil and embed size, configuration and placement.
 - Concrete shall receive continuous special inspection during placement and periodic inspection after placement to ensure proper curing and weather protection procedures.
- Shotcrete placement shall receive continuous special inspection.
- Structural steel fabrication and erection shall be special inspected, including the following:
 - High strength bolts per IBC 1704.3.3 and IBC section 2209.
 - Filet welds smaller than 5/16" per AWS D1.1.
 - Filet welds larger than 5/16", multiple pass welds, and all groove welds shall receive continuous special inspection during weld placement per AWS D1.1.
 - Welding of reinforcement steel shall receive continuous special inspection during weld placement per AWS D1.1.
 - Welding of Headed Stud Anchors (HSA) and Deformed Bar Anchors (DBA) shall be inspected to comply with AWS D1.1 Section 7.6 through 7.8 and Annex IX.
 - Welding or fastening of floor or roof deck per AWS D1.3 or per the code evaluation report for the fastening method.
- Structural Masonry: Special inspection shall be provided, as follows:
 - As masonry construction begins, and periodically during construction, verify the following are in compliance:
 - Proportions of site-mixed mortar and grout.
 - Placement of masonry units and construction of mortar joints.
 - Locations of reinforcement, connectors and embeds.
 - Protection of masonry during cold or hot weather.
 - Prior to grouting, verify the following are in compliance:
 - Grout space.
 - Size and location of structural elements.
 - Grade, size and placement of reinforcement, connectors and embeds.
 - Construction of all mortar joints.
 - Continuous inspection is required during placement of grout and during preparation of grout specimens, mortar specimens, and/or prisms for testing.
- Post-installed anchors, including but not limited to expansion anchors, adhesive anchors and rebar dowels, and low velocity fasteners, shall receive special inspection per the code evaluation reports for the anchors:
 - Continuous special inspection is required during the installation of all adhesive anchors and rebar dowels. Special inspector shall verify the following:
 - Anchor size and steel grade.
 - Hole diameter, location, and type of drill bit.
 - Cleanliness of hole and anchor.
 - Adhesive application.
 - Anchor embedment.
- Foundation pier system
 - Continuous special inspection is required during drilling operations. The project Geotechnical report and the pier suppliers design documents shall be used to determine compliance. Special inspector shall verify the following:
 - Placement (location)
 - Plumbness
 - Pier diameter
 - Bell diameter (if applicable)
 - Length
 - Embedment into bedrock
 - Adequate end bearing strata capacity.
 - Sprayed fire-resistant materials as per section 1704.10.

D. Structural Testing: The following materials shall be tested per IBC sections 1704 and 1708. The owner reserves the right to test any of all materials using any appropriate non-destructive procedure. Any items found to be deficient shall be corrected and retested at no additional cost to the owner.

- Earthwork: All compacted structural fill shall be tested to verify soil gradation, lift thickness, and compaction requirements. See the specifications and earthwork section of the GSN for testing frequency and acceptability criteria.
- Concrete Strength Verification and Testing: All concrete shall be tested to verify strength, slump, unit weight, air content, and temperature. See the specifications for testing criteria, testing frequency and acceptability criteria.
- Masonry Strength Verification:
 - Masonry strength, fm shall be verified using the "Unit Strength Method" per IBC Section 2105.2.2.1 and as follows: Prior to construction, the masonry units and grout shall be tested for compressive strength. Certificates of compliance with strength requirements of the masonry units and grout shall be submitted by the masonry and grout suppliers. During construction, the masonry units and grout shall be tested for every 5,000 square feet of masonry constructed per GSN IV.A, above. Proportions of materials in mortar and grout delivered to the site shall be verified.
 - The contractor has the option of using the "Masonry Prism Test Method" per IBC Section 2105.2.2.2, in lieu of the "Unit Strength Method."
- Welded connections shall be tested for compliance according to IBC Table 1704.3, AWS D1.1 and the contract specifications and plans. As a minimum, the testing shall include the following:
 - All complete penetration groove welds shall be tested 100 percent either by ultrasonic testing or by radiography.
 - Partial penetration groove welds shall be tested either by ultrasonic testing or radiography. A minimum of 50% of these welds shall be tested.
 - Base metal thicker than 1 1/2", when subjected to through thickness weld shrinkage strains, shall be ultrasonically inspected for discontinuities directly behind such welds and three inches above and below the weld after joint assembly completion.
 - Any material discontinuities shall be accepted or rejected on the basis of the defect rating in accordance with the testing in AWS D1.1 Chapter 6, excluding Sections 6.1 through and including 6.6. All deficient welds shall be corrected and retested at no additional cost to the owner.
- Post-installed anchors, including but not limited to expansion anchors, adhesive anchors, and low velocity fasteners, shall be tested per the code evaluation reports for the anchors.

E. Structural Observations by the Engineer of Record

- The Engineer of Record shall perform structural observations at critical phases of the project. Copies of the engineer's report will be distributed to the architect, contractor, owner, building official, and QAA.
- Observation visits to the site by the Engineer's field representatives shall not be construed as inspection or approval of construction.
- Notification of Engineer: The contractor shall notify the engineer twenty-four hours prior to:
 - Placing concrete in any footing.
 - Closing any wall forms.
 - Placing concrete in any column, beam or suspended slab.
 - Grouting any masonry.
 - Completing the welding of major sections of steel decking.

F. Contractor Responsibility: The contractor shall prepare and submit a written statement of responsibility to the building official and the owner prior to commencement of work on the project. As a minimum the statement shall contain the following information:

- Acknowledgement of the quality assurance requirements for the structure.
- Acknowledgement of receipt of the Quality Assurance Implementation Plan (QAIP) from the testing agency.
- Acknowledgement that control will be exercised to obtain conformance to the Contract Documents and the QAIP.
- Quality control procedures within the contractor's organization, methods and frequency of reporting, and distribution of the reports.
- Identification and qualifications of the person(s) responsible for quality control and their position(s) in the organization.

ABBREVIATIONS

AB	ANCHOR BOLT (S)
ABV	ABOVE
ALT	ALTERNATE
APPROX	APPROXIMATE
ARCH	ARCHITECT(U/R/L)
BM	BEAM
BLW	BELOW
BRG	BEARING
BTWN	BETWEEN
BLDG	BUILDING
BOT	BOTTOM
C.J.	CONSTRUCTION JOINT OR CONTROL JOINT
CJP	COMPLETE JOINT PENETRATION
CMU	CONC MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONST	CONSTRUCTION
CONT	CONTINUOUS
CONTR	CONTRACTOR
CTR	CENTER
DB	DECK BEARING
DBA	DEFORMED BAR ANCHORS
DBL	DOUBLE
DET	DETAIL
DIA (OR)	DIAMETER
DIM	DIMENSION
DN	DOWN
DWG	DRAWING
DWL	DOWEL
EA	EACH
E.J.	EXPANSION JOINT (SEISMIC SEPARATION JOINT)
ELEV	ELEVATION
ELEC	ELECTRICAL
EQUIP	EQUIPMENT
EQU	EQUAL
EXIST	EXISTING
EXP	EXPRESSION / EXPOSED
EXT	EXTERIOR
E.F.	EACH FACE
E.W.	EACH WAY
F.D	FLOOR DRAIN
FOUN	FOUNDATION
F.F.	FINISH FLOOR
FIN	FINISH
FL	FLOOR
FT	FOOT
FTV	FOOTING
FTG	FIELD VERIFY
GA	GAUGE
GALV	GALVANIZED
GLB	GLULAMINATED BEAM
GRD	GRADE
GSN	GENERAL STRUCTURAL NOTES
HB	HORIZONTAL BRIDGING
HT	HEIGHT
HORIZ	HORIZONTAL
HSA	HEADED STUD ANCHORS
IBC	INTERNATIONAL BUILDING CODE
ICBO	INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS
ICC	INTERNATIONAL CODE COUNCIL
IN	INCH
INSUL	INSULATION
INT	INTERIOR
I.F.	INSIDE FACE
JT	JOINT
JOIST	JOIST
KIPS	KIPS PER LINEAL FOOT
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
K	KIPS - 1,000 POUNDS
LF	LINEAL FOOT
LBS	POUNDS
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
MAS	MASONRY
MAX	MAXIMUM
MECH	MASONRY C.J.
MECH	MECHANICAL
MFR	MANUFACTURER
MIN	MINIMUM
MISC	MISCELLANEOUS
NIC	NOT IN CONTRACT
NTS	NOT TO SCALE
OPNG	OPENING
OPP	OPPOSITE
O.C.	ON CENTER
O.F.	OUTSIDE FACE
OWSJ	OPEN WEB STEEL JOIST
PCF	POUNDS/CUBIC FOOT
PL	PLATE
PLF	POUNDS/LINEAL FOOT
PNL	PANEL
PSF	POUNDS/SQ FOOT
PSI	POUNDS/SQ INCH
PT	POINT
REINF	REINFORCING
R.D.	ROOF DRAIN
REQD	REQUIRED
SHT	SHEET
SI	SPECIAL INSPECTION
SOG	SLAB ON GRADE
STD	STANDARD
STIFF	STIFFENER
STL	STEEL
SQ	SQUARE
SIM	SIMILAR
STR	STRUCTURAL
STAG	STAGGERED
T&B	TOP AND BOTTOM
TEMP	TEMPERATURE
THDS	THREADS
TOP OF	TOP OF
TOC	TOP OF CONCRETE
TOF	TOP OF FOOTING
TOS	TOP OF SLAB
TOW	TOP OF WALL
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W	WITH
WWF	WELDED WIRE FABRIC

PLAN MARKS

BF-#	BRACED FRAME
CB-#	CONCRETE BEAM
CC-#	CONCRETE COLUMN
CDP-#	CONC DRILLED PIER
CFW-#	CONC FDTN. WALL
CGB-#	CONC GRADE BEAM
CJ-#	CONCRETE JOIST
CL-#	CONCRETE LINTEL
CP-#	CONCRETE PIER
CRW-#	CONC RETAINING WALL
CSG-#	CONC SLAB ON GRADE
CSS-#	CONC SUSPENDED SLAB
CSW-#	CONC SHEAR WALL
CW-#	CONCRETE WALL
FC#	CONTINUOUS FOOTING
FM#	MAT FOOTING
FR#	RECTANGULAR FOOTING
FS#	SQUARE FOOTING
FTS#	THICKEND SLAB FOOTING
MC-#	MASONRY COLUMN
MF-#	MOMENT FRAME
ML-#	MASONRY LINTEL
MP-#	MASONRY PIER
MW-#	MASONRY WALL
PD-#	PLYWOOD DIAPHRAGM
PSW-#	PLYWOOD SHEAR WALL
SBP-#	STEEL BASE PLATE
SC-#	STEEL COLUMN
SCP-#	STEEL CAP PLATE
SD-#	STEEL DECK

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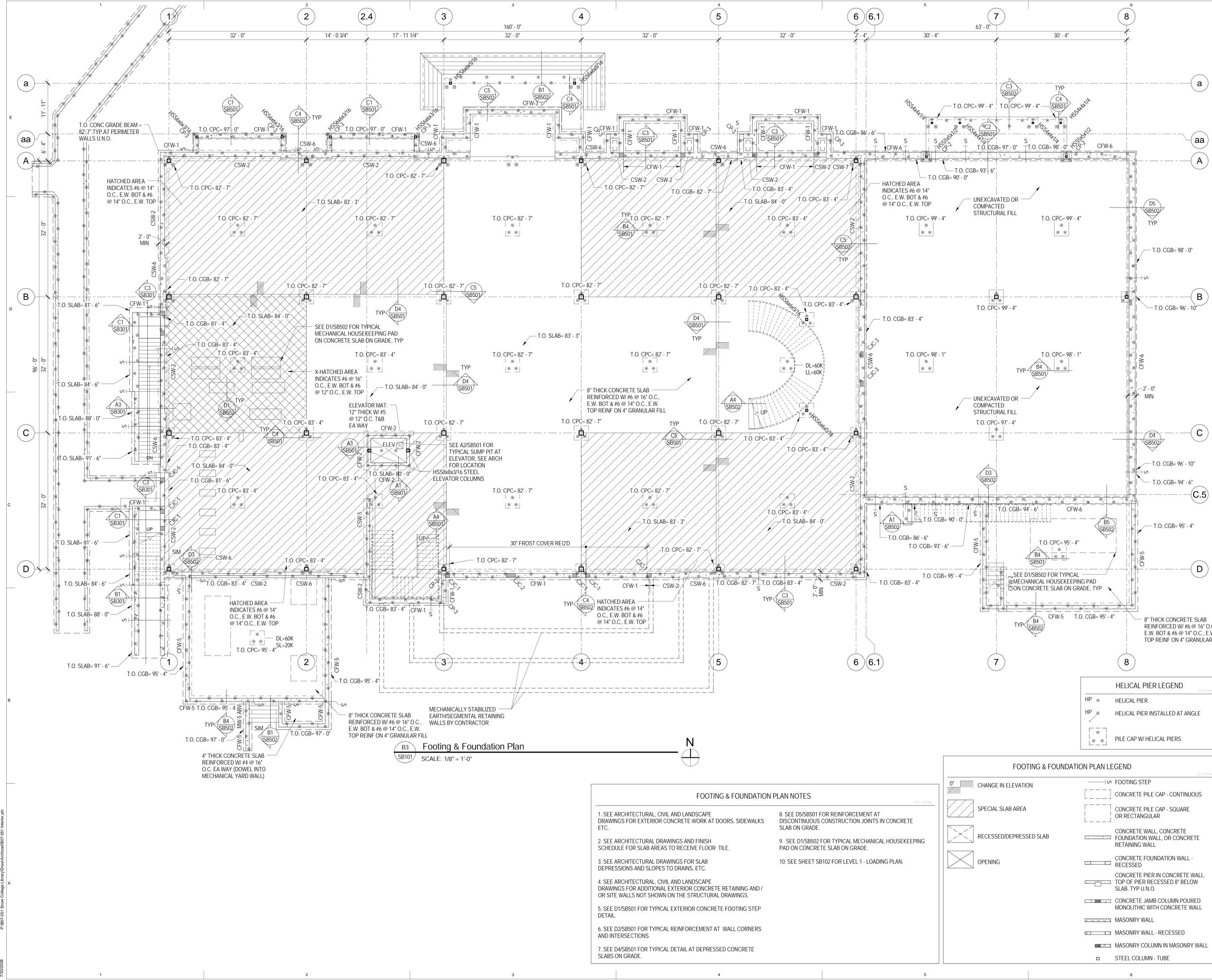
07258700

SHEET TITLE:

Basement - Footing & Foundation Plan

SHEET NUMBER:

SB101



B3 Footing & Foundation Plan
SCALE: 1/8" = 1'-0"

FOOTING & FOUNDATION PLAN NOTES

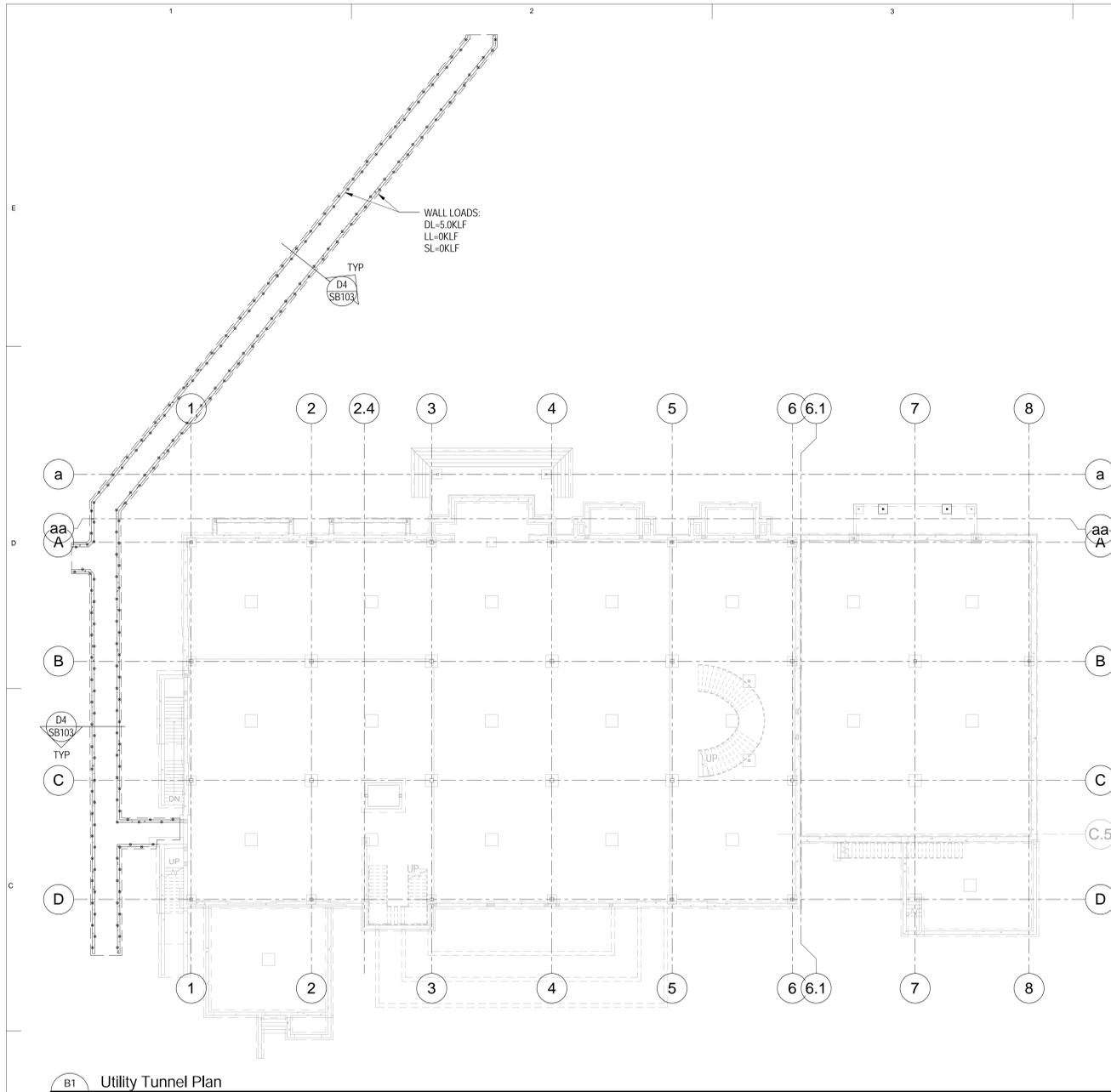
1. SEE ARCHITECTURAL, CIVIL AND LANDSCAPE DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS ETC.
2. SEE ARCHITECTURAL DRAWINGS AND FINISH SCHEDULE FOR SLAB AREAS TO RECEIVE FLOOR TILE.
3. SEE ARCHITECTURAL DRAWINGS FOR SLAB DEPRESSIONS AND SLOPES TO DRAINS, ETC.
4. SEE ARCHITECTURAL, CIVIL AND LANDSCAPE DRAWINGS FOR ADDITIONAL EXTERIOR CONCRETE RETAINING AND/OR SITE WALLS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
5. SEE D1/SB501 FOR TYPICAL EXTERIOR CONCRETE FOOTING STEP DETAIL.
6. SEE D2/SB501 FOR TYPICAL REINFORCEMENT AT WALL CORNERS AND INTERSECTIONS.
7. SEE D4/SB501 FOR TYPICAL DETAIL AT DEPRESSED CONCRETE SLABS ON GRADE.
8. SEE D5/SB501 FOR REINFORCEMENT AT DISCONTINUOUS CONSTRUCTION JOINTS IN CONCRETE SLAB ON GRADE.
9. SEE D1/SB502 FOR TYPICAL MECHANICAL HOUSEKEEPING PAD ON CONCRETE SLAB ON GRADE.
10. SEE SHEET SB102 FOR LEVEL 1 - LOADING PLAN.

HELICAL PIER LEGEND

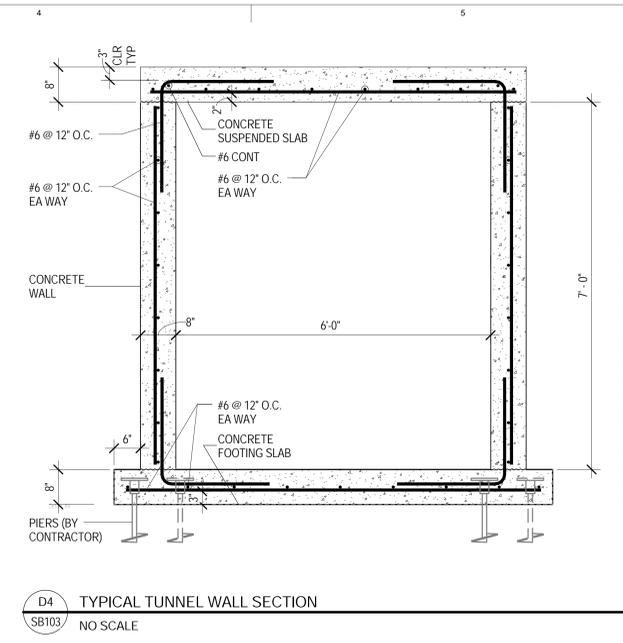
HP	HELICAL PIER
HP	HELICAL PIER INSTALLED AT ANGLE
	PILE CAP W/ HELICAL PIERS

FOOTING & FOUNDATION PLAN LEGEND

[Symbol]	CHANGE IN ELEVATION	[Symbol]	FOOTING STEP
[Symbol]	SPECIAL SLAB AREA	[Symbol]	CONCRETE PILE CAP - CONTINUOUS
[Symbol]	RECESSED/DEPRESSED SLAB	[Symbol]	CONCRETE PILE CAP - SQUARE OR RECTANGULAR
[Symbol]	OPENING	[Symbol]	CONCRETE WALL, CONCRETE FOUNDATION WALL, OR CONCRETE RETAINING WALL
		[Symbol]	CONCRETE FOUNDATION WALL - RECESSED
		[Symbol]	CONCRETE PIER IN CONCRETE WALL, TOP OF PIER RECESSED 8" BELOW SLAB, TYP U.N.O.
		[Symbol]	CONCRETE JAMB COLUMN POURED MONOLITHIC WITH CONCRETE WALL
		[Symbol]	MASONRY WALL
		[Symbol]	MASONRY WALL - RECESSED
		[Symbol]	MASONRY COLUMN IN MASONRY WALL
		[Symbol]	STEEL COLUMN - TUBE



B1 Utility Tunnel Plan
SB103 SCALE: 1/16" = 1'-0"



D4 TYPICAL TUNNEL WALL SECTION
SB103 NO SCALE

- FOOTING & FOUNDATION PLAN NOTES**
1. SEE SB102 FOR PIER & FOUNDATION NOTES.
 2. SEE ARCHITECTURAL & CIVIL FOR DIMENSIONS & LOCATION OF TUNNEL.
 3. SEE SB101 FOR FOOTING & FOUNDATION PLAN NOTES.

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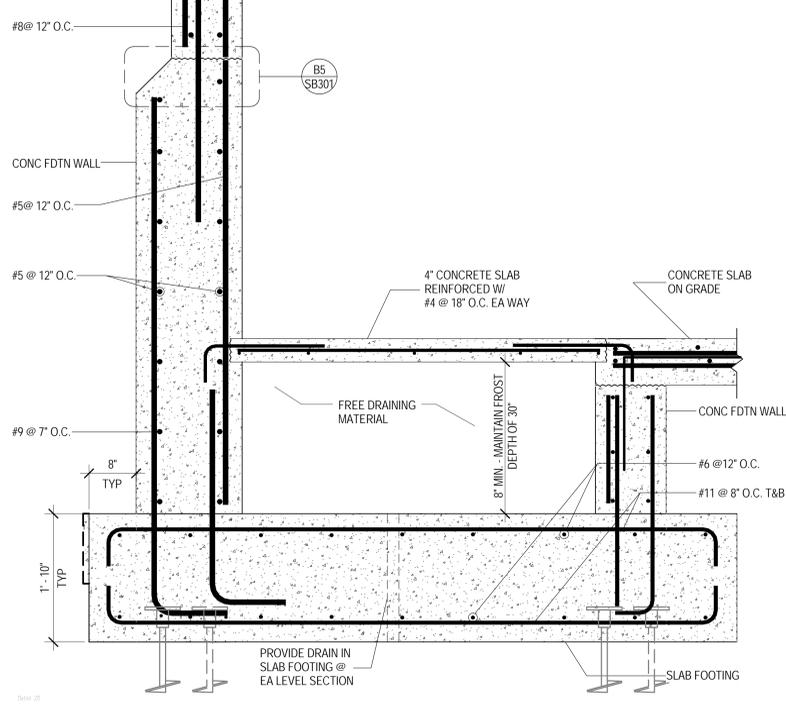
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ISSUE DATE:
SEPTEMBER 15 2008, 100% CD

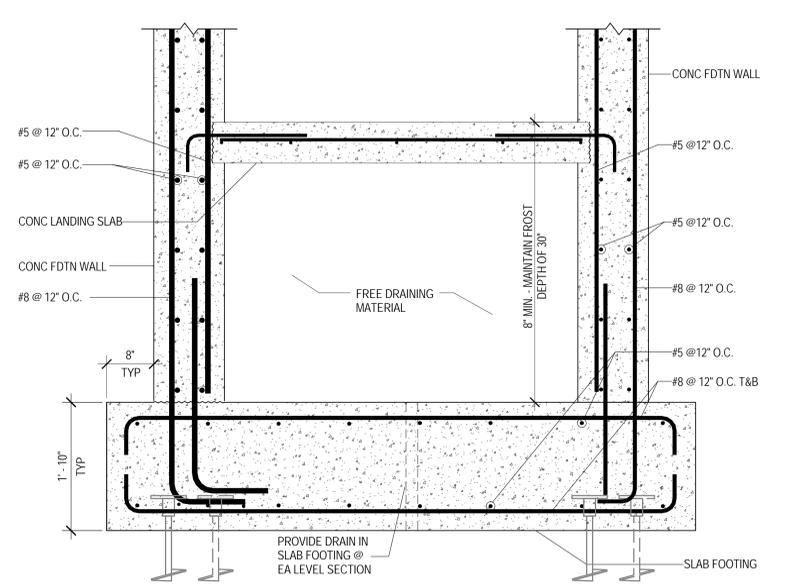
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B07-051
DFCM PROJECT NUMBER:
07258700
SHEET TITLE:

Stair Sections

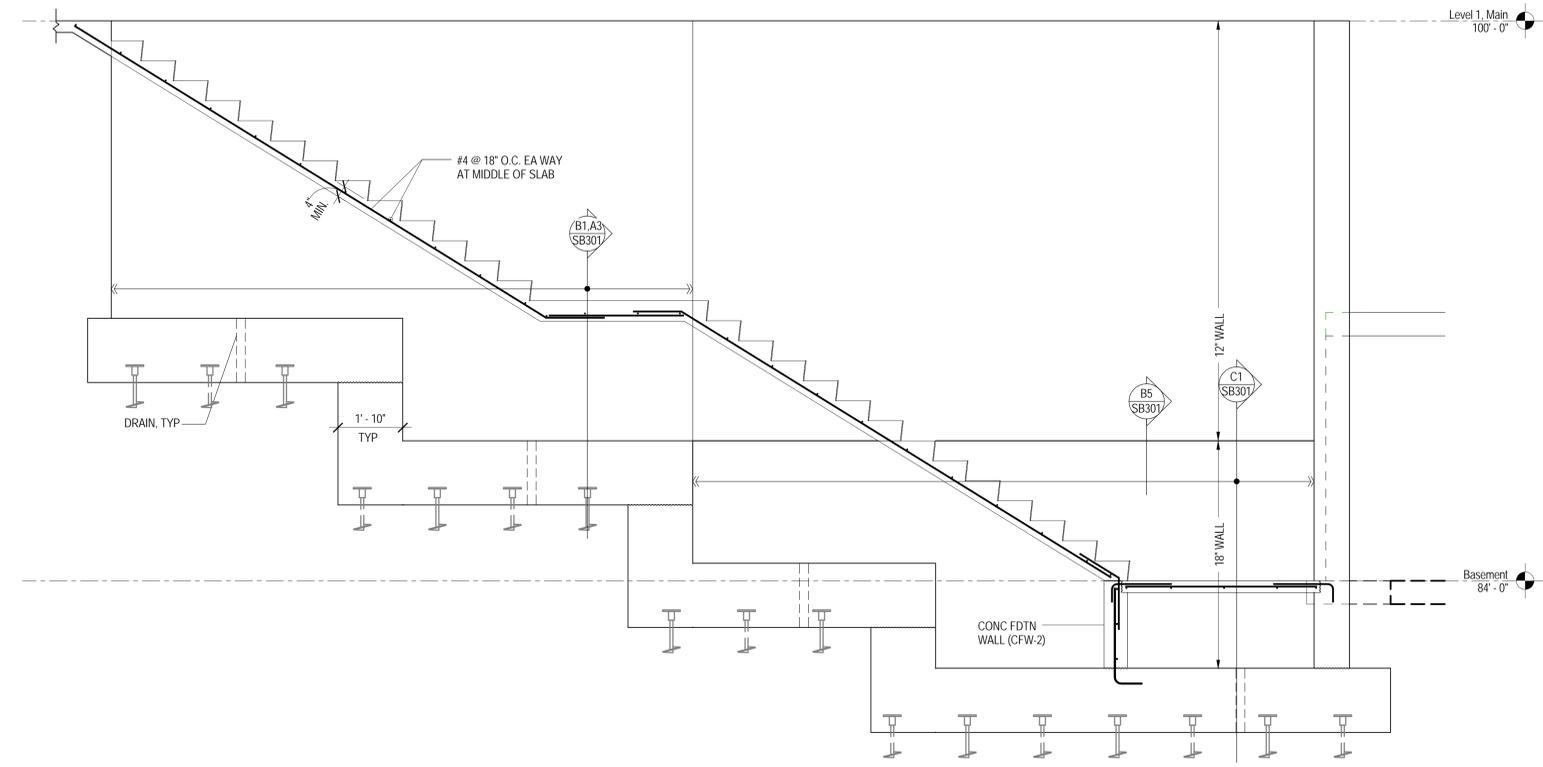
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SB301



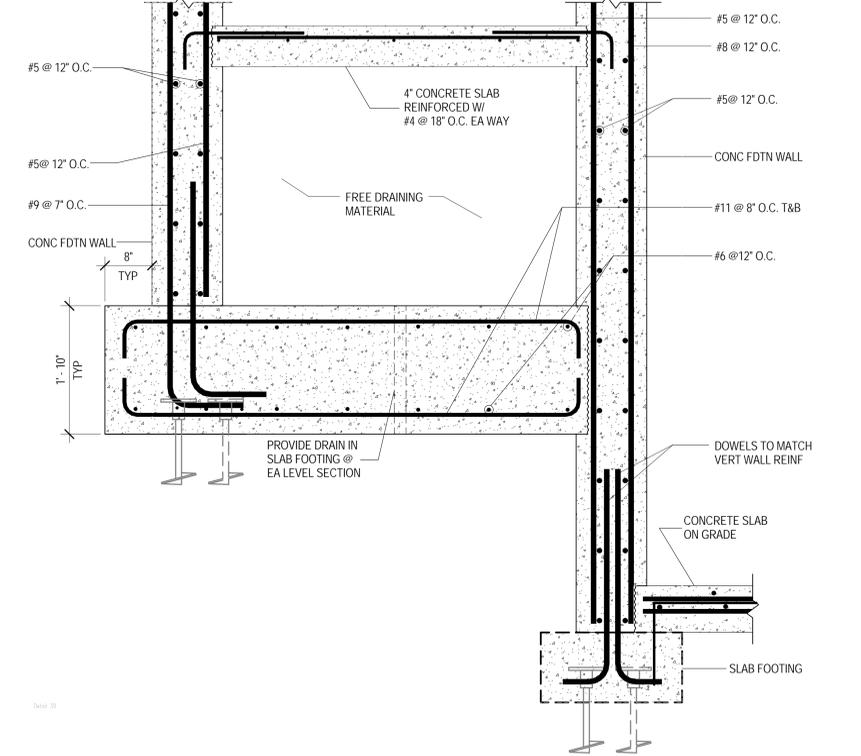
B5
SB301
STAIR SECTION
NO SCALE



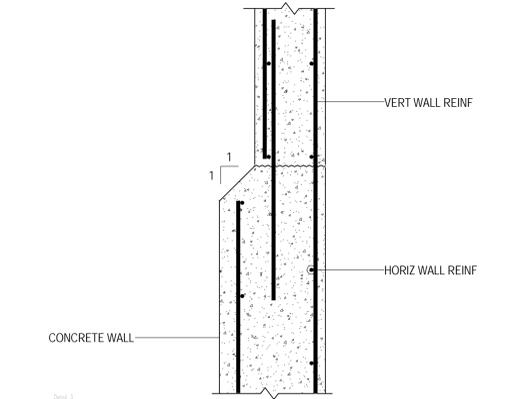
B1
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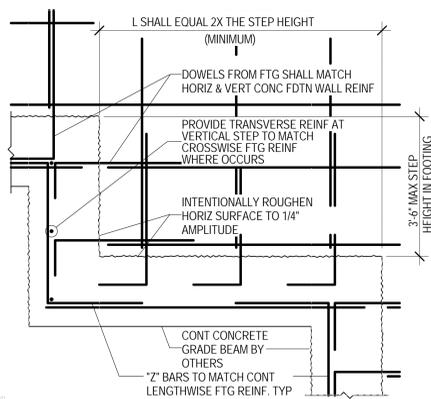
C3
SB301
TYPICAL STAIR SECTION
NO SCALE



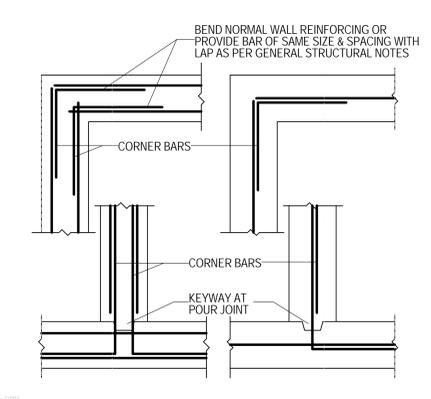
A3
SB301
STAIR SECTION
NO SCALE



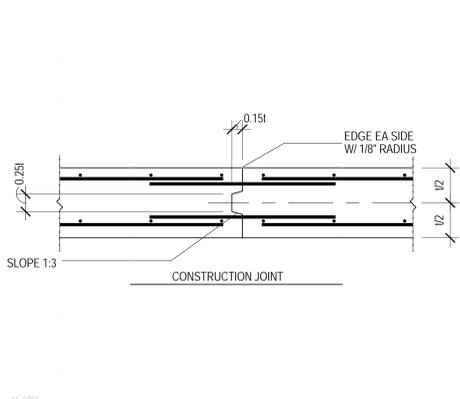
B5
SB301
**TYPICAL EXTERIOR WALL THICKNESS
TRANSITION AT STAIRS**
NO SCALE



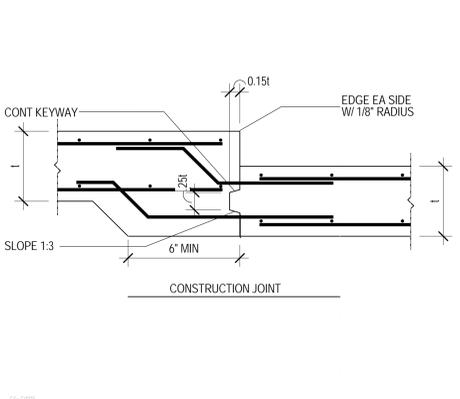
D1 TYPICAL STEP CONCRETE WALL STEP DETAIL
SB501 NO SCALE



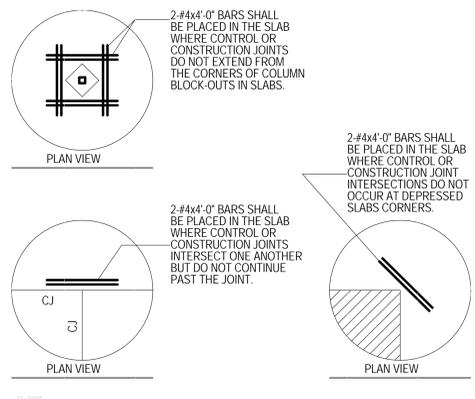
D2 PLAN VIEW - TYPICAL WALL CORNERS & INTERSECTION
SB501 NO SCALE



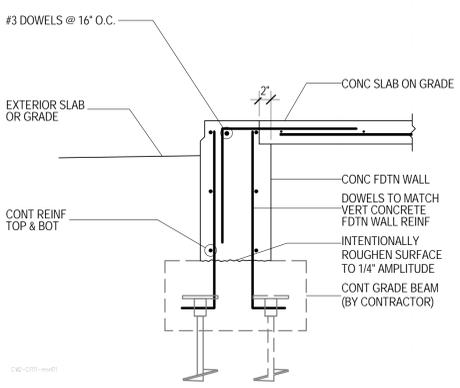
D3 TYPICAL SLAB JOINTS
SB501 NO SCALE



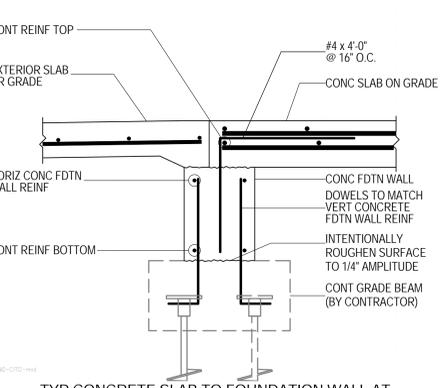
D4 TYP SLAB JOINTS AT DEPRESSED SLAB
SB501 NO SCALE



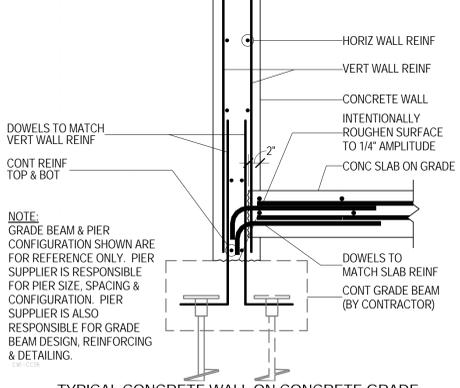
D5 TYPICAL SLAB REINF AT DISCONTINUOUS SLAB JOINTS
SB501 NO SCALE



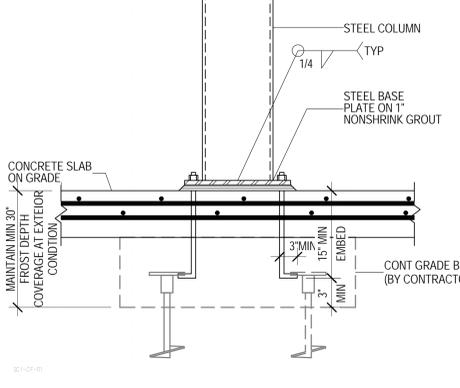
C1 TYP EXTERIOR CONCRETE FOUNDATION WALL
SB501 NO SCALE



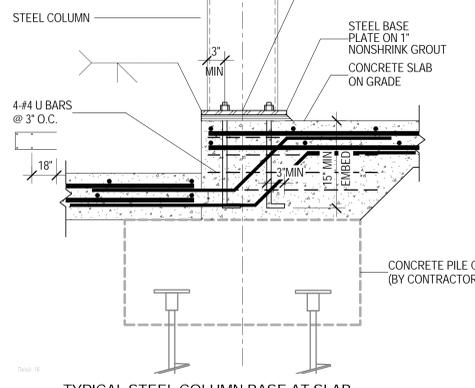
C2 TYP CONCRETE SLAB TO FOUNDATION WALL AT DOOR OPENING
SB501 NO SCALE



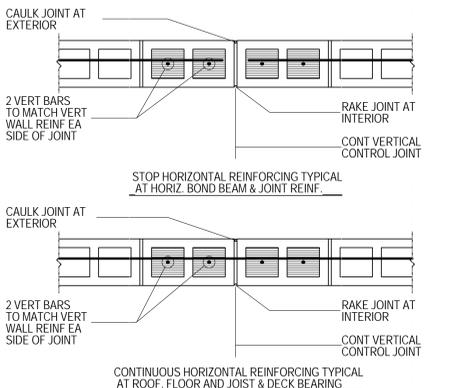
C3 TYPICAL CONCRETE WALL ON CONCRETE GRADE BEAM
SB501 NO SCALE



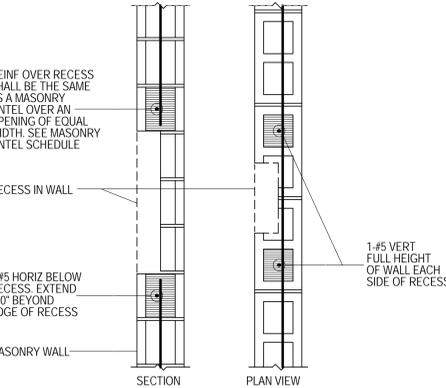
C4 TYPICAL STEEL COLUMN BASE
SB501 NO SCALE



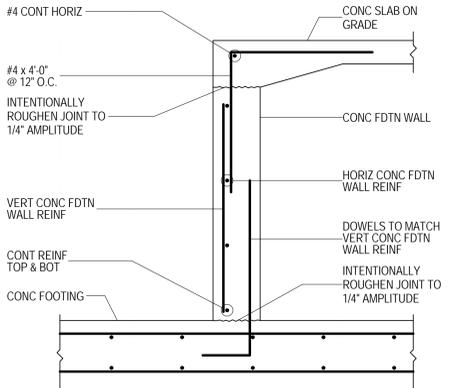
C5 TYPICAL STEEL COLUMN BASE AT SLAB TRANSITION
SB501 NO SCALE



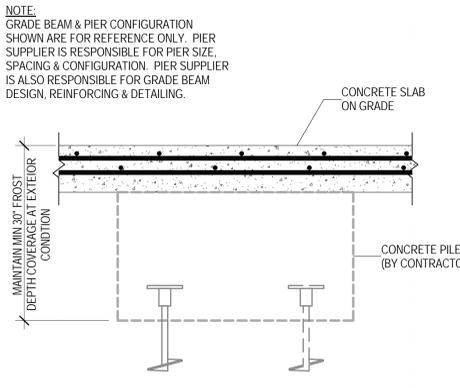
B1 TYPICAL CONTROL JOINTS IN 10\"/>



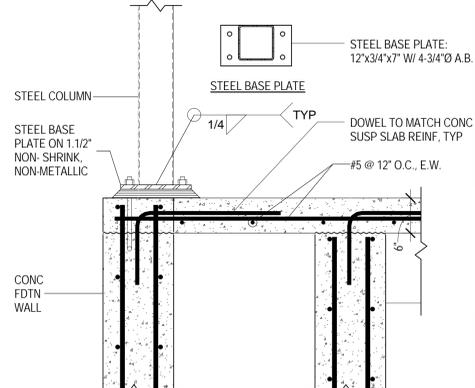
B2 TYPICAL RECESS IN 10\"/>



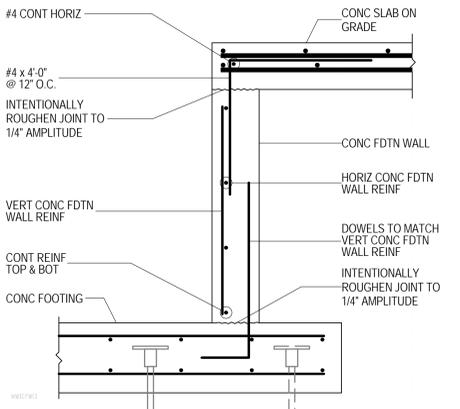
B3 CONCRETE WALL TO CONCRETE SLAB FOOTING
SB501 NO SCALE



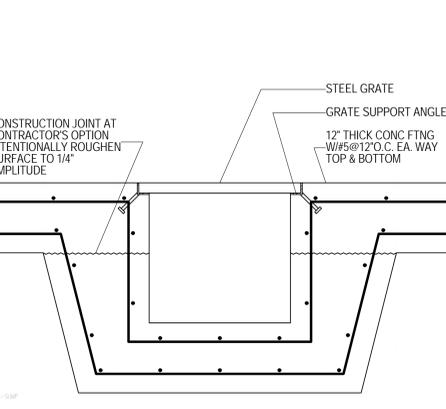
B4 TYPICAL CONCRETE SLAB ON GRADE TO CONCRETE PILE CAP
SB501 NO SCALE



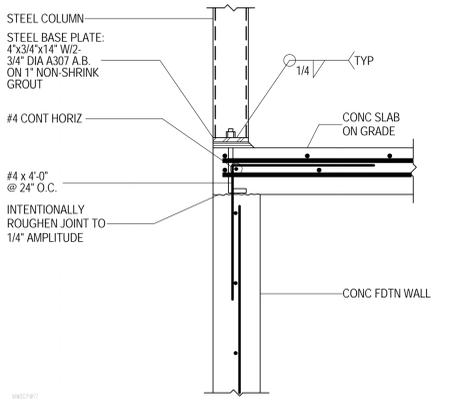
B5 STEEL COLUMN BASE AT CONC FDTN WALL
SB501 NO SCALE



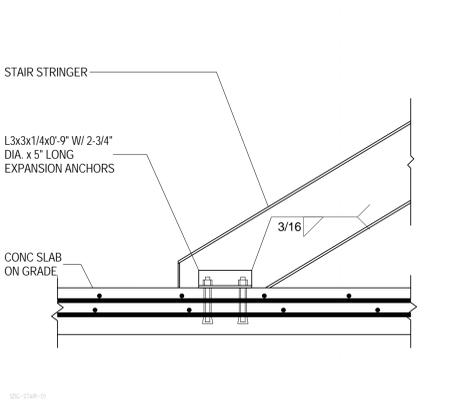
A1 TYPICAL ELEVATOR PIT DETAIL
SB501 NO SCALE



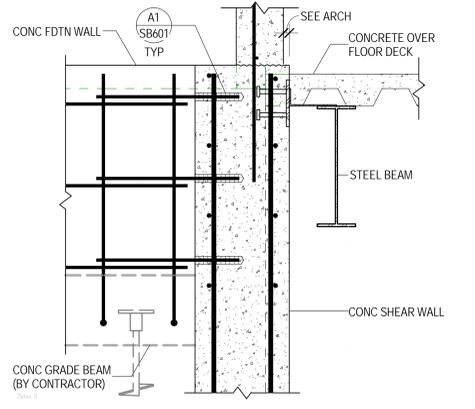
A2 TYPICAL SUMP PIT AT ELEVATOR
SB501 NO SCALE



A3 TYP ELEVATOR GUIDE RAIL SUPPORT COLUMN BEARING ON CONC FDTN WALL
SB501 NO SCALE



A4 TYPICAL STAIR STRINGER CONNECTION TO CONCRETE SLAB ON GRADE
SB501 NO SCALE



A5 CONCRETE FOUNDATION WALL TO CONCRETE FOUNDATION WALL
SB501 NO SCALE

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Ogden, UT 84403
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Contact: Andrea Michaels
4809 North 73rd Street, Suite 100
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(480) 998-4776 FAX (480) 998-9390
andrea@madcinc.com

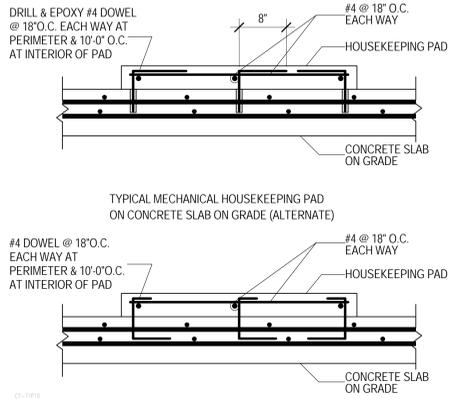


PROJECT NAME:
Snow College Library
150 College Avenue
Ephraim, Utah 84627

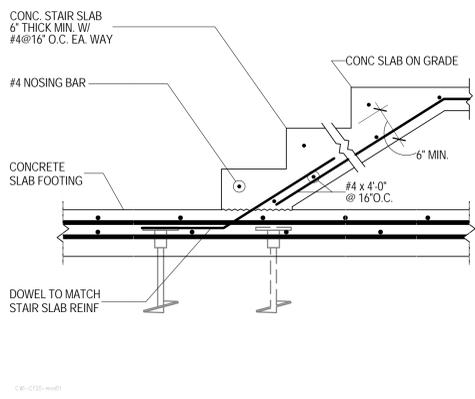
REVISIONS:

100% CD September 15, 2008
100% CD Review, August 4, 2008
ISSUE DATE:
SEPTEMBER 15 2008, 100% CD
ARCHITECT'S PROJECT NUMBER:
B07-051
DFCM PROJECT NUMBER:
07258700
SHEET TITLE:

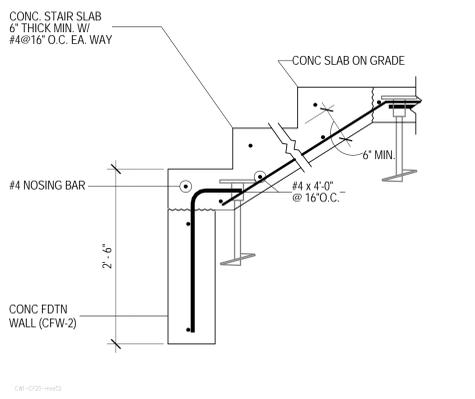
Typical Footing &
Foundation Details
SHEET NUMBER:
SB501



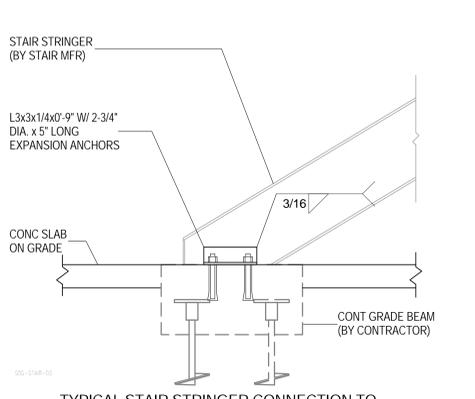
D1
TYPICAL MECHANICAL HOUSEKEEPING PAD ON CONCRETE SLAB ON GRADE
NO SCALE



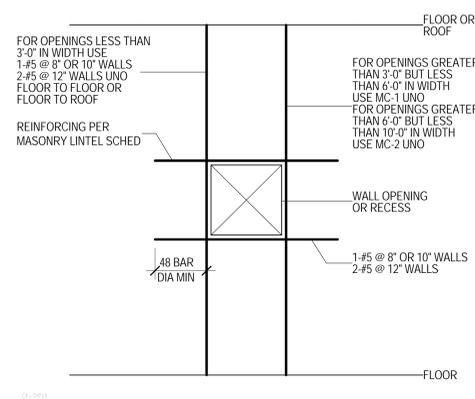
C1
TYPICAL CONCRETE STAIR SLAB ON GRADE
NO SCALE



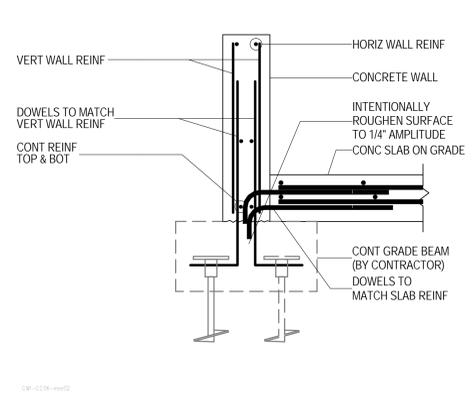
B1
TYPICAL CONCRETE STAIR SLAB ON GRADE
NO SCALE



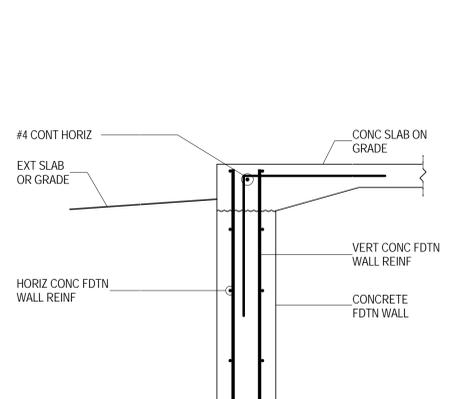
A1
TYPICAL STAIR STRINGER CONNECTION TO CONCRETE SLAB ON GRADE
NO SCALE



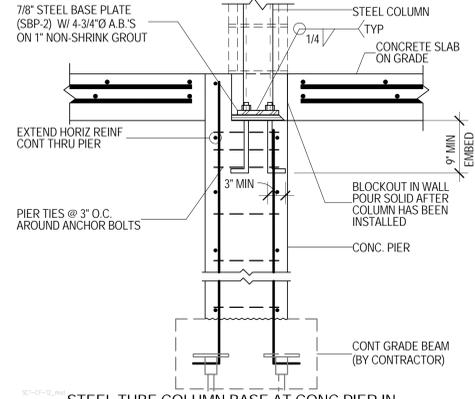
D2
TYP REINF AROUND MISC OR RECESSED MASONRY OPENINGS
NO SCALE



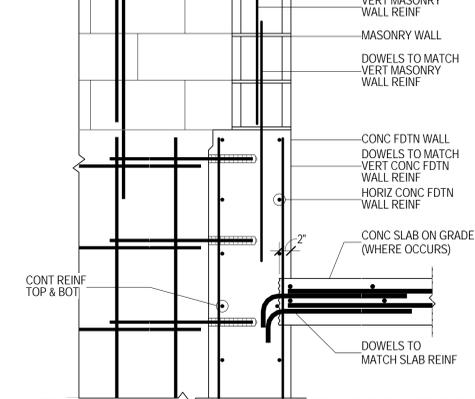
C2
TYPICAL CONCRETE WALL AT WINDOW ON CONCRETE GRADE BEAM
NO SCALE



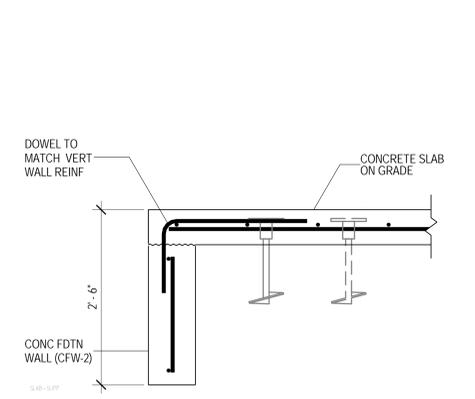
B2
CONC FDTN WALL TO CONC FOOTING AT MECHANICAL YARD
NO SCALE



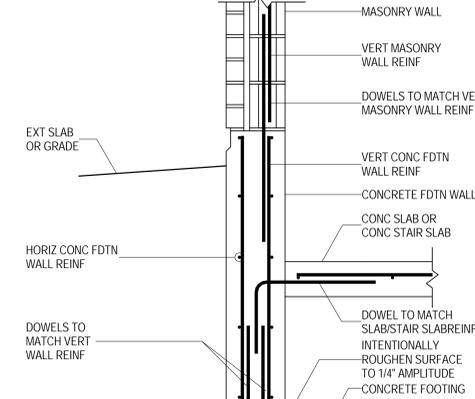
A2
STEEL TUBE COLUMN BASE AT CONC PIER IN FOUNDATION WALL
NO SCALE



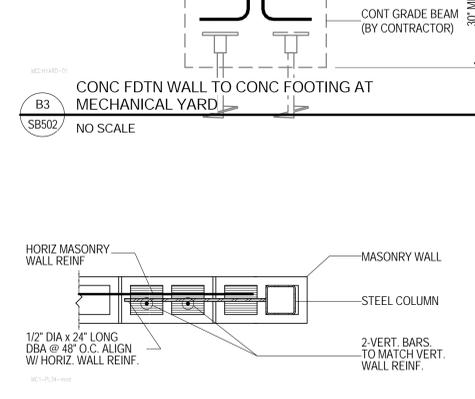
D3
TYP MASONRY WALL W/ VENEER ON CONCRETE GRADE BEAM
NO SCALE



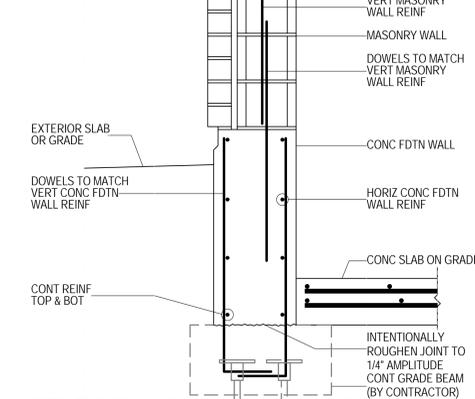
C3
TYPICAL SLAB EDGE SUPPORT
NO SCALE



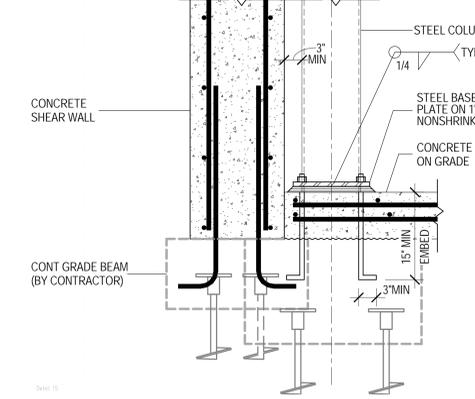
B3
CONC FDTN WALL TO CONC FOOTING AT MECHANICAL YARD
NO SCALE



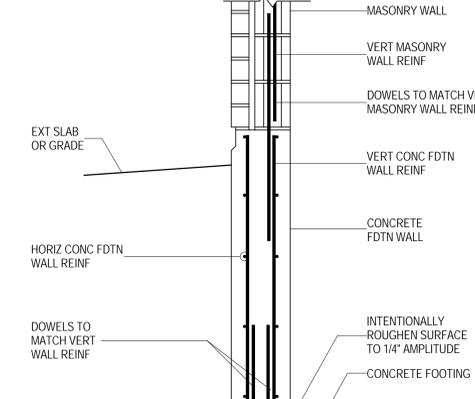
A3
TYPICAL STEEL COLUMN IN MASONRY WALL
NO SCALE



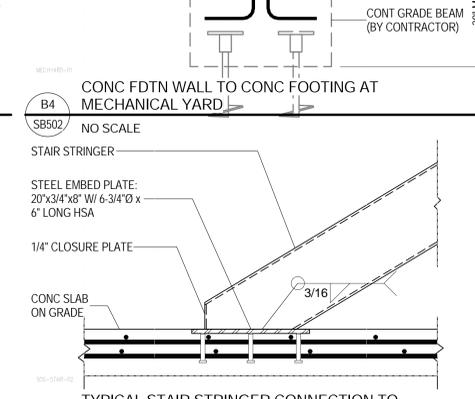
D4
TYP MASONRY WALL W/ VENEER ON CONCRETE GRADE BEAM
NO SCALE



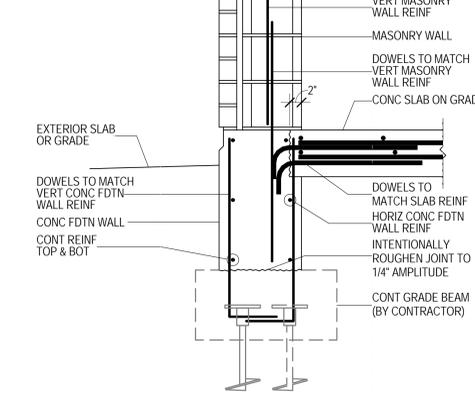
C4
STEEL COLUMN BASE AT CONC FDTN/SHEAR WALL
NO SCALE



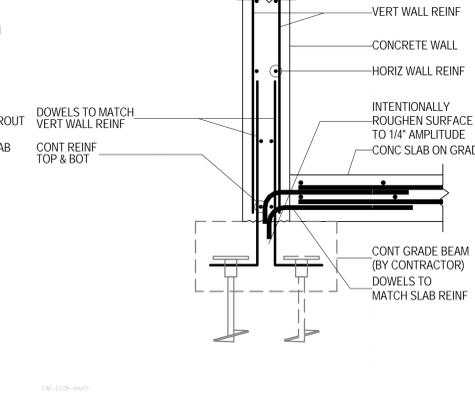
B4
CONC FDTN WALL TO CONC FOOTING AT MECHANICAL YARD
NO SCALE



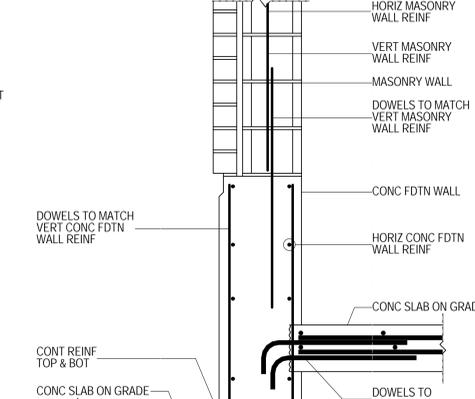
A4
TYPICAL STAIR STRINGER CONNECTION TO CONCRETE SLAB ON GRADE
NO SCALE



D5
TYP MASONRY WALL W/ VENEER ON CONCRETE GRADE BEAM
NO SCALE



C5
TYPICAL CONCRETE WALL ON CONCRETE GRADE BEAM
NO SCALE



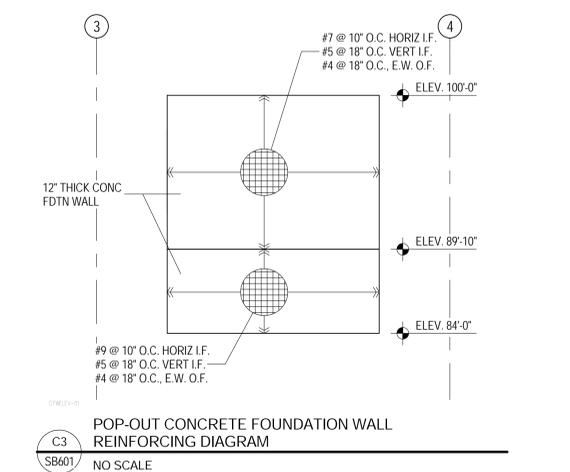
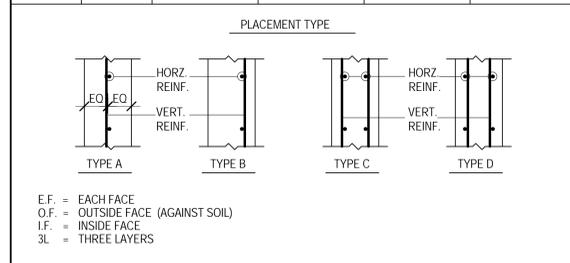
B5
MASONRY WALL W/ VENEER ON CONCRETE GRADE BEAM AT MECHANICAL YARD
NO SCALE

CONCRETE LINTEL SCHEDULE					
MARK	THICK	DEPTH	REINFORCING		REMARKS
			HORIZONTAL	STIRRUPS	
CL-1	12"	16"	2-#6 T&B	#3 @ 8" O.C.	
CL-2	12"	18"	2-#7 T&B	#3 @ 8" O.C.	
CL-3	12"	24"	2-#8 T&B	#3 @ 8" O.C.	

CONCRETE JAMB COLUMN SCHEDULE				
MARK	SIZE	REINFORCING		REMARKS
		VERTICAL	TIES	
CJC-1	12" x 12"	4-#7	#3 @ 8" O.C.	
CJC-2	12" x 24"	6-#7	#3 @ 8" O.C.	

CONCRETE PIER SCHEDULE				
MARK	SIZE	REINFORCING		REMARKS
		VERTICAL	TIES	
CP-1	8" x 16"	4-#5	#3 @ 8" O.C.	
CP-2	14" x 18"	4-#8	#4 @ 10" O.C.	
CP-3	12" x 16"	4-#7	#3 @ 8" O.C.	

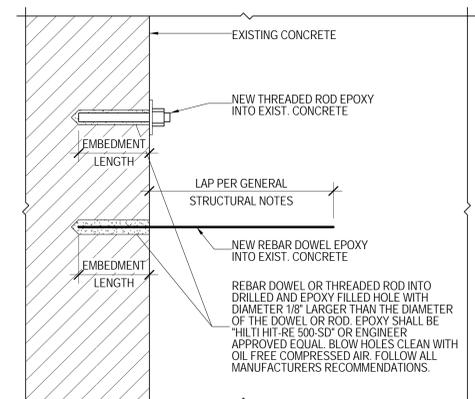
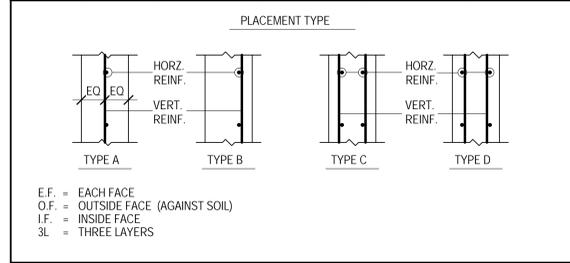
CONCRETE FOUNDATION WALL SCHEDULE					
MARK	THICK	HORIZONTAL REINFORCING	VERTICAL REINFORCING	TOP & BOTTOM HORIZONTAL BARS	PLACEMENT
CFW-1	12"	#6 @ 12" O.C., E.F.	#6 @ 12" O.C., E.F.	2-#7	TYPE D
CFW-2	8"	#5 @ 12" O.C.	#5 @ 12" O.C.	1-#7	TYPE A
CFW-3	12"	SEE C3/SB601	SEE C3/SB601	SEE C3/SB601	SEE C3/SB601
CFW-4	14"	#6 @ 12" O.C., E.F.	#6 @ 12" O.C., E.F.	2-#7	TYPE C
CFW-5	10"	#4 @ 12" O.C., E.F.	#4 @ 12" O.C., E.F.	2-#6	TYPE C
CFW-6	17"	#6 @ 12" O.C., E.F.	#6 @ 12" O.C., E.F.	2-#7	TYPE D



CONCRETE REINFORCING BAR LAP SPLICE SCHEDULE												
BAR SIZE Fy = 60 KSI	TENSION BARS										COMP. BARS	
	f _c = 3000 PSI		f _c = 4000 PSI		f _c = 5000 PSI		f _c = 6000 PSI		f _c = ALL		f _c = ALL	
	REGULAR	TOP	REGULAR	TOP	REGULAR	TOP	REGULAR	TOP	REGULAR	TOP		
	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS		
#3	A	B	A	B	A	B	A	B	A	B	12"	
#4	22"	29"	29"	38"	19"	25"	25"	33"	17"	23"	23"	15"
#5	28"	36"	36"	47"	24"	31"	31"	41"	22"	28"	28"	19"
#6	33"	43"	43"	56"	29"	37"	37"	49"	26"	34"	34"	23"
#7	48"	63"	63"	81"	42"	54"	54"	71"	38"	49"	49"	27"
#8	55"	72"	72"	93"	48"	62"	62"	81"	43"	56"	56"	30"
#9	62"	81"	81"	105"	54"	70"	70"	91"	48"	63"	63"	34"
#10	70"	91"	91"	118"	61"	79"	79"	102"	54"	71"	71"	39"
#11	78"	101"	101"	131"	67"	87"	87"	114"	60"	78"	78"	43"

- NOTES: THESE NOTES SHALL BE USED FOR ALL SPLICES, UNLESS NOTED OTHERWISE ON DRAWINGS.
- TOP BARS ARE HORIZONTAL BARS, SPLICED SO THAT 12" OR MORE OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE REINFORCING BAR.
 - CLASS A SPLICES MAY BE USED ONLY WHEN 50% OR LESS OF THE BARS ARE SPLICED WITHIN THE LAP SPLICE LENGTH.
 - CLASS B SPLICES SHALL BE USED FOR ALL SPLICES IN SLABS, BEAMS, JOISTS, WALLS, MOMENT RESISTING COLUMNS, AND JAMB COLUMNS, UNLESS THEY MEET THE REQUIREMENTS OF NOTE #2 ABOVE.
 - TIES AND STIRRUPS SHALL NOT BE SPLICED.
 - A. FOR BUNDLED BARS OF THREE OR LESS, LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.2.
B. FOR BUNDLED BARS OF FOUR OR MORE, LAP LENGTHS SHALL BE MULTIPLIED BY 1.33.
C. INDIVIDUAL BAR SPLICES WITHIN A BUNDLE SHALL NOT OVERLAP. ENTIRE BUNDLES SHALL NOT BE LAP SPLICED.
 - FOR ALL LIGHTWEIGHT CONCRETE, LAP LENGTHS SHALL BE MULTIPLIED BY 1.3.
 - FOR ALL EPOXY COATED BARS WITH COVER LESS THAN 3 BAR DIAMETERS OF CLEAR SPACING LESS THAN 6 BAR DIAMETERS THE LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.5. FOR ALL OTHER EPOXY BARS THE SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.2.
 - THE BAR LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.5 WHEN EITHER OF THE FOLLOWING IS TRUE:
A. CLEAR SPACING OF BARS BEING DEVELOPED IS LESS THAN ONE BAR DIAMETER, CLEAR COVER IS LESS THAN ONE BAR DIAMETER AND STIRRUPS OR TIES ALONG THE LENGTH OF THE SPLICE ARE LESS THAN THE CODE MINIMUM.
B. CLEAR SPACING OF BARS BEING DEVELOPED IS LESS THAN 2 BAR DIAMETERS AND CLEAR COVER IS LESS THAN ONE BAR DIAMETER.

CONCRETE SHEAR WALL SCHEDULE					
MARK	THICK	HORIZONTAL REINFORCING	VERTICAL REINFORCING	TOP & BOTTOM HORIZONTAL BARS	NOTES
CSW-1	8"	#7 @ 12" O.C.	#7 @ 12" O.C.	1-#7	TYPE A
CSW-2	12"	#6 @ 12" O.C., E.F.	#6 @ 12" O.C., E.F.	2-#7	TYPE D
CSW-3	8"	#6 @ 12" O.C.	#6 @ 12" O.C.	1-#7	TYPE A
CSW-4	8"	#8 @ 12" O.C.	#8 @ 12" O.C.	1-#8	TYPE A
CSW-5	8"	#5 @ 12" O.C.	#5 @ 12" O.C.	1-#7	TYPE A
CSW-6	16"	#6 @ 12" O.C., E.F.	#6 @ 12" O.C., E.F.	2-#7	TYPE D
CSW-7	17"	#6 @ 12" O.C., E.F.	#6 @ 12" O.C., E.F.	2-#7	TYPE D



EPOXY ANCHORING SCHEDULE		
REBAR DOWEL SIZE	THREADED ROD DIAMETER	EMBEDMENT LENGTH
#3	3/8"	4.1/2"
#4	1/2"	6.1/2"
#5	5/8"	7.1/2"
#6	3/4"	10"
#7	7/8"	12"
#8	1"	13"
#9	1.1/8"	14"
#10	1.1/4"	15"
#11	1.3/8"	16"

NOTES:
1. EMBEDMENT LENGTHS SPECIFIED ON PLANS OR DETAILS TAKE PRECEDENCE OVER EMBEDMENT LENGTHS IN THIS SCHEDULE.
2. EMBEDMENT LENGTHS SHALL BE ADJUSTED WHEN EXISTING CONCRETE IS OF EQUAL OR LESS THICKNESS THAN SCHEDULE REQUIRES. IN THESE CASES THE EMBEDMENT LENGTH SHALL BE THE CONCRETE THICKNESS MINUS THE CLEAR COVER REQUIREMENTS, SEE GSN.
3. CONTINUOUS SPECIAL INSPECTION REQUIRED DURING INSTALLATION FOR ALL DOWELS AND THREADED RODS.

A1 EPOXY ANCHORING SCHEDULE WITH DETAIL
SB601 NO SCALE

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PROJECT NAME:
Snow College Library
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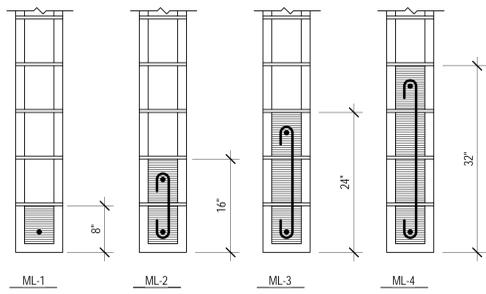
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SB601

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7/20/2008

MASONRY LINTEL SCHEDULE							
MARK	LINTEL DEPTH	LINTEL WIDTH	MASONRY TYPE	LINTEL		MAXIMUM SPAN	REMARKS
				HORIZONTAL	STIRRUPS		
ML-1	8"	6'-8", 10" OR 12"	BRICK OR CMU	1-#7 CONT.	----	3'-4"	
ML-2	16"	6'-8", 10" OR 12"	BRICK OR CMU	1-#7 CONT. TOP & BOT.	#3 @ 8" O.C.	5'-4"	
ML-3	24"	6'-8", 10" OR 12"	BRICK OR CMU	1-#7 CONT. TOP & BOT.	#3 @ 8" O.C.	8'-0"	
ML-4	32"	6'-8", 10" OR 12"	BRICK OR CMU	1-#7 CONT. TOP & BOT.	#3 @ 8" O.C.	10'-0"	

NOTES:

- MASONRY LINTELS ML-1 THROUGH ML-4 SHALL BE USED OVER OPENINGS IN MASONRY WALLS WHEN A SPECIFIC MASONRY LINTEL IS NOT OTHERWISE SPECIFIED. THE MASONRY LINTEL TO BE USED SHALL BE DETERMINED BY THE MAXIMUM SPAN AS SPECIFIED IN THIS SCHEDULE. WHEN A SPECIFIC MASONRY LINTEL IS CALLED OUT ON THE PLAN, THE MAXIMUM SPAN AS NOTED IN THIS SCHEDULE SHALL NOT APPLY.
- MASONRY LINTELS ML-1 THROUGH ML-4 SHALL NOT BE LOCATED BELOW ANY FLOOR, OR ROOF BEAM, OR GIRDER, OR ANY OTHER CONCENTRATED LOAD UNLESS SHOWN SPECIFICALLY ON THE PLAN SHEET. JOISTS SHALL NOT BEAR ON ANY LINTEL LESS THAN 16" DEEP.
- FOR MASONRY LINTELS NOT SHOWN ON THE DRAWINGS THAT CARRY ANY FLOOR, OR ROOF BEAM, OR GIRDER, OR ANY OTHER CONCENTRATED LOAD, OR THAT SPAN GREATER THAN 10'-0" CONSULT THE STRUCTURAL ENGINEER.
- EXTEND ALL HORIZONTAL REINFORCING 48 BAR DIAMETERS BEYOND THE EDGE OF THE OPENING. IF HORIZONTAL REINFORCING CANNOT BE EXTENDED 48 BAR DIAMETERS BEYOND THE EDGE OF THE OPENING, PROVIDE 90 DEGREE STANDARD HOOK.
- GROUT MASONRY LINTELS MONOLITHICALLY WITH THE SUPPORT WALL OR COLUMN AT EACH END.
- SPLICE TOP BARS AT MIDSPAN OF LINTEL ONLY.
- SPLICE BOTTOM BARS OVER SUPPORTS ONLY.
- FOR WALL ABOVE LINTEL, DOWEL VERTICAL REINFORCING INTO FULL DEPTH OF THE LINTEL OR 48 BAR DIAMETERS, WHICHEVER IS LESS.
- HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING WOULD OCCUR IN THE SAME COURSE, THE LARGER BARS ARE TO REPLACE THE SMALLER BARS.



C4
SB602
TYPICAL MASONRY LINTEL DETAILS
SCALE: 1" = 1'-0"

MASONRY WALL SCHEDULE					
MARK	THICK	MATERIALS	REINFORCING		NOTES
			VERTICAL	HORIZONTAL	
MW-1	10"	CMU	1-#5 @ 32" O.C.	2-#6 @ 32" O.C.	
MW-2	10"	CMU	1-#5 @ 32" O.C.	2-#6 @ 48" O.C.	
MW-3	10"	CMU	1-#5 @ 32" O.C.	2-#5 @ 48" O.C.	
MW-4	10"	CMU	1-#5 @ 32" O.C.	2-#5 @ 48" O.C.	
MW-5	6"	CMU	1-#5 @ 32" O.C.	#4 @ 48" O.C.	

NOTES:

- PROVIDE SCHEDULED VERTICAL REINFORCING BARS AT ALL CORNERS, ENDS OF WALLS, AND SPACED AS SCHEDULED, UNLESS NOTED OTHERWISE.
- HORIZONTAL REINFORCING BARS SHALL BE CONTINUOUS AT ALL CORNERS AND AT INTERSECTING WALLS. PROVIDE CORNER BARS WITH THE REQUIRED LAP SPLICE LENGTH.
- TERMINATE ALL HORIZONTAL REINFORCING BARS AT ENDS OF WALLS AND EDGES OF OPENINGS WITH A STANDARD HOOK AROUND VERTICAL REINFORCING BARS.
- SEE PLANS, DETAILS AND GENERAL STRUCTURAL NOTES FOR ADDITIONAL REINFORCING REQUIREMENTS.
- GROUT SOLID ALL CELLS BELOW GRADE, CELLS CONTAINING EMBEDS (HSA'S, DBA'S, ANCHOR BOLTS, ETC.), AND CELLS CONTAINING REINFORCING. CONSOLIDATE GROUT AS PER THE GENERAL STRUCTURAL NOTES.
- HORIZONTAL WALL REINFORCING SHALL BE PLACED INSIDE THE VERTICALS OF MASONRY COLUMNS.
- HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, THE LARGER BARS ARE TO REPLACE THE SMALLER BARS.

MASONRY REINFORCING BAR LAP SPLICE SCHEDULE										
BAR SIZE	f'm = 1500 psi						f'm = 2500 psi			
	6" CMU CLASS		8" CMU CLASS		10" CMU CLASS		12" CMU CLASS		6" ATLAS CLASS	8" ATLAS CLASS
	A	B	A	B	A	B	A	B	A	B
#3	19"	19"	19"	19"	19"	19"	19"	15"	15"	15"
#4	25"	25"	30"	25"	28"	25"	28"	20"	20"	24"
#5	35"	31"	49"	31"	45"	31"	45"	31"	24"	40"
#6	81"	57"	**	53"	92"	53"	92"	64"	45"	87"
#7	-	79"	**	61"	**	61"	**	-	63"	**
#8	-	**	**	87"	**	75"	**	-	89"	**
#9	-	-	-	**	**	90"	**	-	-	-

NOTES:

- CLASS A SPLICES MAY BE USED WHEN ONLY ONE BAR IS CONTINUOUS IN THE MASONRY CELL OR COURSE.
- CLASS B SPLICES SHALL BE USED WHEN TWO BARS ARE CONTINUOUS IN THE MASONRY CELL OR COURSE.
- ** INDICATES THAT A LAP SPLICE IS NOT ALLOWED AND MECHANICAL BAR COUPLERS ARE REQUIRED FOR THE BAR SPLICES.
- WHERE VERTICAL BARS HAVE A REQUIRED LAP SPLICE GREATER THAN THE HEIGHT OF THE GROUT POUR, THE BAR SPLICE SHALL BE MADE WITH A MECHANICAL BAR COUPLER. WHERE THE HEIGHT OF THE GROUT POUR EXCEEDS 60 INCHES, HIGH LIFT GROUTING PROCEDURES SHALL BE FOLLOWED.
- WHERE MECHANICAL BAR COUPLERS ARE USED, THE CONNECTOR SHALL DEVELOP 125% OF THE SPECIFIED YIELD STRENGTH OF THE BAR IN TENSION AND COMPRESSION.

MASONRY COLUMN SCHEDULE				
MARK	SIZE	REINFORCING		REMARKS
		VERTICAL	TIES	
MC-1	10" x 8"	2-#5	NONE	
MC-2	10" x 16"	4-#5	NONE	

NOTES:

- THE CENTERLINE OF VERTICAL BARS SHALL BE LOCATED 2.1/2" FROM THE FACE OF THE MASONRY. HORIZONTAL BARS SHALL BE LOCATED TO THE INSIDE OF THE VERTICAL BARS.
- UNLESS NOTED OTHERWISE, VERTICAL REINFORCING AND TIES SHALL EXTEND FULL HEIGHT OF THE WALL.
- MASONRY COLUMN VERTICAL BARS OR DOWELS IN CONCRETE FOUNDATION WALLS SHALL BE TIED WITH #3 TIES @ 8" O.C.

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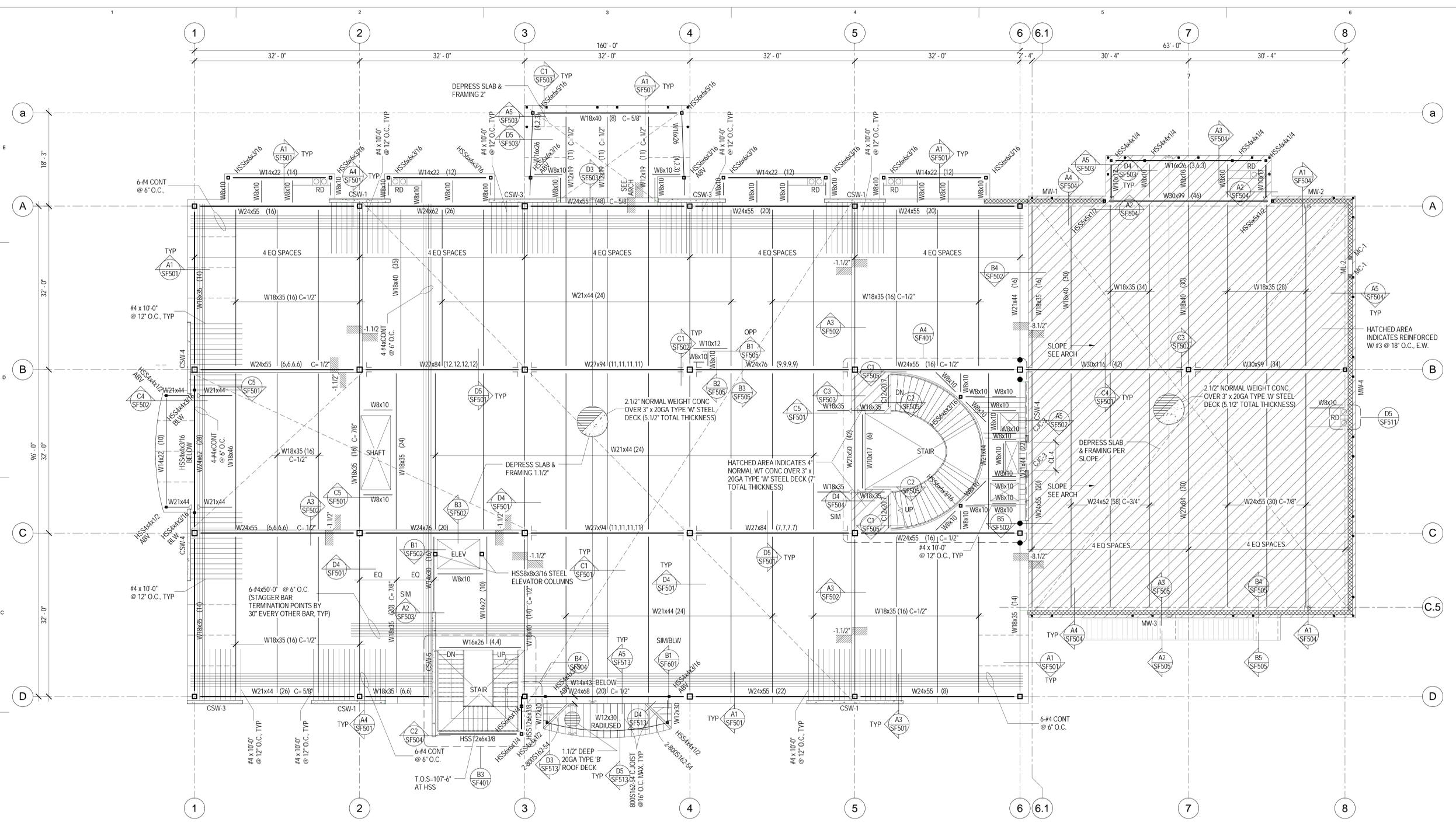
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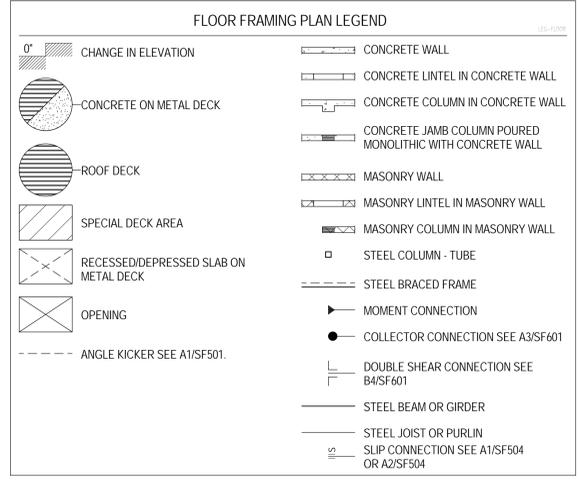
SHEET TITLE:
Level 2 Floor Framing Plan

SHEET NUMBER:
SF103



B3 Level 2 Floor Framing Plan
SCALE: 1/8" = 1'-0"

- FLOOR FRAMING PLAN NOTES**
- SEE GENERAL STRUCTURAL NOTE (III.D.8) AND DETAILS D4 & D5/SF501 FOR CONTROL JOINTS IN SUSPENDED SLABS OVER STEEL DECK.
 - SEE DETAILS D1 & D3/SF501 FOR MISCELLANEOUS FLOOR OPENINGS.
 - SEE GENERAL STRUCTURAL NOTE (V.J.7) FOR STEEL DECK REQUIREMENTS WHERE 3-SPAN CONDITIONS ARE NOT POSSIBLE.
 - SEE D1/SF502 FOR TYPICAL MECHANICAL HOUSEKEEPING PAD OVER COMPOSITE FLOOR BEAMS.
 - SEE D2/SF502 FOR TYPICAL HEADED STUD ANCHOR PLACEMENT PATTERN FOR COMPOSITE STEEL BEAMS.
 - MAXIMUM LIFT WEIGHT PLUS PLATFORM CAPACITY = 3.2K.





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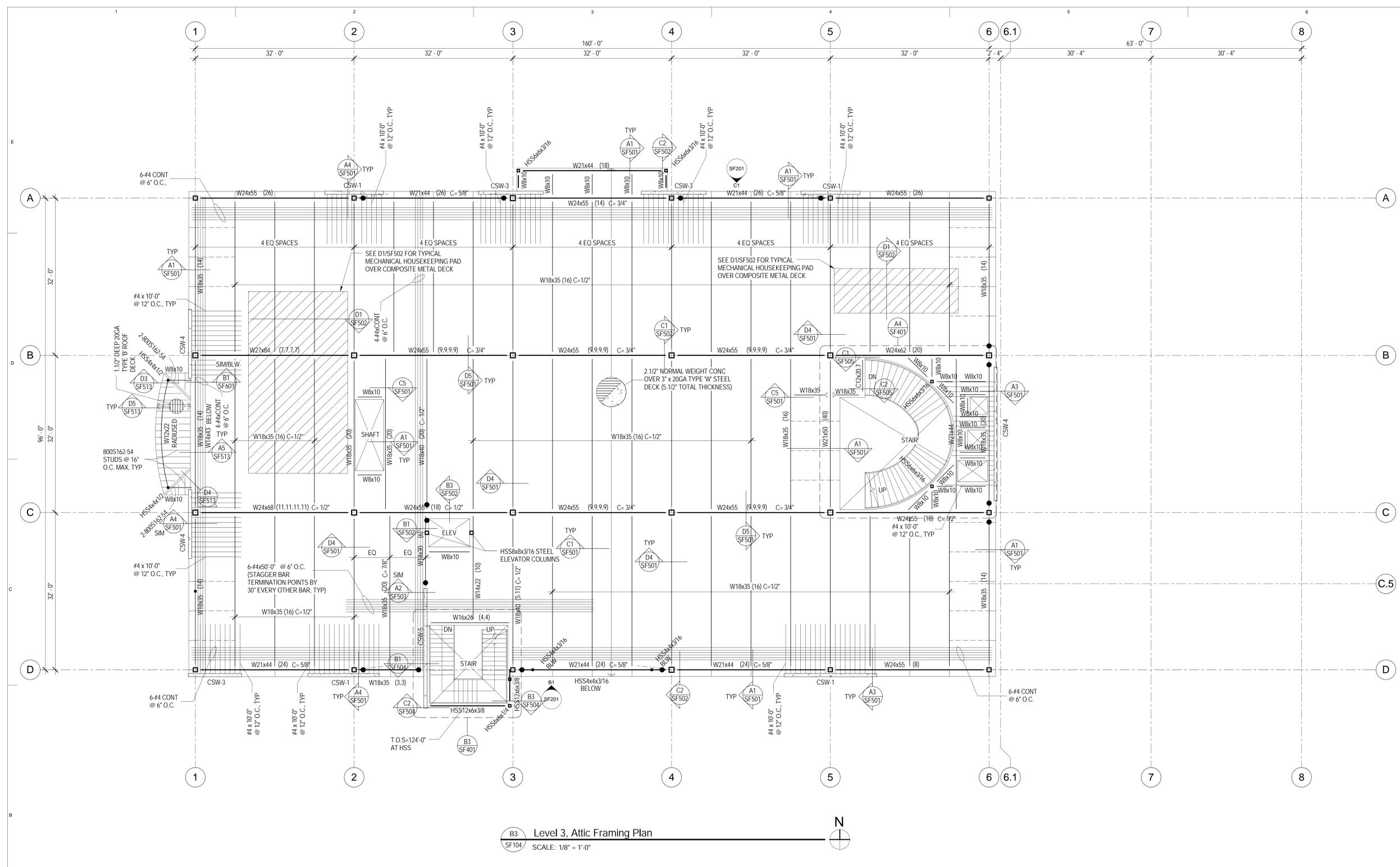
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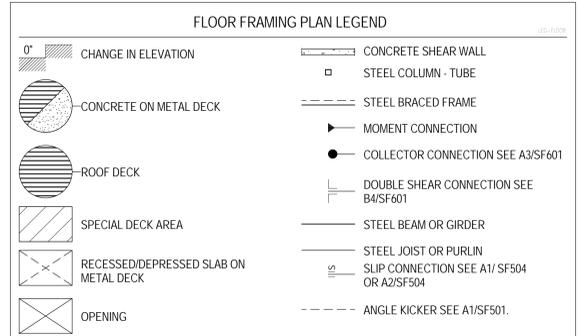
SHEET TITLE:
Level 3, Attic Framing Plan

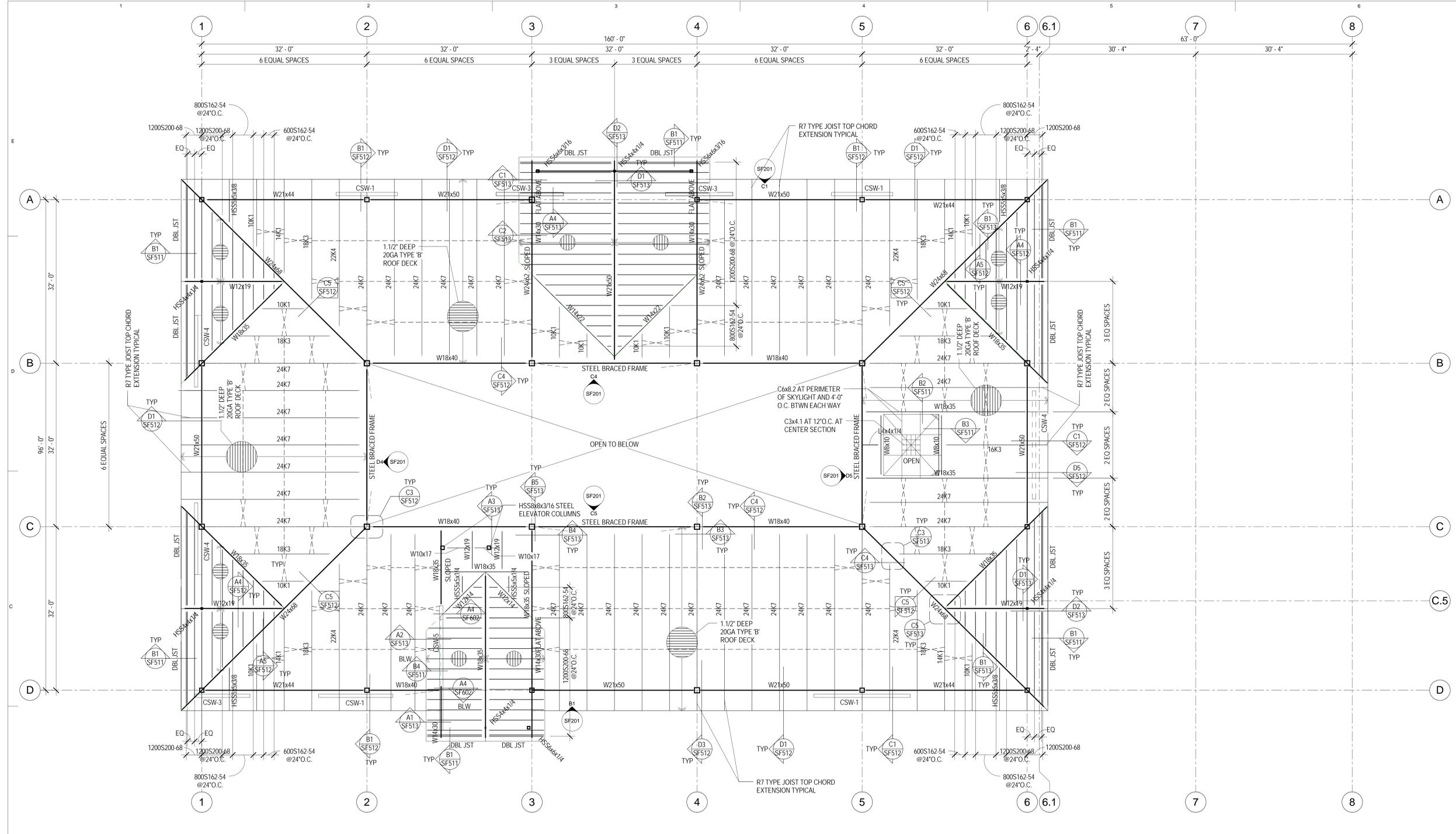
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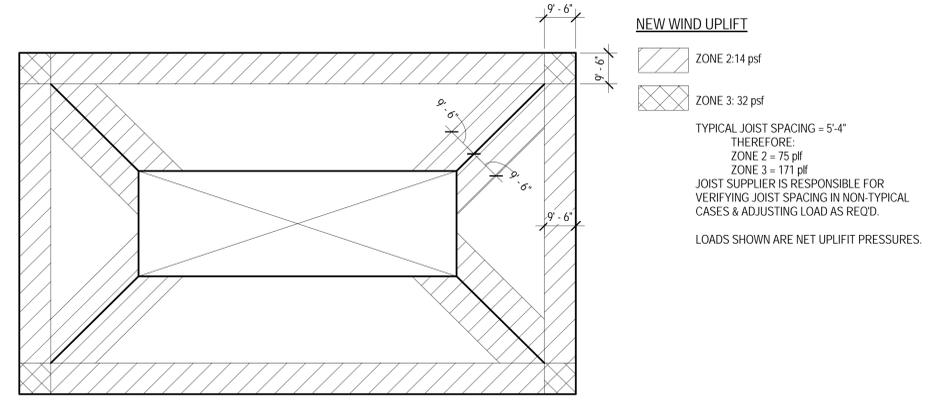
B3 Level 3, Attic Framing Plan
SCALE: 1/8" = 1'-0"

- FLOOR FRAMING PLAN NOTES**
- SEE GENERAL STRUCTURAL NOTE (III.D.8) AND DETAILS D4 & D5/SF501 FOR CONTROL JOINTS IN SUSPENDED SLABS OVER STEEL DECK.
 - SEE DETAILS D1 & D3/SF501 FOR MISCELLANEOUS FLOOR OPENINGS.
 - SEE GENERAL STRUCTURAL NOTE (V.J.7) FOR STEEL DECK REQUIREMENTS WHERE 3-SPAN CONDITIONS ARE NOT POSSIBLE.
 - SEE D1/SF502 FOR TYPICAL MECHANICAL HOUSEKEEPING PAD OVER COMPOSITE FLOOR BEAMS.
 - SEE D2/SF502 FOR TYPICAL HEADED STUD ANCHOR PLACEMENT PATTERN FOR COMPOSITE STEEL BEAMS.
 - MAXIMUM LIFT WEIGHT PLUS PLATFORM CAPACITY = 3.2K.

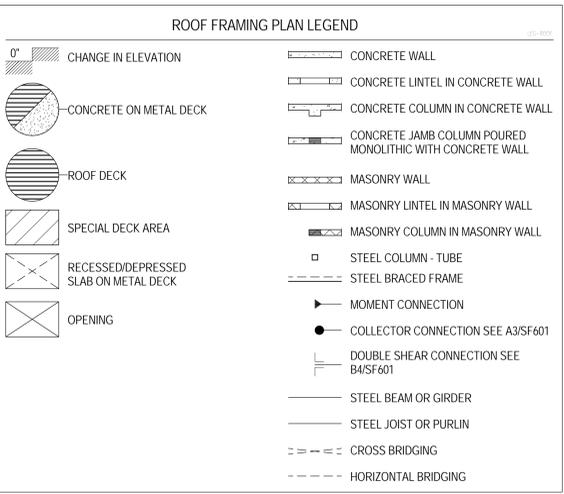




Roof Framing Plan
SCALE: 1/8" = 1'-0"
N



- ROOF FRAMING PLAN NOTES**
- VERIFY SIZE, WEIGHT, LOCATION AND CONFIGURATION OF ALL ROOF TOP EQUIPMENT WITH ARCHITECT AND MECHANICAL ENGINEER. PROVIDE STEEL FRAMES FOR SUPPORT OF ROOF TOP EQUIPMENT AS INDICATED IN DETAIL D3/SF511 COORDINATE OPENINGS WITH MECHANICAL AND ELECTRICAL AND GENERAL CONTRACTORS.
 - ALL ROOF OPENINGS SHALL BE FRAMED AS INDICATED IN DETAIL D2/SF511. FOR ROUND OPENINGS WHICH ARE LESS THAN 12" DIA. SEE DETAIL D4/SF511.
 - SEE ARCHITECTURAL FOR ROOF SLOPES AND DRAINS. SEE D5/SF511 FOR ROOF DRAIN OPENING FRAME.
 - OPEN WEB STEEL JOISTS AND JOIST GIRDERS SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT THE MECHANICAL AND LATERAL LOADS SHOWN ON THE ROOF FRAMING PLANS IN ADDITION TO THE UNIFORM AND POINT LOADS SHOWN.
 - ##K ADD - INDICATES ADDITIONAL UPLIFT / DOWNWARD FORCE ON STEEL JOIST IN ADDITION TO REGULAR LOADS.
 - TIC=##K - INDICATES ADDITIONAL TOP CHORD AXIAL FORCE ON STEEL JOIST OR GIRDER IN ADDITION TO REGULAR LOADS. THIS FORCE IS A SEISMIC LRFD LOAD THAT SHALL BE CONSIDERED IN BOTH TENSION AND COMPRESSION. INCLUDES APPLICABLE OVERSTRENGTH FACTORS FROM IBC SECTION 1613 AND ASCE 7 SECTION 12.10. STEEL JOISTS AND GIRDERS WITH TIC FORCE SHALL BE DESIGNED AS COLLECTOR ELEMENTS PER IBC SECTION 1613 AND ASCE 7 SECTION 12.10 WITH STRENGTH TO RESIST APPLICABLE LOAD COMBINATIONS OF IBC SECTION 1605.4 AND ASCE 7 SECTION 12.4.
 - ALL LOADS SUPPORTED BY OPEN WEB STEEL JOISTS AND GIRDERS SHALL BE LOCATED WITHIN 6" OF JOIST OR GIRDER PANEL POINT OR THE JOIST OR GIRDER SHALL BE REINFORCED PER DETAIL D1/SF511.
 - SEE DETAIL C1/SF511 FOR SUPPORT OF HANGING MECHANICAL UNITS.
 - HORIZONTAL AND CROSS BRIDGING SHALL BE SIZED AND SUPPLIED BY THE JOIST MANUFACTURER. CONNECT TO WALLS AS INDICATED IN DETAILS.
 - WHERE SKYLIGHTS OR MECHANICAL UNITS INTERRUPT HORIZONTAL BRIDGING PROVIDE CROSS BRIDGING AT JOIST SPACES ON EACH SIDE. TYPICAL.





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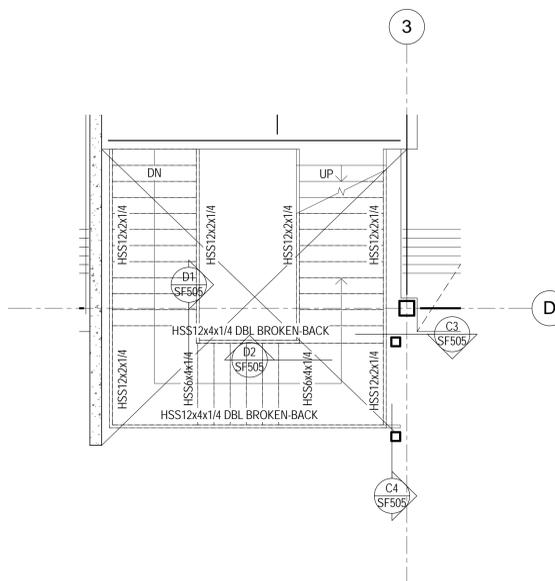
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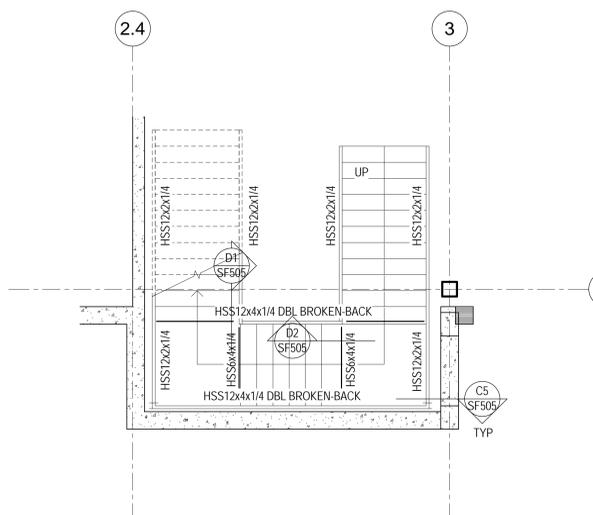
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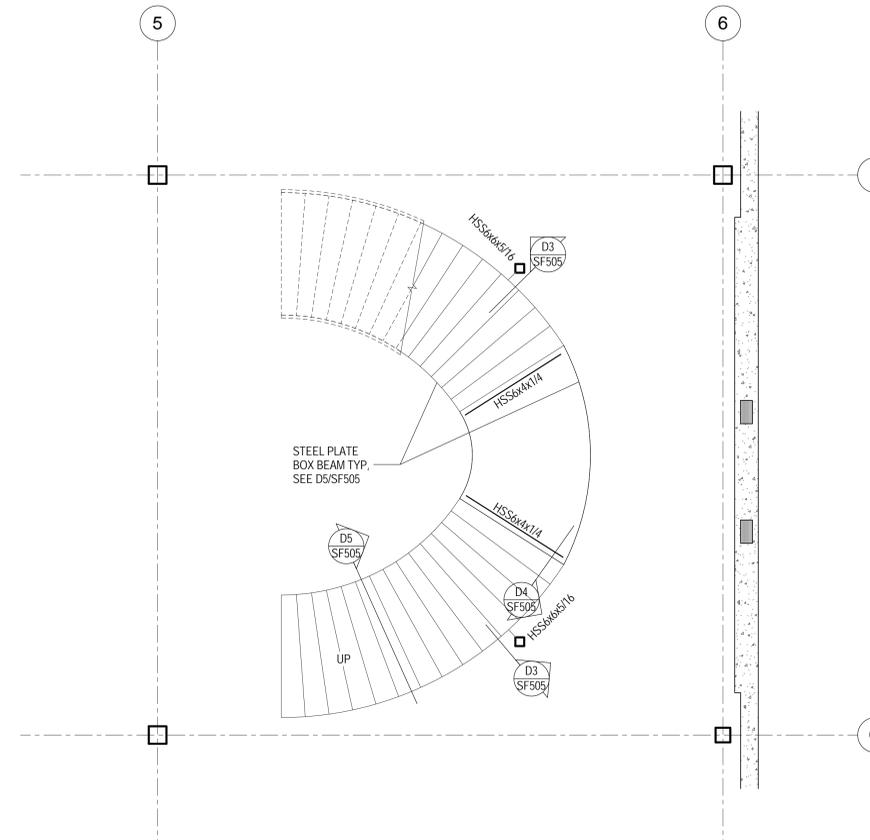
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Enlarged Stair Framing Plans		
SHEET NUMBER:		
SF401		



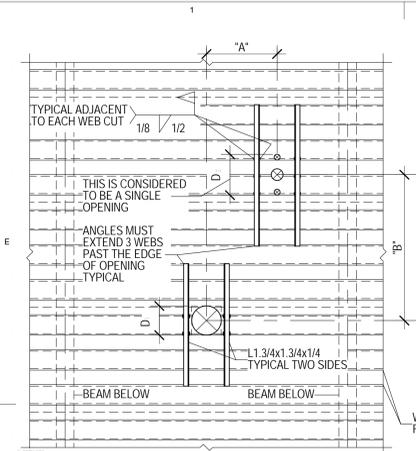
B3 South Stair - Stair Framing Plan - Upper
SF401 SCALE: 1/4" = 1'-0"



A3 South Stair - Stair Framing Plan - Lower
SF401 SCALE: 1/4" = 1'-0"



A4 Grand Stair - Typical Stair Framing Plan
SF401 SCALE: 1/4" = 1'-0"



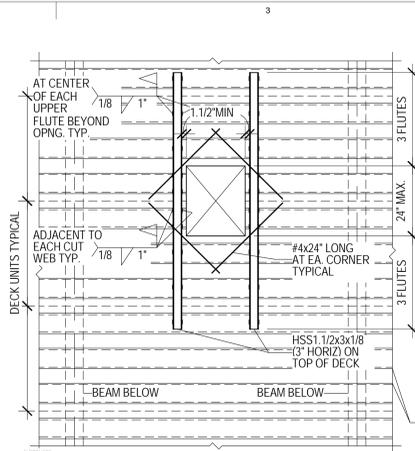
USE THIS DETAIL FOR HOLES CUTTING NO MORE THAN 3 ADJACENT WEBS FOR 6" AND 8" MODULE DECK, 2 ADJACENT WEBS FOR 12" MODULE DECK.

PRIOR TO CONCRETE POUR, SMALL OPENINGS SHOULD BE BLOCKED OUT AND FLOOR DECK LEFT INTACT. HOLES LESS THAN 6" IN DIAMETER AND CUTTING NO MORE THAN 1 WEB NEED NO REINFORCING. AFTER THE CONCRETE HAS CURED, THE BLOCKOUT CAN BE REMOVED AND THE FLOOR DECK IN THE AREA OF THE HOLE REMOVED.

NOTES:

1. ANGLES SHALL BE PLACED ON TOP OF THE DECK.
2. IF DIMENSION "A" IS GREATER THAN 4D, .4D, OR 32" WHICHEVER IS LARGER, THEN THERE IS NO RESTRICTION ON DIMENSION "B".
3. IF DIMENSION "B" IS GREATER THAN 4D, .4D, OR 32" WHICHEVER IS LARGER, THEN THERE IS NO RESTRICTION ON DIMENSION "A".
4. IF DIMENSIONS "A" AND "B" ARE LESS THAN 4D, .4D, OR 32" WHICHEVER IS LARGER, THE OPENING GROUP WILL BE CONSIDERED AS A SINGLE HOLE, AND MUST BE REINFORCED AS REQUIRED FOR THE LARGER OPENING.

D1
SF501
TYPICAL MISCELLANEOUS OPENING REINFORCING DETAIL
NO SCALE



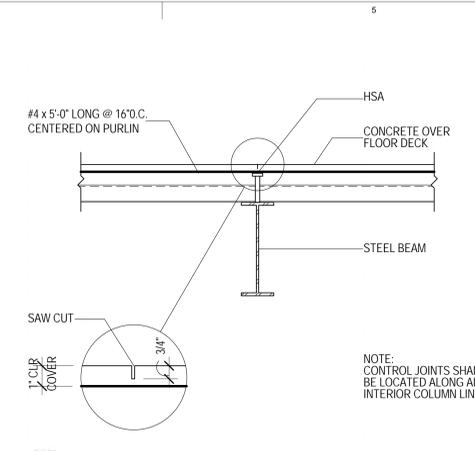
USE THIS DETAIL FOR LARGER HOLES, RECTANGULAR OR SQUARE.

PRIOR TO CONCRETE POUR, SMALL OPENINGS SHOULD BE BLOCKED OUT AND FLOOR DECK LEFT INTACT. HOLES LESS THAN 6" IN DIAMETER AND CUTTING NO MORE THAN 1 WEB NEED NO REINFORCING. AFTER THE CONCRETE HAS CURED, THE BLOCKOUT CAN BE REMOVED AND THE FLOOR DECK IN THE AREA OF THE HOLE REMOVED.

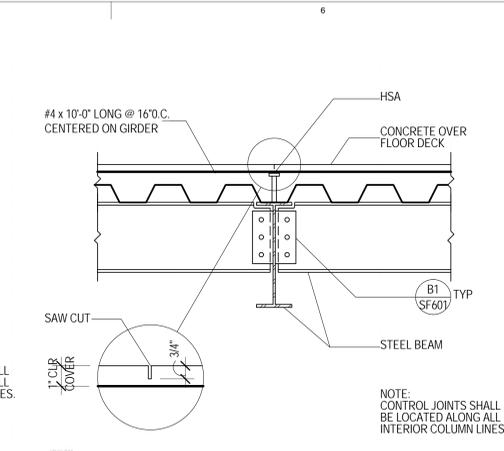
NOTES:

1. IF THE OPENING OR GROUP OF OPENINGS OCCURS IN ONE DECKING UNIT, THE OPENING OR OPENING GROUP MAY BE CUT PRIOR TO POURING OF CONCRETE.
2. IF, AS SHOWN, THE OPENING OR OPENING GROUP CUTS THROUGH TWO DECKING UNITS, THE DECKING SHALL NOT BE CUT UNTIL CONCRETE HAS BEEN PLACED AND CURED.
3. WHEN THE MAXIMUM DIMENSION OF AN OPENING OR OPENING GROUP EXCEEDS 24", PROVIDE W8x10 HEADER BEAMS BELOW THE DECK AT ALL SIDES.

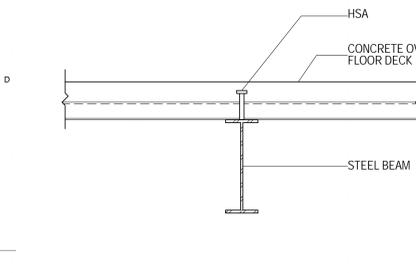
D3
SF501
TYPICAL MISCELLANEOUS OPENING REINFORCING DETAIL
NO SCALE



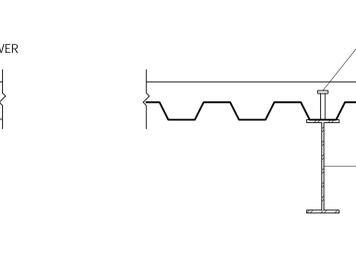
D4
SF501
TYPICAL COMPOSITE DECK CONTROL JOINT AT STEEL PURLIN
NO SCALE



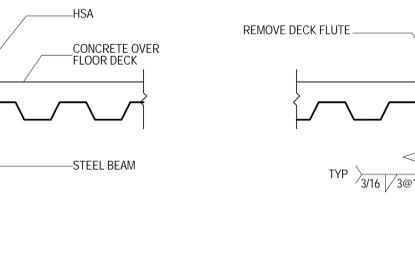
D5
SF501
TYPICAL COMPOSITE DECK CONTROL JOINT AT STEEL GIRDER
NO SCALE



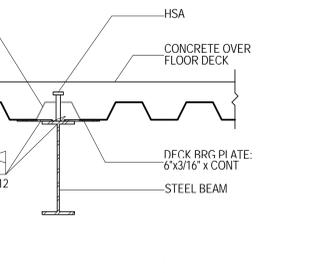
C1
SF501
TYPICAL COMPOSITE STEEL BEAM
NO SCALE



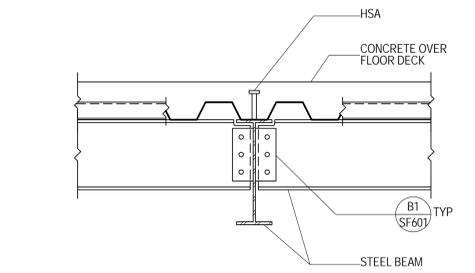
C2
SF501
TYPICAL COMPOSITE STEEL BEAM
NO SCALE



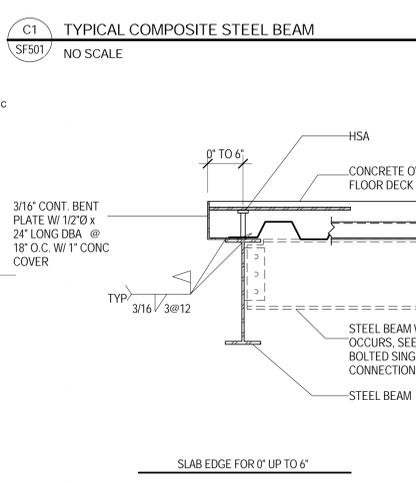
C3
SF501
TYPICAL ALTERNATE DECK BEARING AT COMPOSITE STEEL BEAM
NO SCALE



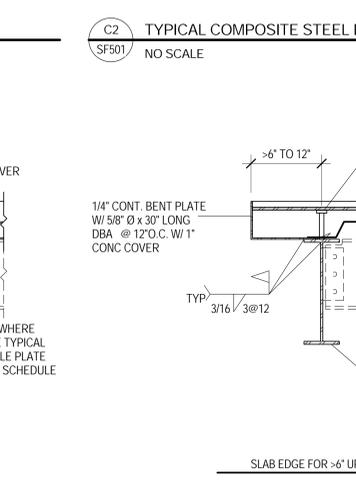
C4
SF501
TYPICAL COMPOSITE STEEL BEAM W/ STEEL BEAM TWO SIDES
NO SCALE



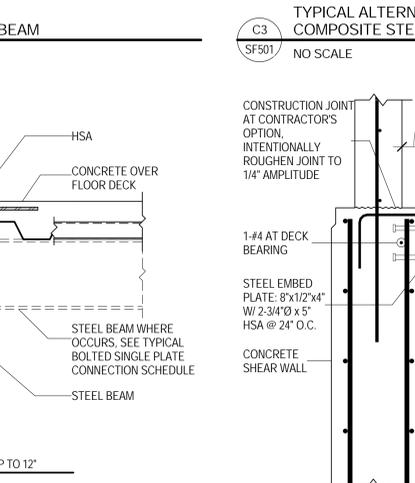
C5
SF501
TYPICAL COMPOSITE STEEL BEAM W/ STEEL BEAM ONE SIDE
NO SCALE



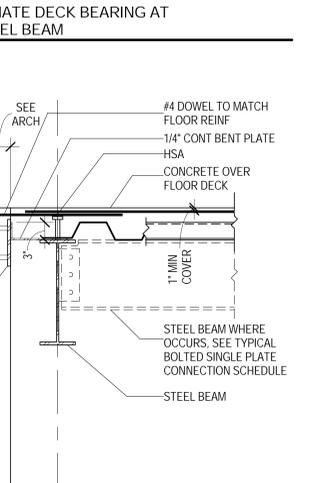
A1
SF501
TYPICAL SLAB EDGE DETAILS AT COMPOSITE BEAMS
NO SCALE



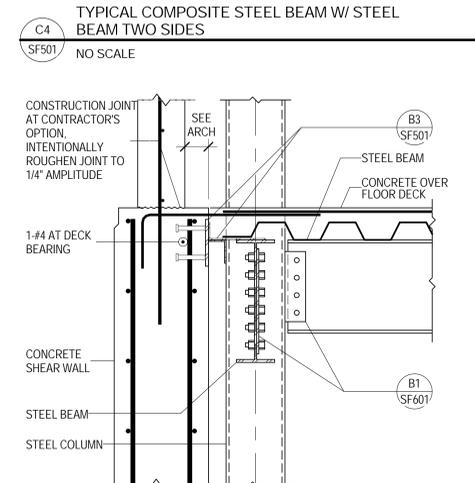
A2
SF501
TYPICAL SLAB EDGE DETAILS AT COMPOSITE BEAMS
NO SCALE



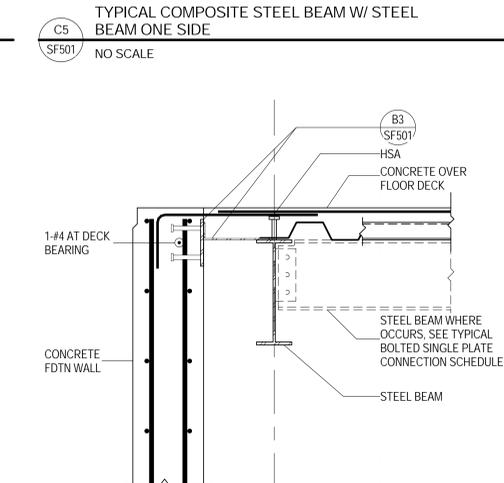
B3
SF501
COMPOSITE SLAB EDGE AT CONCRETE SHEAR WALL
NO SCALE



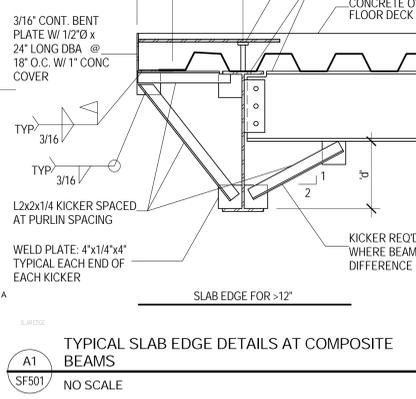
B4
SF501
TYPICAL BOLTED CONNECTION TO STEEL COLUMN - BEAM ONE SIDE
NO SCALE



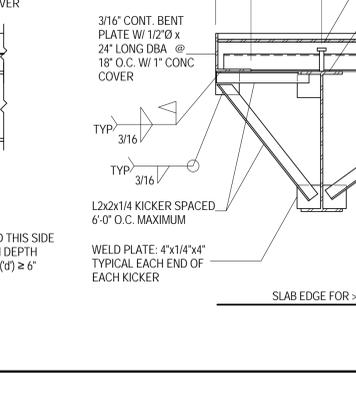
B5
SF501
COMPOSITE SLAB EDGE AT CONCRETE SHEAR WALL
NO SCALE



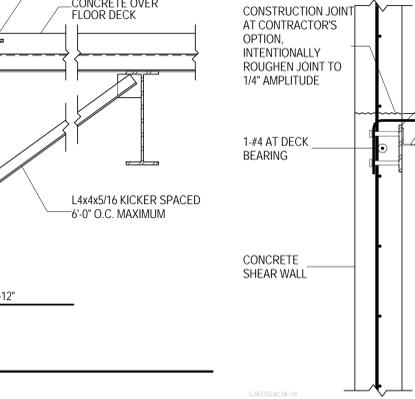
A3
SF501
TYPICAL SLAB EDGE DETAILS AT COMPOSITE BEAMS
NO SCALE



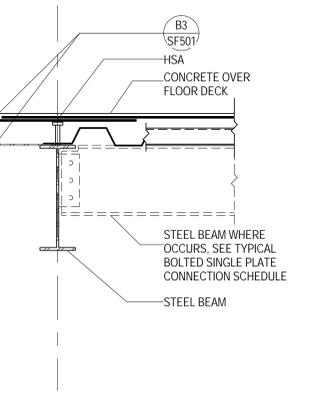
A4
SF501
TYPICAL BOLTED CONNECTION TO STEEL COLUMN - BEAM ONE SIDE
NO SCALE



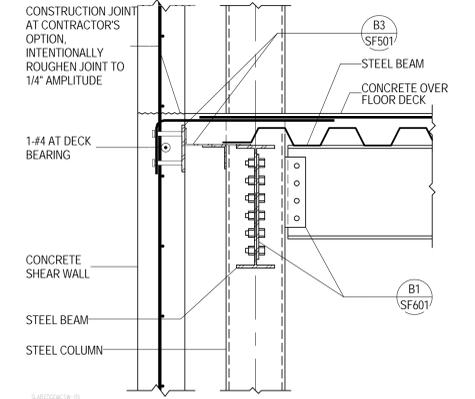
A5
SF501
TYPICAL BOLTED CONNECTION TO STEEL COLUMN - BEAM ONE SIDE
NO SCALE



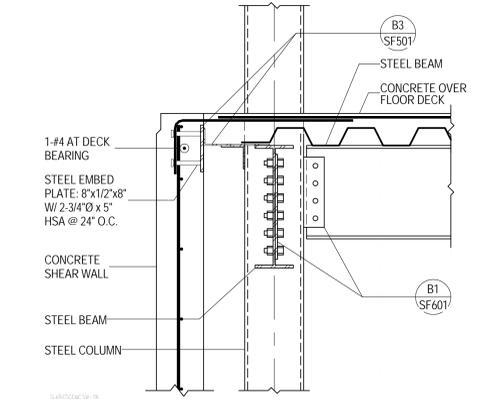
A3
SF501
COMPOSITE SLAB EDGE AT CONCRETE SHEAR WALL
NO SCALE



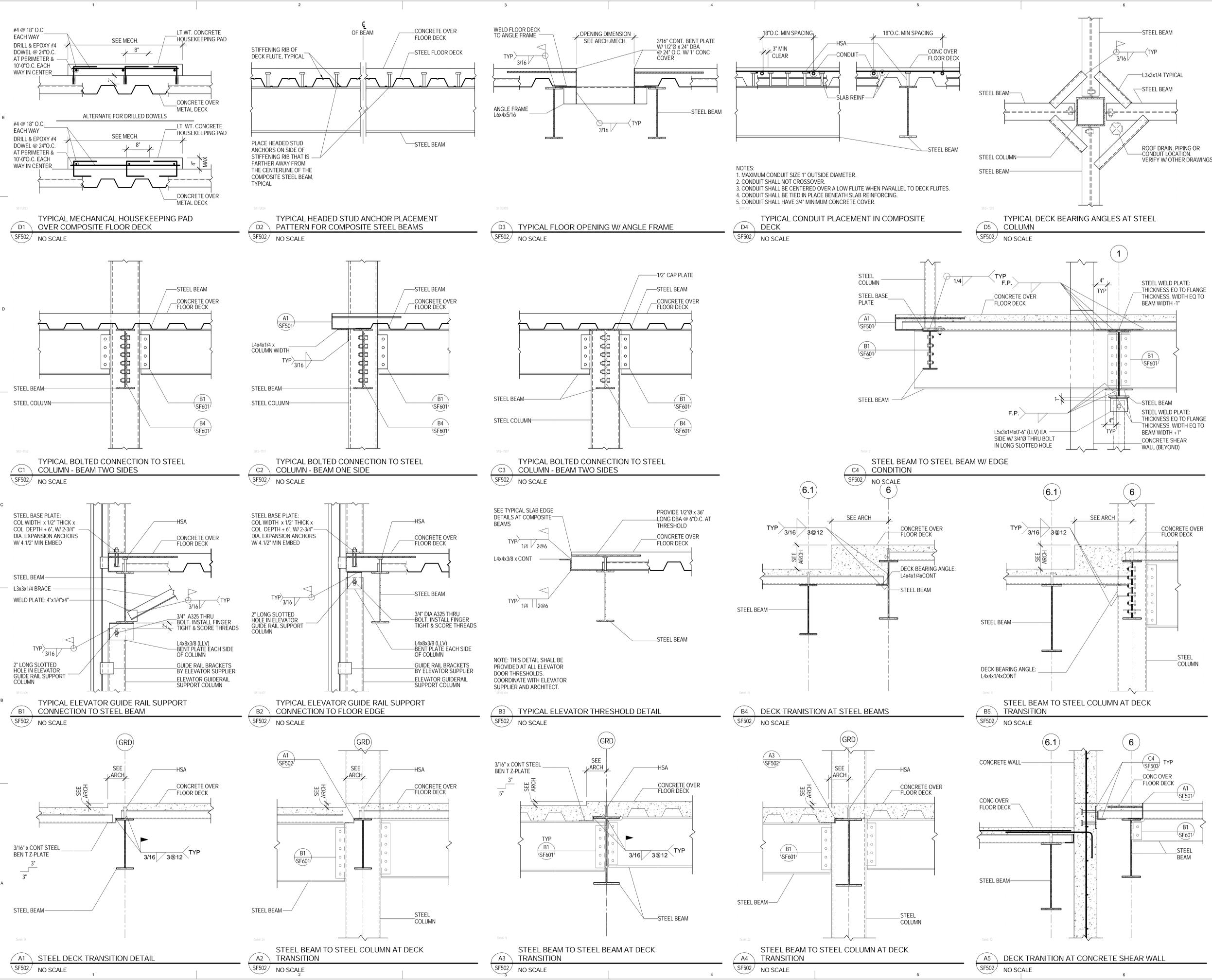
A4
SF501
TYPICAL BOLTED CONNECTION TO STEEL COLUMN - BEAM ONE SIDE
NO SCALE



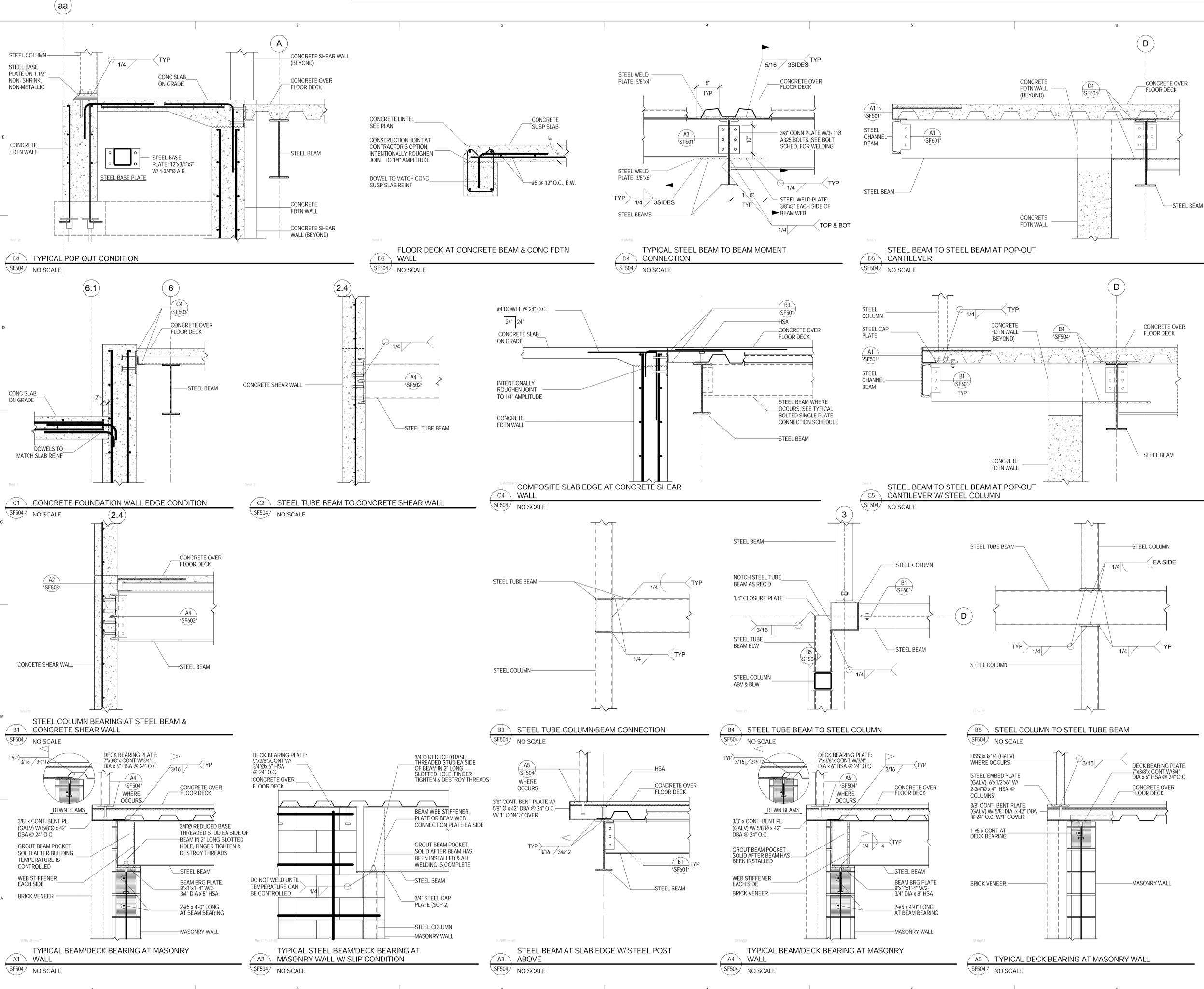
A5
SF501
TYPICAL BOLTED CONNECTION TO STEEL COLUMN - BEAM ONE SIDE
NO SCALE



A5
SF501
TYPICAL BOLTED CONNECTION TO STEEL COLUMN - BEAM ONE SIDE
NO SCALE



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7/26/2008



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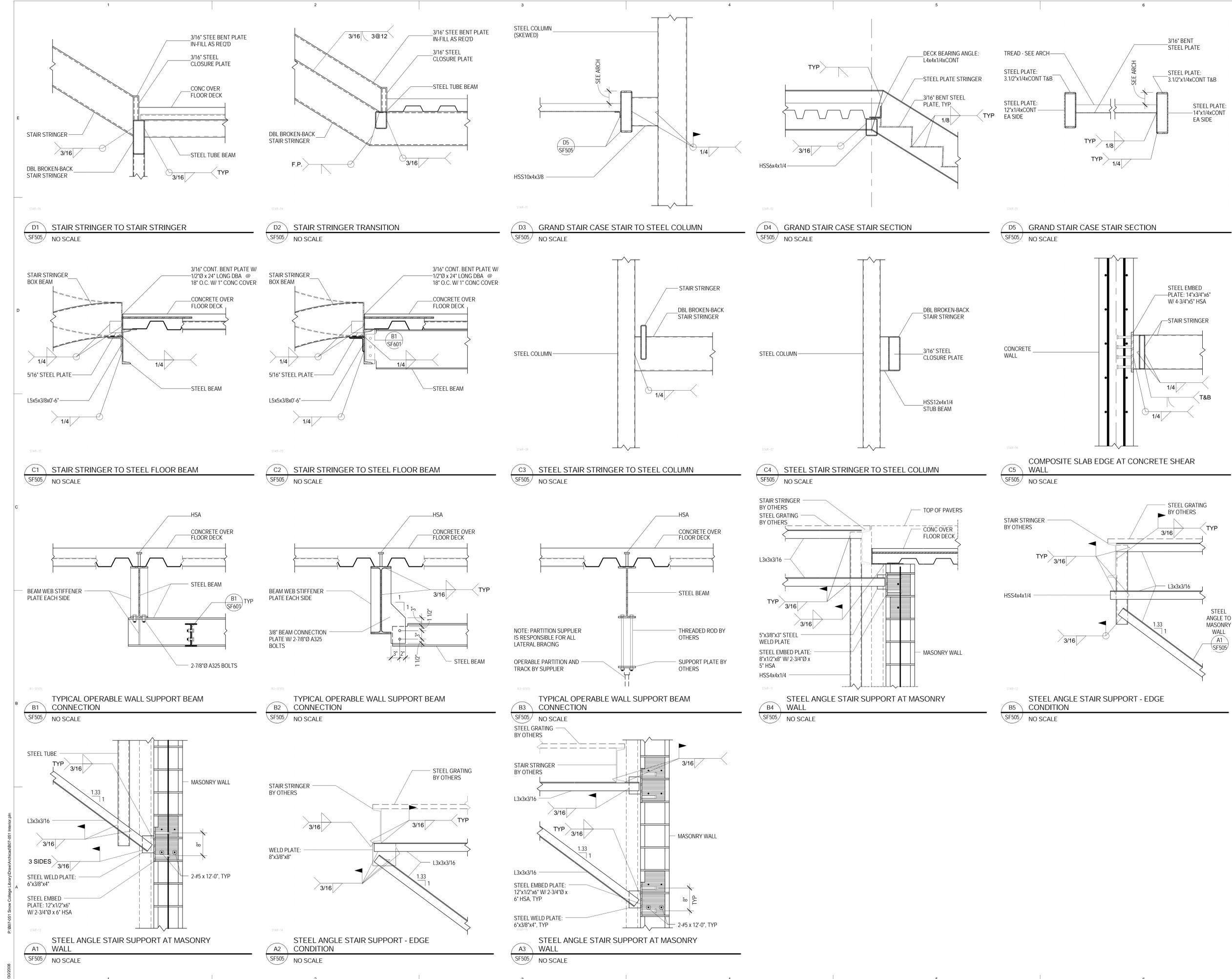
PROJECT NAME:
Snow College Library
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REVISIONS:

100% CD September 15, 2008
100% CD Review, August 4, 2008
ISSUE DATE:
SEPTEMBER 15 2008, 100% CD
ARCHITECT'S PROJECT NUMBER:
B07-051
DFCM PROJECT NUMBER:
07258700
SHEET TITLE:

Typical Floor Framing Details

SHEET NUMBER:
SF504



7/20/2008 P:\B07\051 Snow College Library\Drawings\Architect\B07\051 Interior.rvt

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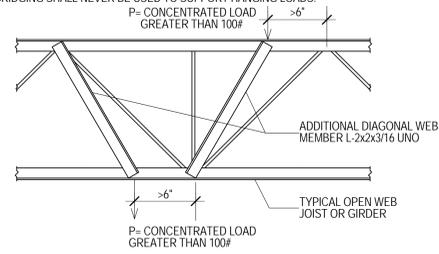
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SHEET TITLE:

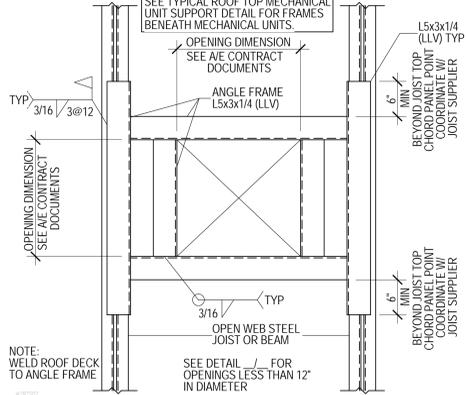
Typical Floor Framing
Details
SHEET NUMBER:

SF505

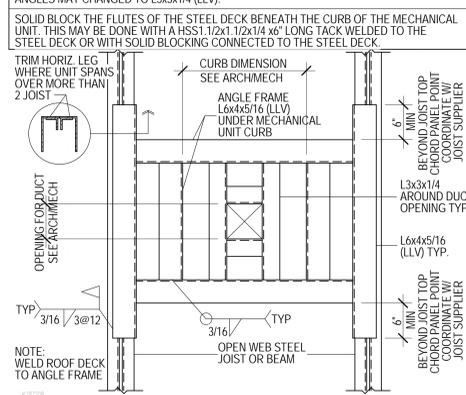
NOTE:
WHEN CONCENTRATED LOADS (GREATER THAN 100#) ON OPEN WEB JOISTS OR GIRDERS ARE LOCATED MORE THAN 6 INCHES FROM THE PANEL WORKPOINTS AT EITHER THE TOP OR BOTTOM CHORD ADDITIONAL DIAGONAL WEB MEMBERS SHALL BE FURNISHED AND INSTALLED AT THE LOCATION OF THE CONCENTRATED LOAD BY THE CONTRACTOR. CONCENTRATED POINT LOADS, SINGLE OR MULTIPLE, TOTALING 100# OR LESS CAN BE LOCATED AT ANY POINT ALONG THE TOP OR BOTTOM CHORD OF AN OPEN WEB JOIST OR GIRDER BETWEEN ADJACENT PANEL POINTS WITHOUT MEETING THESE REQUIREMENTS. A LIMIT OF FOUR CONCENTRATED 100# MAX. POINT LOADS PER JOIST OR GIRDER WILL BE PERMITTED ON SPANS OF 12' & GREATER. ONE CONCENTRATED 100# MAX. LOAD ON SPANS LESS THAN 12' UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS. JOIST BRIDGING SHALL NEVER BE USED TO SUPPORT HANGING LOADS.



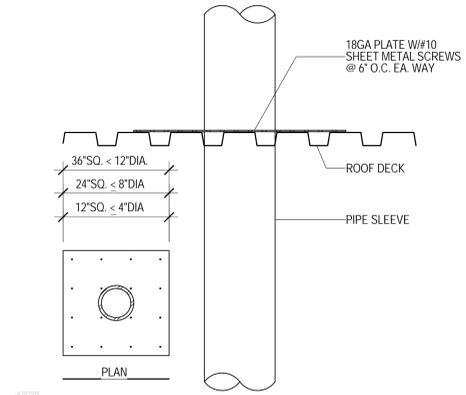
D1 TYPICAL DETAIL AT ADDITIONAL CONCENTRATED POINT LOAD
SF511 NO SCALE



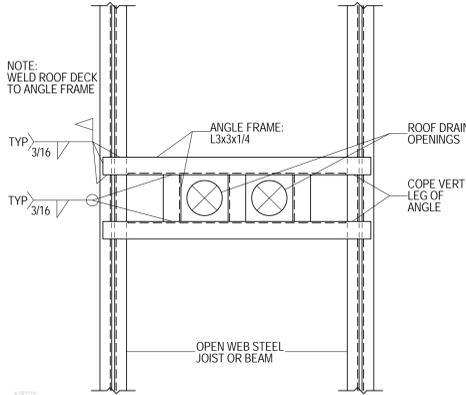
D2 TYPICAL ROOF OPENING DETAIL (PLAN VIEW)
SF511 NO SCALE



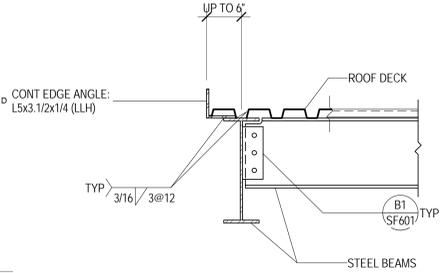
D3 TYP ROOF TOP MECHANICAL UNIT SUPPORT DETAIL (PLAN VIEW)
SF511 NO SCALE



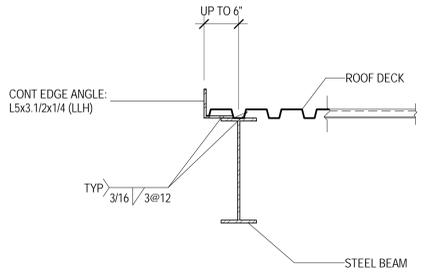
D4 TYPICAL PIPE SLEEVE HOLE THRU ROOF DECK
SF511 NO SCALE



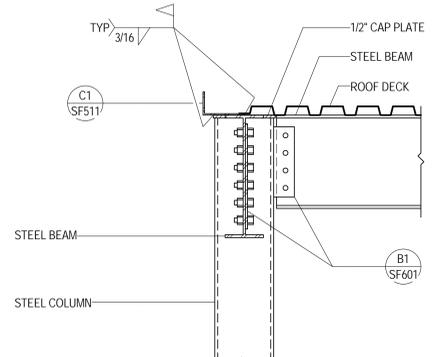
D5 TYPICAL ROOF DRAIN OPENING (PLAN VIEW)
SF511 NO SCALE



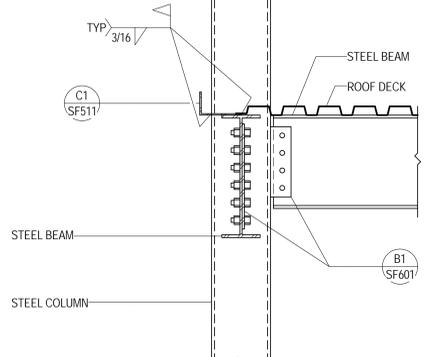
C1 TYPICAL STEEL ROOF BEAM AT ROOF EDGE DETAIL UP TO 6"
SF511 NO SCALE



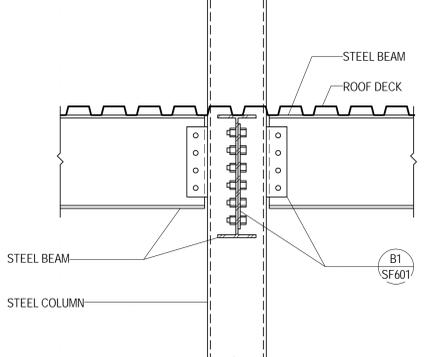
C2 TYPICAL STEEL ROOF BEAM AT ROOF EDGE DETAIL UP TO 6"
SF511 NO SCALE



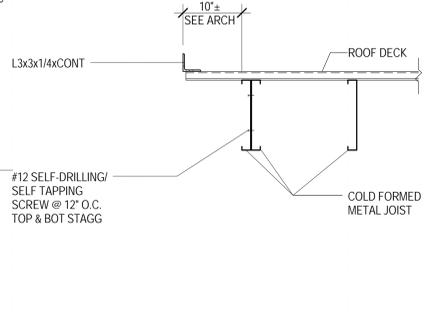
C3 TYPICAL BOLTED CONNECTION TO STEEL COLUMN - BEAM ONE SIDE
SF511 NO SCALE



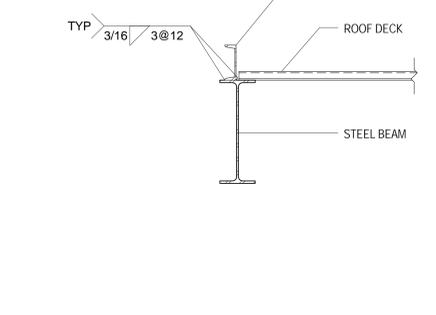
C4 TYPICAL BOLTED CONNECTION TO STEEL COLUMN - BEAM ONE SIDE
SF511 NO SCALE



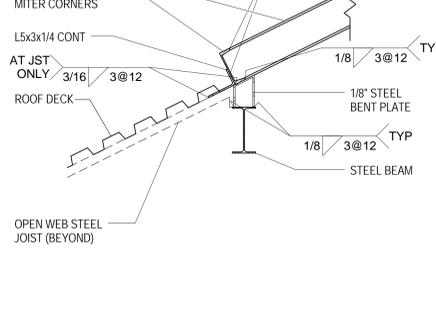
C5 TYPICAL BOLTED CONNECTION TO STEEL COLUMN - BEAM TWO SIDES
SF511 NO SCALE



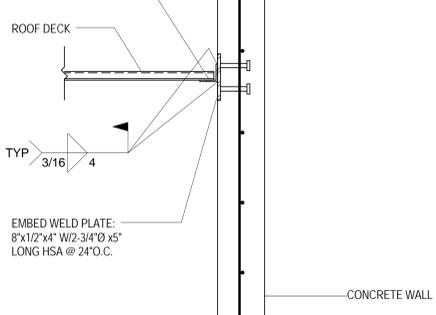
B1 TYPICAL ROOF DECK EDGE DETAIL
SF511 NO SCALE



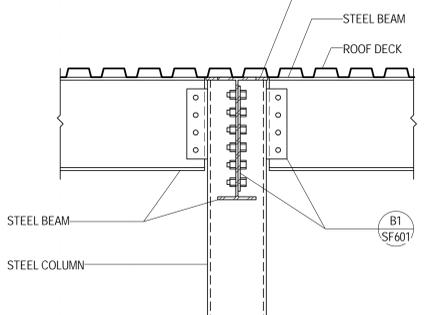
B2 STEEL CHANNEL TO STEEL BEAM
SF511 NO SCALE



B3 STEEL CHANNEL TO STEEL BEAM
SF511 NO SCALE



B4 TYPICAL ROOF DECK BEARING AT CONCRETE WALL
SF511 NO SCALE



B5 TYPICAL BOLTED CONNECTION TO STEEL COLUMN - BEAM TWO SIDES
SF511 NO SCALE

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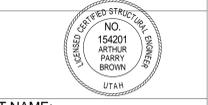
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ARCHITECT'S PROJECT NUMBER:
B07-051
DFCM PROJECT NUMBER:
07258700
SHEET TITLE:

Typical Roof Framing
Details

SHEET NUMBER:
SF511

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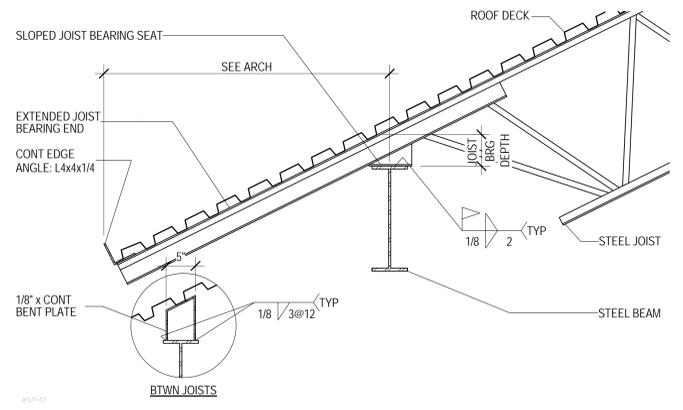
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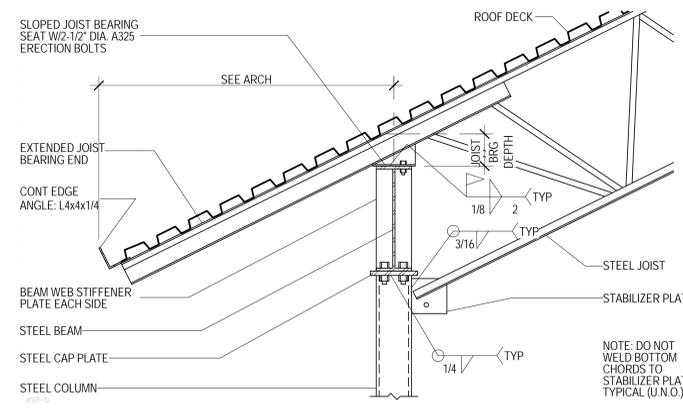
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07258700

SHEET TITLE:
Typical Roof Framing
Details

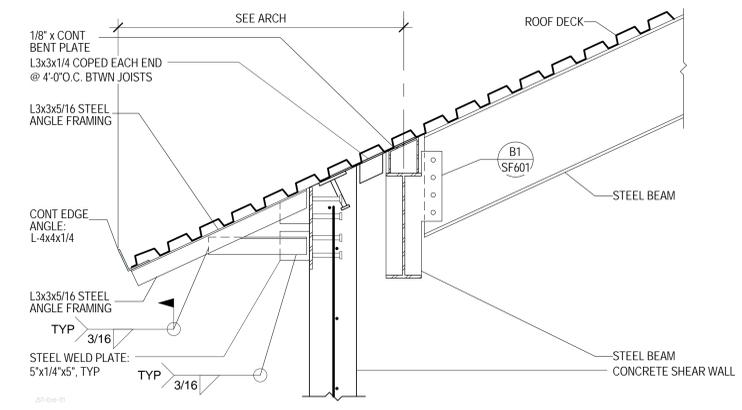
SHEET NUMBER:
SF512



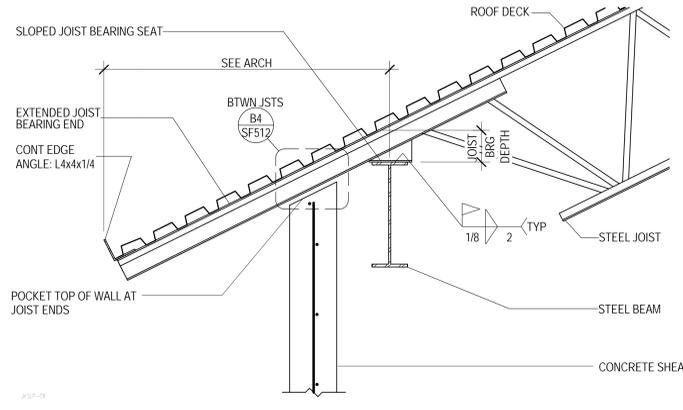
D1 TYPICAL STEEL JOIST BEARING AT STEEL BEAM
SF512 NO SCALE



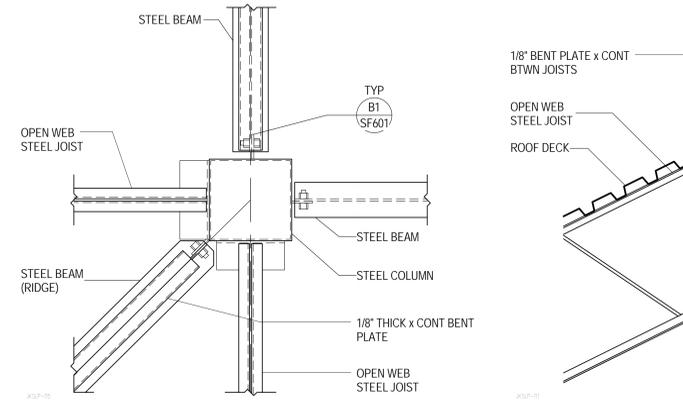
D3 TYPICAL STEEL JOIST BEARING AT STEEL COLUMN
SF512 NO SCALE



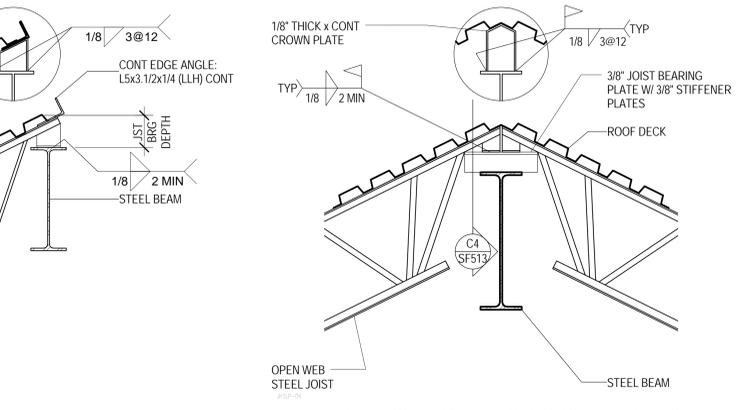
D5 STEEL BEAM CONNECTION TO STEEL BEAM WITH ANGLE KICKERS
SF512 NO SCALE



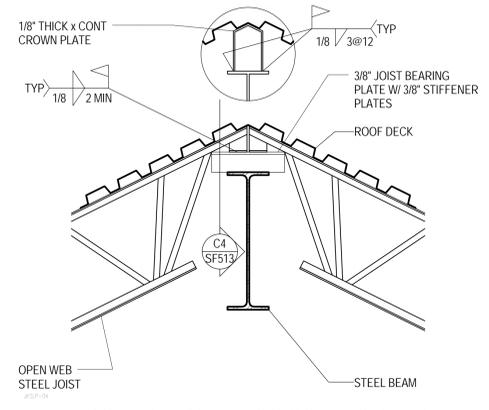
C1 TYPICAL STEEL JOIST BEARING AT STEEL BEAM
SF512 NO SCALE



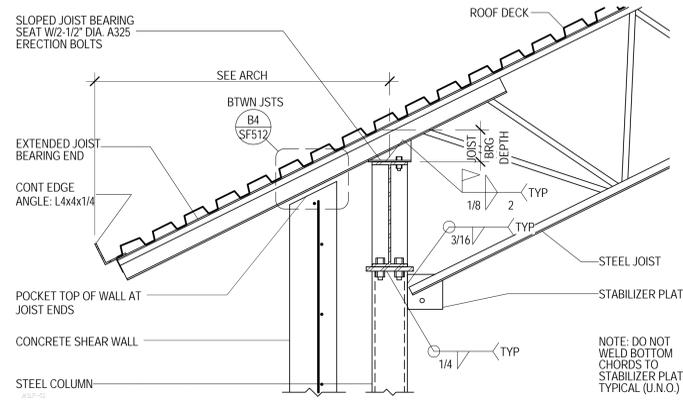
C3 STEEL BEAM TO STEEL COLUMN CONNECTION
SF512 NO SCALE



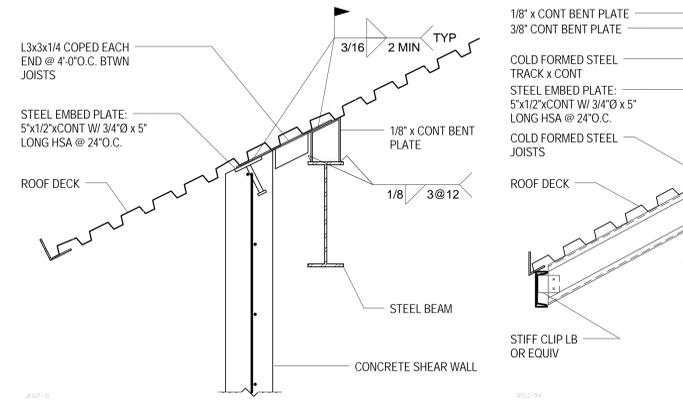
C4 STEEL JOIST TO STEEL BEAM
SF512 NO SCALE



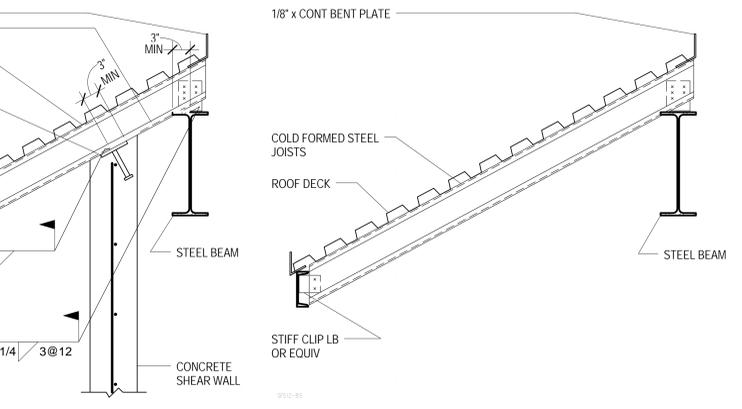
C5 TYPICAL BOLTED STEEL JOISTS CONNECTION TO STEEL RIDGE BEAM
SF512 NO SCALE



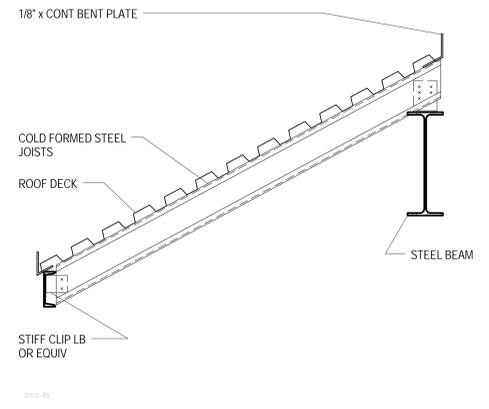
B1 TYPICAL STEEL JOIST BEARING AT STEEL COLUMN
SF512 NO SCALE



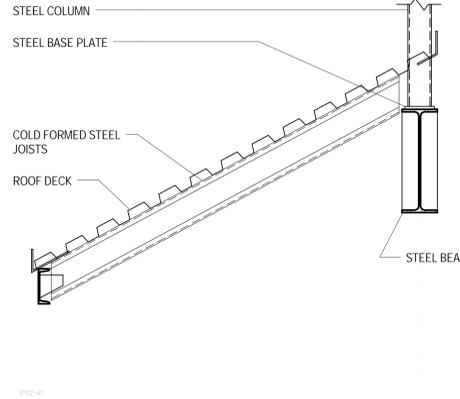
B3 TYPICAL ROOF DECK BEARING AT CONCRETE SHEAR WALL
SF512 NO SCALE



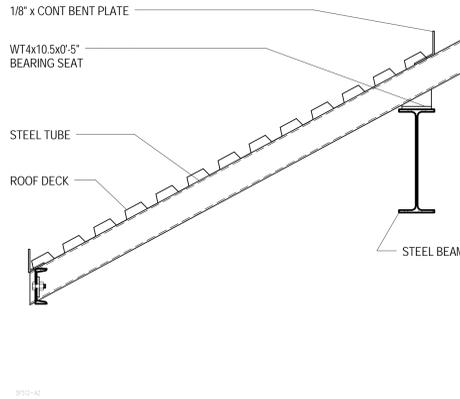
B4 TYPICAL STEEL JOIST CONNECTION TO CONCRETE WALL
SF512 NO SCALE



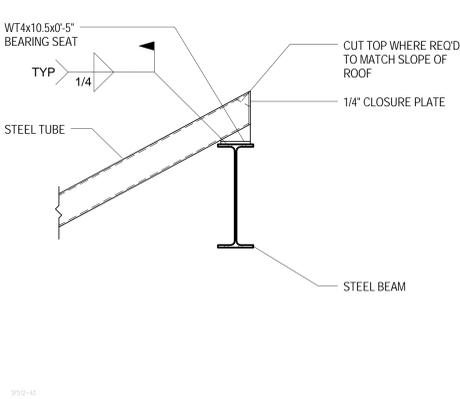
B5 TYPICAL STEEL JOIST CONNECTION TO STEEL BEAM
SF512 NO SCALE



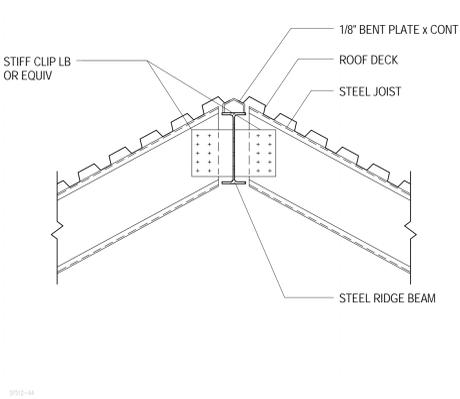
A1 TYPICAL STEEL BEAM CONNECTION TO STEEL BEAM
SF512 NO SCALE



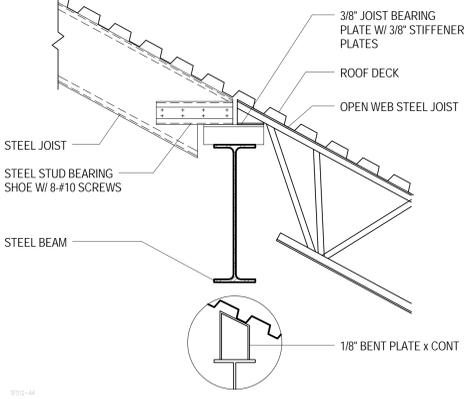
A2 TYPICAL STEEL TUBE CONNECTION TO STEEL BEAM
SF512 NO SCALE



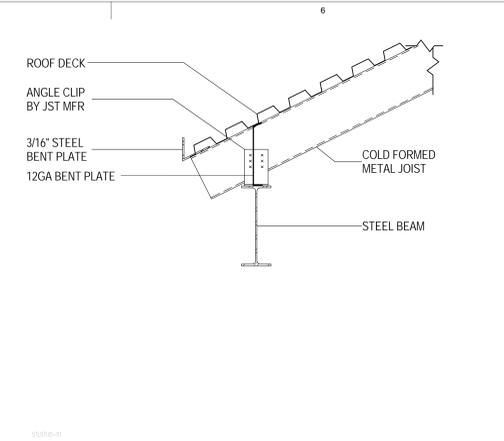
A3 TYPICAL STEEL TUBE CONNECTION TO STEEL BEAM
SF512 NO SCALE



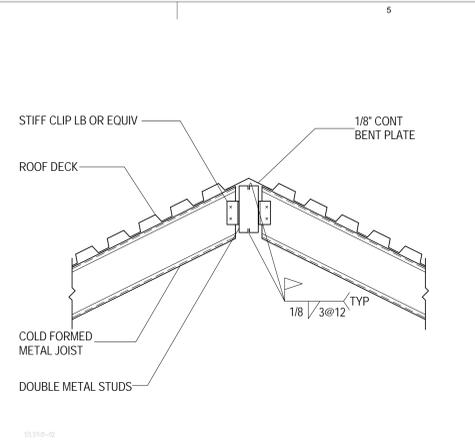
A4 TYPICAL STEEL JOIST CONNECTION TO STEEL BEAM AT RIDGE
SF512 NO SCALE



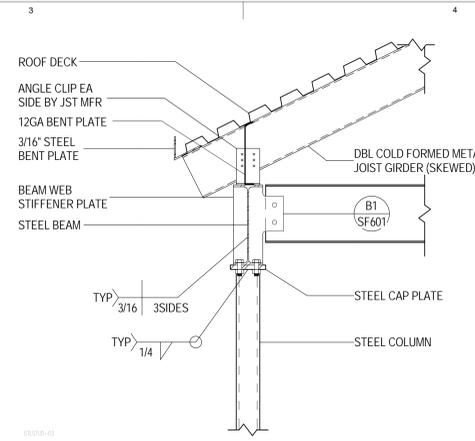
A5 TYPICAL STEEL JOIST CONNECTION TO STEEL BEAM
SF512 NO SCALE



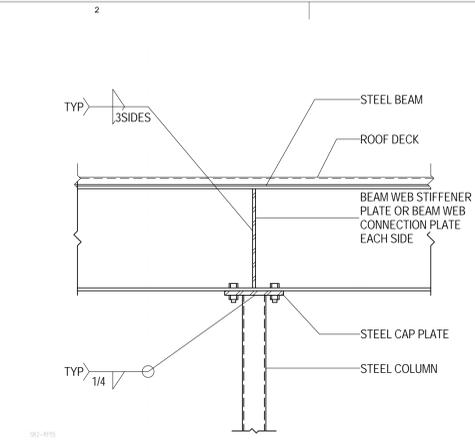
D5
SF513
METAL STUD JOIST TO STEEL BEAM
NO SCALE



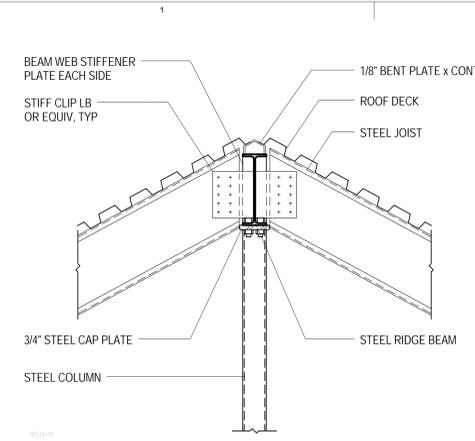
D4
SF513
METAL STUD JOIST BEARING AT PEAK
NO SCALE



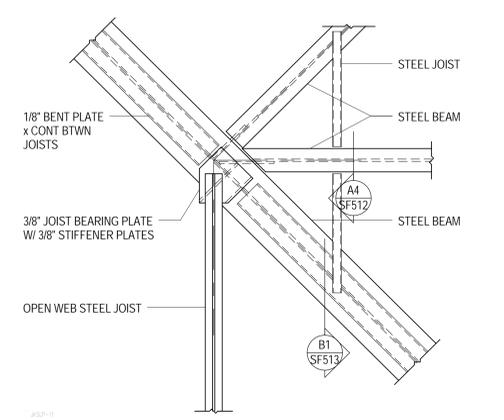
D3
SF513
METAL STUD JOIST TO STEEL BEAM
NO SCALE



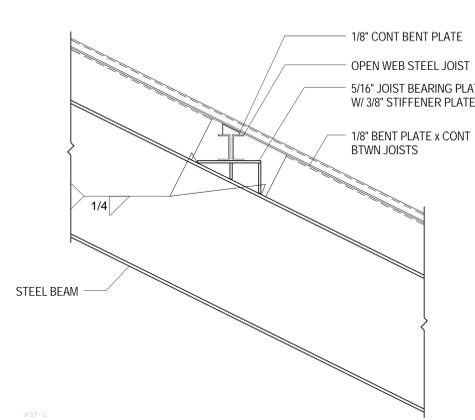
D2
SF513
TYPICAL STEEL BEAM TO STEEL COLUMN
NO SCALE



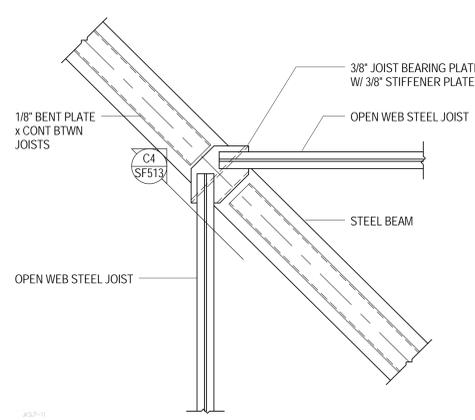
D1
SF513
TYPICAL STEEL BEAM CONNECTION TO STEEL BEAM AT RIDGE
NO SCALE



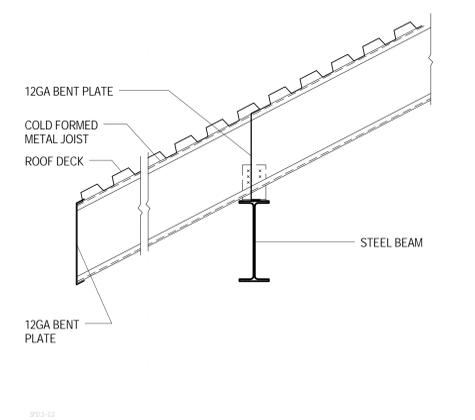
C5
SF513
TYPICAL JOIST & BEAM BEARING AT STEEL RIDGE BEAM - PLAN VIEW
NO SCALE



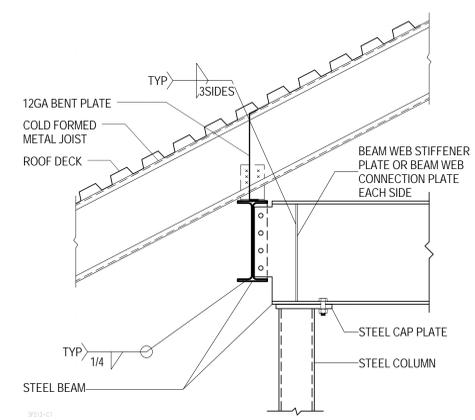
C4
SF513
TYPICAL JOIST BEARING AT STEEL RIDGE BEAM
NO SCALE



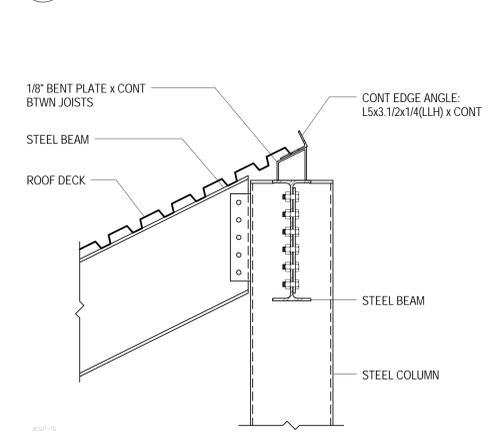
C3
SF513
TYPICAL JOIST BEARING AT STEEL RIDGE BEAM - PLAN VIEW
NO SCALE



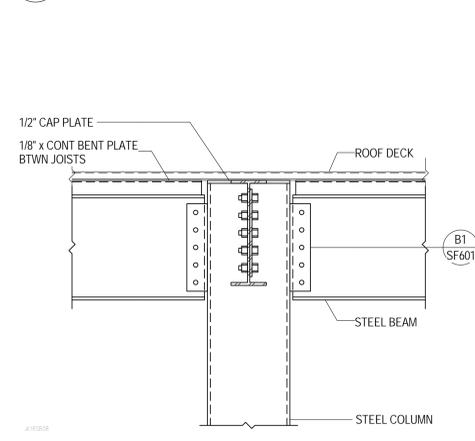
C2
SF513
TYPICAL CANTILEVERED STEEL JOIST BEARING AT STEEL BEAM
NO SCALE



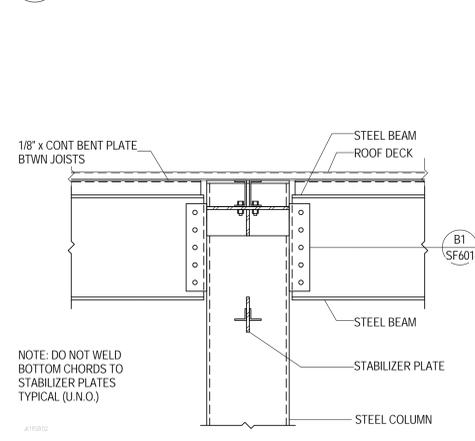
C1
SF513
TYPICAL CANTILEVERED STEEL BEAM CONNECTION AT STEEL COLUMN
NO SCALE



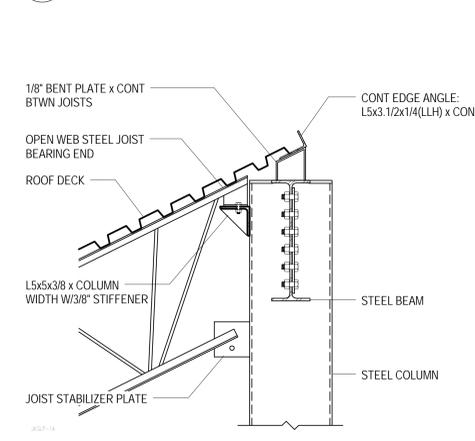
B5
SF513
TYPICAL STEEL BEAM TO STEEL COLUMN
NO SCALE



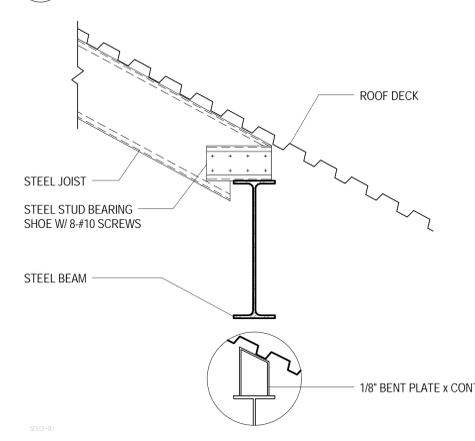
B4
SF513
TYPICAL STEEL BEAM CONNECTION TO STEEL COLUMN
NO SCALE



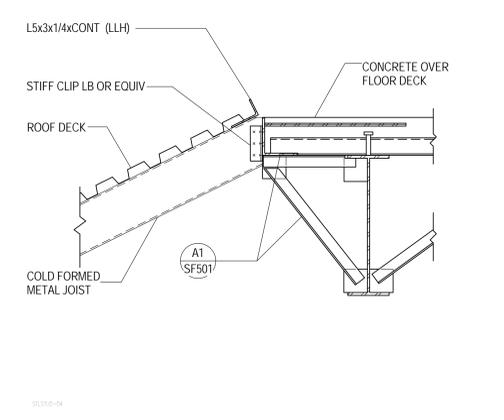
B3
SF513
TYPICAL STEEL BEAM CONNECTION TO STEEL COLUMN
NO SCALE



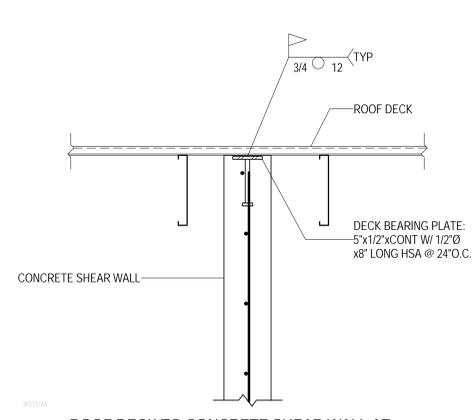
B2
SF513
TYPICAL STEEL JOIST TO STEEL COLUMN
NO SCALE



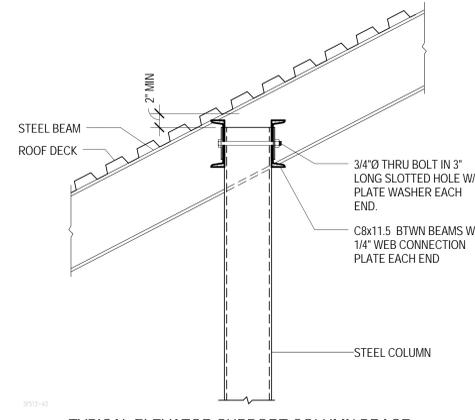
B1
SF513
TYPICAL STEEL JOIST CONNECTION TO STEEL BEAM
NO SCALE



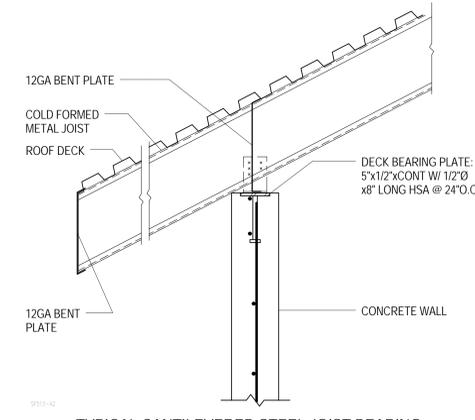
A5
SF513
METAL STUD JOIST TO STEEL BEAM
NO SCALE



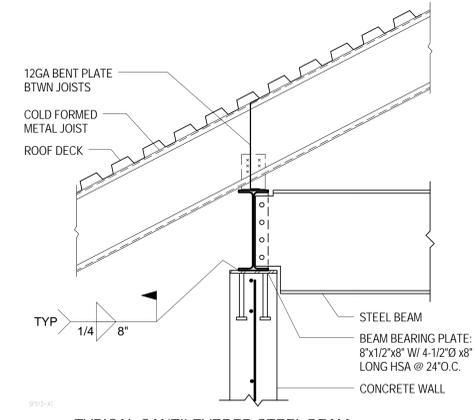
A4
SF513
ROOF DECK TO CONCRETE SHEAR WALL AT GABLE ROOF/ROOF
NO SCALE



A3
SF513
TYPICAL ELEVATOR SUPPORT COLUMN BRACE AT ROOF
NO SCALE



A2
SF513
TYPICAL CANTILEVERED STEEL JOIST BEARING AT CONCRETE WALL
NO SCALE



A1
SF513
TYPICAL CANTILEVERED STEEL BEAM CONNECTION AT CONCRETE WALL
NO SCALE



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PROJECT NAME:

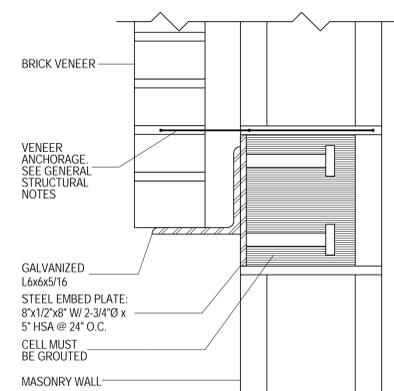
Snow College Library

**150 College Avenue
Ephraim, Utah 84627**

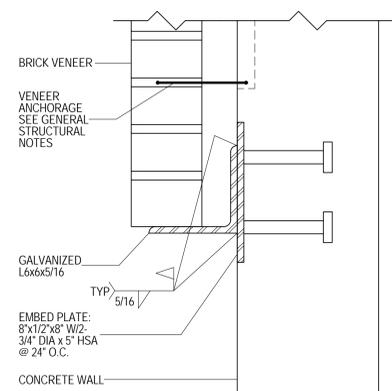
REVISIONS:

NO.	DATE	DESCRIPTION
1	SEPTEMBER 15, 2008	100% CD
2	AUGUST 4, 2008	100% CD Review
ISSUE DATE:		
SEPTEMBER 15, 2008, 100% CD		
ARCHITECT'S PROJECT NUMBER:		
B07-051		
DFCM PROJECT NUMBER:		
07258700		
SHEET TITLE:		
Structural Schedules		
SHEET NUMBER:		
SF602		

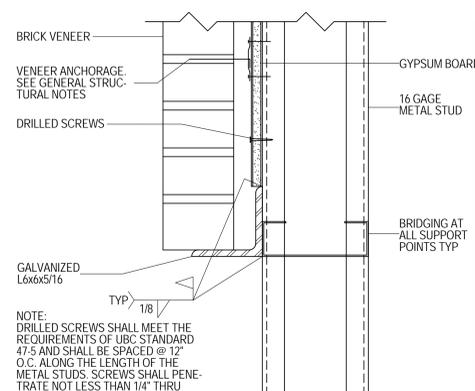
STEEL ANGLE LINTEL SCHEDULE		
BEARING EQUALS 1" PER FOOT OF OPENING OR 6" MINIMUM TYPICAL		
OPENING SIZE	ANGLE SIZE	NOTES:
0'-0" - 7'-0"	L3.1/2x3.1/2x1/4	LINTELS ARE DESIGNED TO SUPPORT UNIFORM LOADS CONSISTING ONLY OF WEIGHT OF WALL WITHIN A 60 DEGREE ISOSCELES TRIANGLE AREA ABOVE OPENING. ALL STEEL LINTELS ARE TO HAVE LONG LEG VERTICAL. LINTEL SCHEDULE FOR 4" VENEER ALL ANGLE LINTELS SHALL BE GALVANIZED.
7'-0" - 9'-0"	L4x3.1/2x1/4	
9'-0" - 10'-0"	L5x3.1/2x1/4	



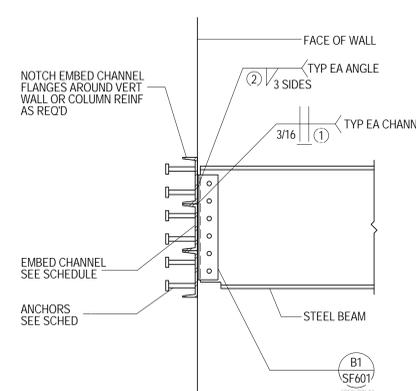
A1
TYPICAL BRICK LEDGER DETAIL AT MASONRY WALL
NO SCALE



A2
TYPICAL BRICK LEDGER DETAIL AT CONCRETE WALL
NO SCALE



A3
TYPICAL BRICK LEDGER DETAIL AT METAL STUD WALL
NO SCALE

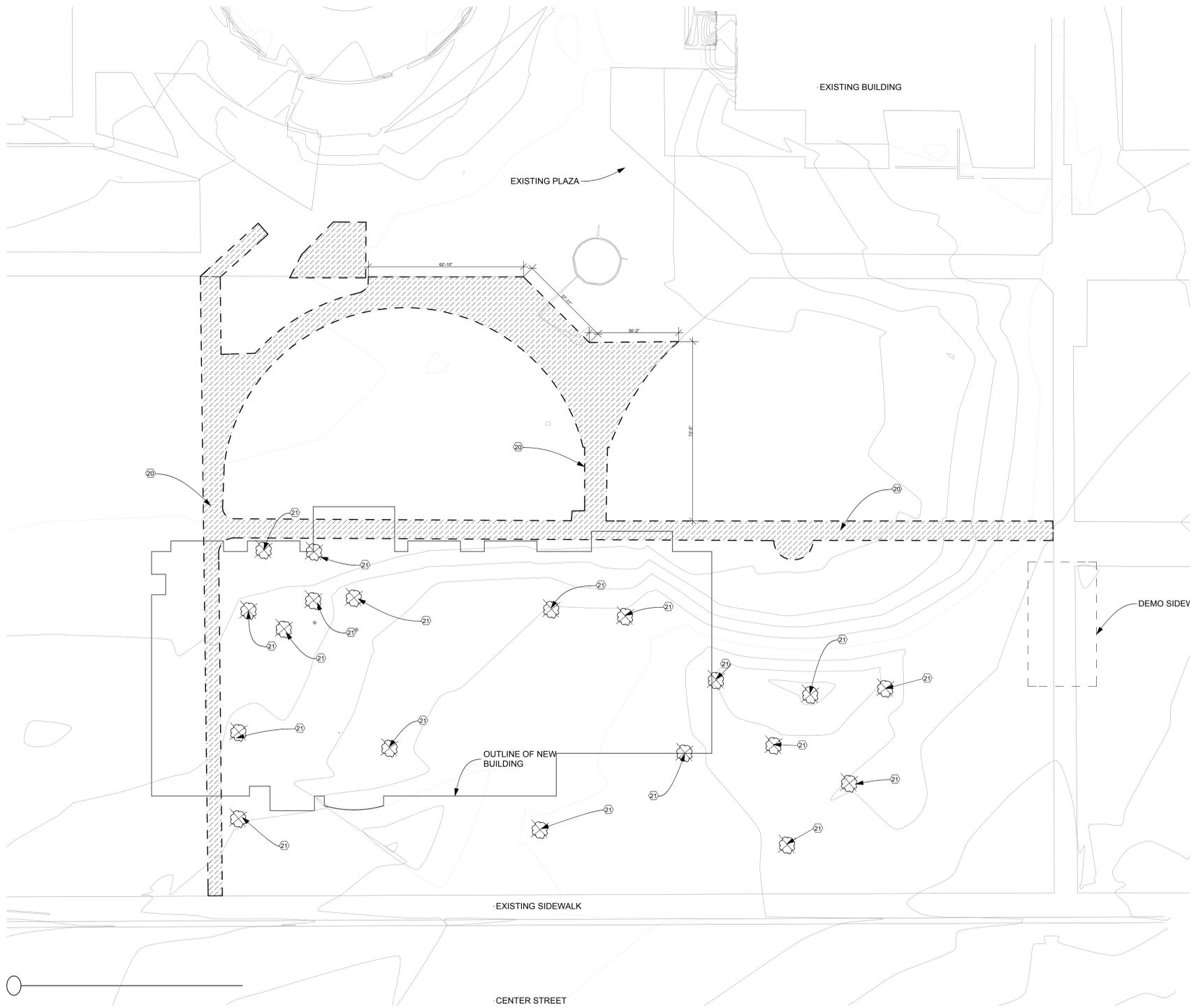


A4
TYPICAL EMBED CHANNEL CONNECTION SCHEDULE
NO SCALE

CONNECTION SCHEDULE		
BEAM DEPTH	EMBED CHANNEL	ANCHORS
TO 10"	1-C8x18.75x12"	4-3/4"Ø x 5" STUDS
TO 16"	2-C8x18.75x12"	8-3/4"Ø x 5" STUDS
TO 24"	3-C8x18.75x12"	12-3/4"Ø x 5" STUDS

① WELD MULTIPLE EMBEDS TOGETHER WITH BUTT WELD EACH SIDE, FULL LENGTH
② FILLET WELDS SHALL BE ANGLE THICKNESS MINUS 1/16" (1/4" MIN.)

GENERAL NOTES	KEYED NOTES
<p>A. REFER TO SNOW COLLEGE STANDARDS</p> <p>B. REFER TO LANDSCAPE & CIVIL DRAWINGS FOR EXTENT OF SELECTED DEMOLITION FOR ALL ADD. ALTERNATIVES</p>	<ol style="list-style-type: none"> 1. NEW SIDEWALK 2. BIKE RACK LOCATION 3. ELECTRICAL EQUIPMENT ENCLOSURE 4. DUMPSTER ENCLOSURE, SEE AS103 FOR DETAILS 5. CHILLER ENCLOSURE, SEE AS103 FOR DETAILS 6. TUNNEL, SEE MECHANICAL 7. ADA RAMP, SEE DETAIL A2/AS104 8. PARKING SPOTS RESERVED FOR ALTERNATIVE FEUL VEHICLES, SEE SIGN DETAIL A6/AS104 AND B4/AS104 9. PARKING SPOTS RESERVED FOR CARPOOL VEHICLES, SEE SIGN DETAIL A6/AS104 AND B4/AS104 10. BOUNDARY, BASE PROJECT SCOPE 11. BOUNDARY, ADD ALTERNATE A 12. RETAINING WALLS- SEE SHEET C1.1 FOR DETAILS 13. DOWNSPOUT LOCATIONS- SEE ROOF PLANS 14. ROOF DRAINS - SEE ROOF PLANS 15. LAND DRAINS - SEE SHEET C3.1 FOR DETAILS 16. CATCH BASINS - SEE SHEET C3.1 FOR DETAILS 17. FIRE HYDRANT 18. FLOOR DRAINS - SEE SHEET C3.1 FOR DETAILS 19. LIGHT BOLLARD - SEE ES101 FOR DETAILS 20. SIDEWALK TO BE REMOVED 21. TREES TO BE REMOVED



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PROJECT NAME:
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REVISIONS:

100% CD, September 15, 2008
100% CD Review, August 4, 2008

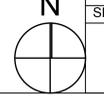
ISSUE DATE:
SEPTEMBER 15 2008, 100% CD

ARCHITECT'S PROJECT NUMBER:
B07-051

DFCM PROJECT NUMBER:
07258700

SHEET TITLE:
Architectural Demo Plan

SHEET NUMBER:
AD101



GENERAL NOTES

A. REFER TO SNOW COLLEGE STANDARDS
 B. REFER TO LANDSCAPE & CIVIL DRAWINGS FOR EXTENT OF SELECTED DEMOLITION FOR ALL ADD. ALTERNATIVES

KEYED NOTES

1. NEW SIDEWALK
2. BIKE RACK LOCATION
3. ELECTRICAL EQUIPMENT ENCLOSURE
4. DUMPSTER ENCLOSURE, SEE AS103 FOR DETAILS
5. CHILLER ENCLOSURE, SEE AS103 FOR DETAILS
6. TUNNEL, SEE MECHANICAL
7. ADA RAMP, SEE DETAIL A2/AS104
8. PARKING SPOTS RESERVED FOR ALTERNATIVE FEUL VEHICLES, SEE SIGN DETAIL A6/AS104 AND B4/AS104
9. PARKING SPOTS RESERVED FOR CARPOOL VEHICLES, SEE SIGN DETAIL A6/AS104 AND B4/AS104.
10. BOUNDARY, BASE PROJECT SCOPE
11. BOUNDARY, ADD ALTERNATE A
12. RETAINING WALLS - SEE SHEET C1.1 FOR DETAILS
13. DOWNSPOUT LOCATIONS - SEE ROOF PLANS
14. ROOF DRAINS - SEE ROOF PLANS
15. LAND DRAINS - SEE SHEET C3.1 FOR DETAILS
16. CATCH BASINS - SEE SHEET C3.1 FOR DETAILS
17. FIRE HYDRANT
18. FLOOR DRAINS - SEE SHEET C3.1 FOR DETAILS
19. LIGHT BOLLARD - SEE ES101 FOR DETAILS
20. SIDEWALK TO BE REMOVED
21. TREES TO BE REMOVED

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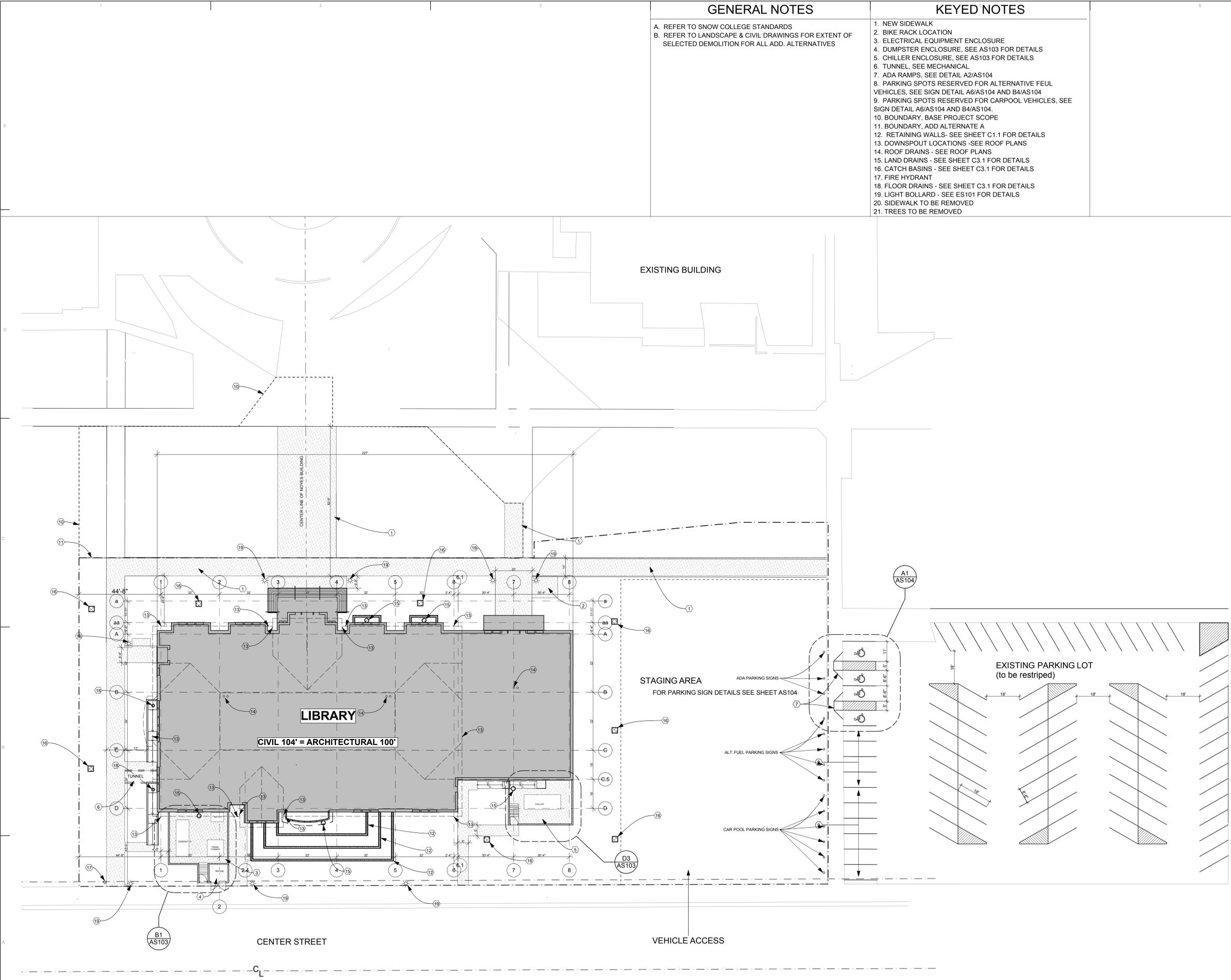
REVISIONS:

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ISSUE DATE:

SEPTEMBER 15 2008, 100% CD
ARCHITECT'S PROJECT NUMBER:
 B07-051
DFCM PROJECT NUMBER:
 07258700
SHEET TITLE:

Architectural Site Plan, Add. Alt Scheme A

SHEET NUMBER:
 AS101



1 Architectural Site Plan, Add Alt. 1, Scheme A
 SCALE: 1" = 20'



P:\007-051 Snow College Library\DRAWING\Architect\B07-051 Interior site plan.rvt
 2008-10-27



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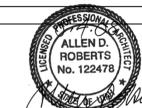
AV Consultant

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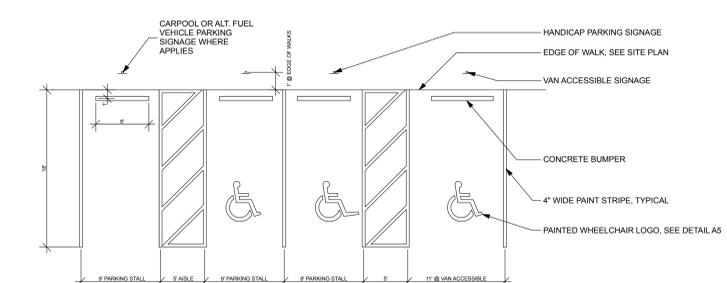
PROJECT NAME:

Snow College Library

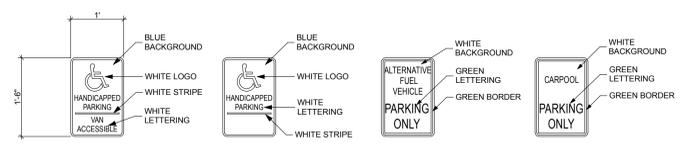
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Ephraim, Utah 84627**

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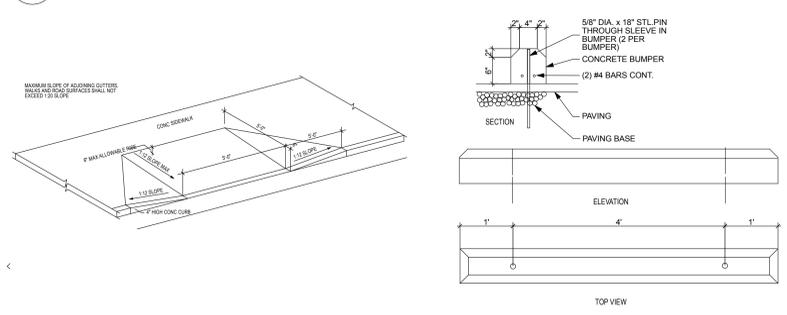
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100% CD Review	August 4, 2008	
ISSUE DATE:		
SEPTEMBER 15 2008, 100% CD		
ARCHITECT'S PROJECT NUMBER:		
B07-051		
DFCM PROJECT NUMBER:		
07258700		
SHEET TITLE:		
Architectural Site Details		
SHEET NUMBER:		
AS104		



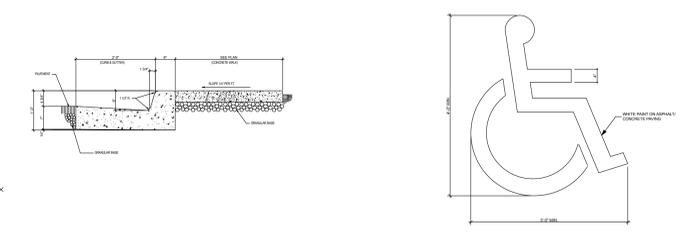
B2 Accessible Parking Typical Details
SCALE: 1/8" = 1'-0"



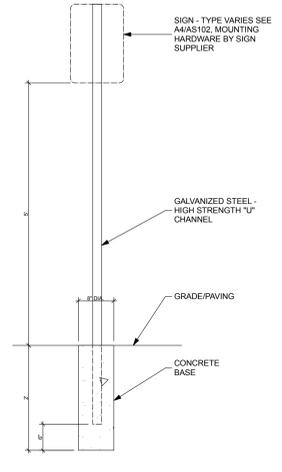
B4 SCALE: 3/4" = 1'-0"



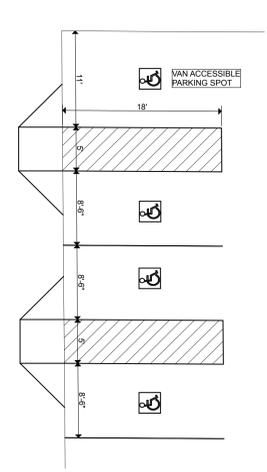
A3 Parking Stall Bumper Details
SCALE: 3/4" = 1'-0"



A4 Sidewalk Curb & Gutter Detail
SCALE: 1/2" = 1'-0"



A6 SCALE: 3/4" = 1'-0"



A1 Accessible Parking Enlarged
SCALE: 1/8" = 1'-0"

A2 Accessible Ramp Detail
SCALE: 6" = 1'-0"



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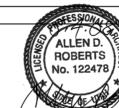
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PROJECT NAME:

Snow College Library

**150 College Avenue
Ephraim, Utah 84627**

REVISIONS:

NO.	DATE	DESCRIPTION

100% CD, September 15, 2008
100% CD Review, August 4, 2008

ISSUE DATE:

SEPTEMBER 15 2008, 100% CD

ARCHITECT'S PROJECT NUMBER:

B07-051

DFCM PROJECT NUMBER:

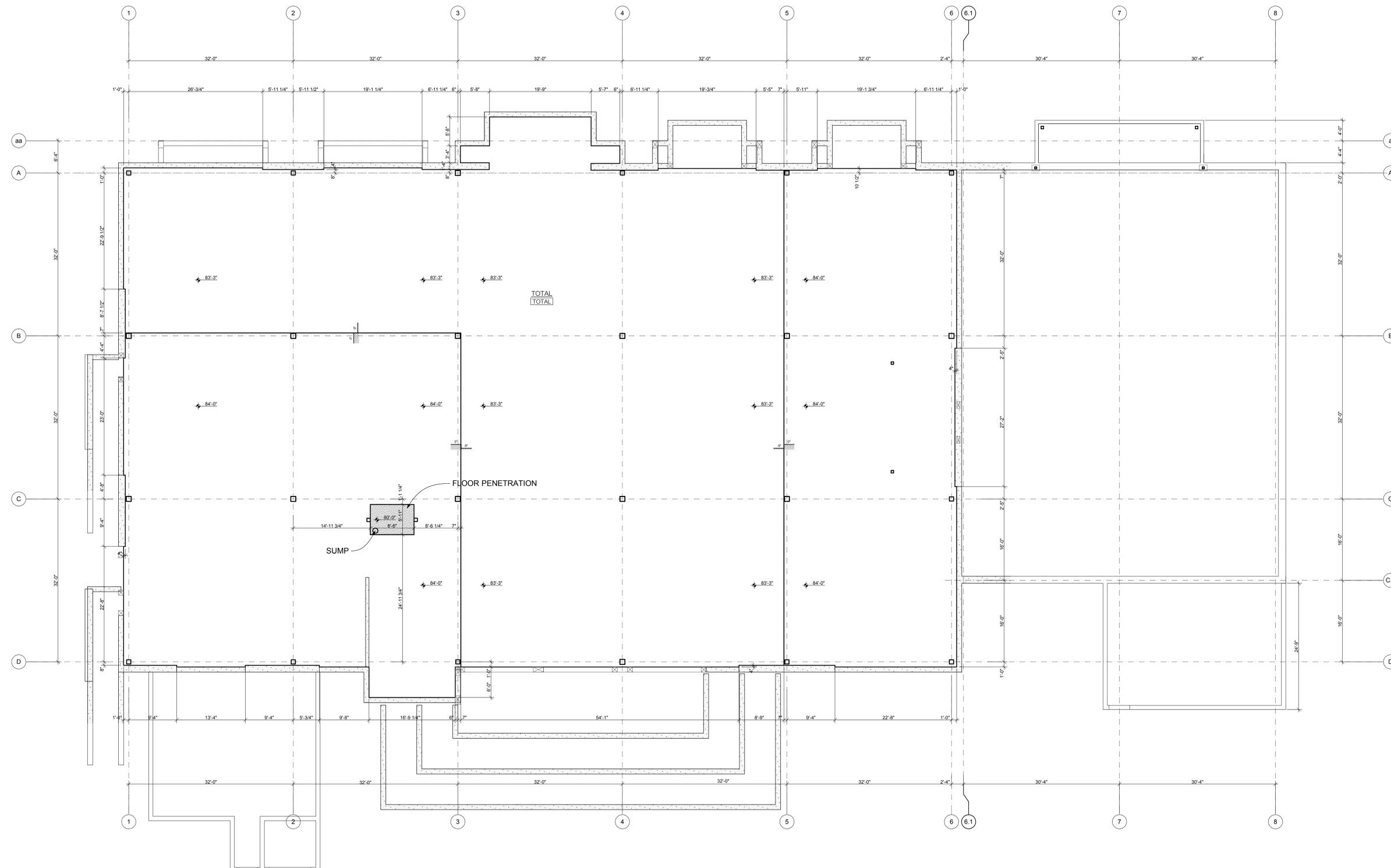
07258700

SHEET TITLE:

Basement Level Slab Edge Plan

SHEET NUMBER:

AE101



1 Basement Slab Edge Plan
SCALE: 1/8" = 1'-0"



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PROJECT NAME:
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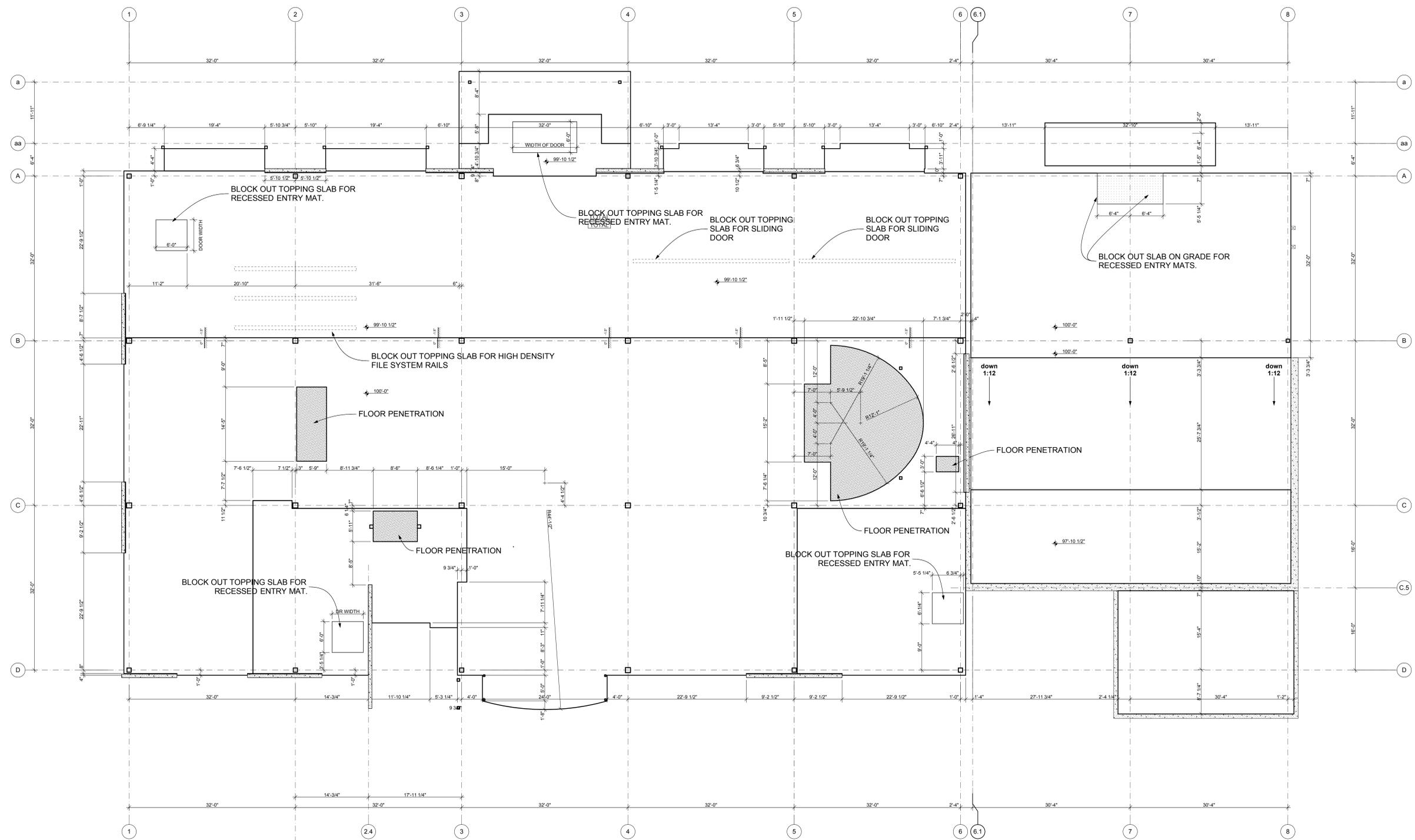
SEPTEMBER 15 2008, 100% CD

ARCHITECT'S PROJECT NUMBER:
B07-051

DFCM PROJECT NUMBER:
07258700

SHEET TITLE:
Level 1 Slab Edge Plan

SHEET NUMBER:
AE102



1 L1, Main Level Slab Edge Plan
SCALE: 1/8" = 1'-0"



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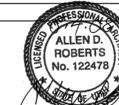
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ARCHITECT'S PROJECT NUMBER:

B07-051

DFCM PROJECT NUMBER:

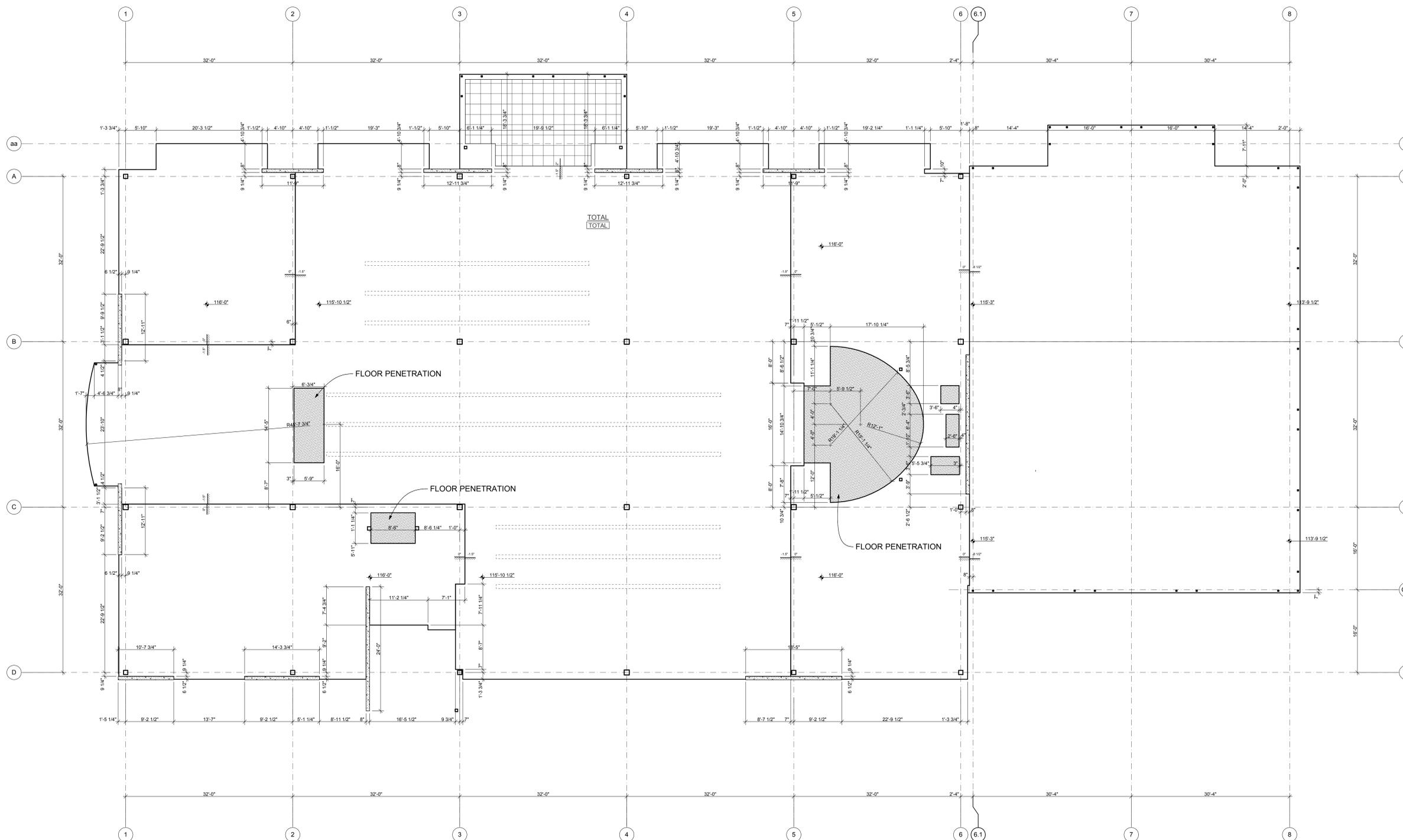
07258700

SHEET TITLE:

Level 2 Slab Edge Plan

SHEET NUMBER:

AE103



1 L2 Slab Edge Plan

SCALE: 1/8" = 1'-0"



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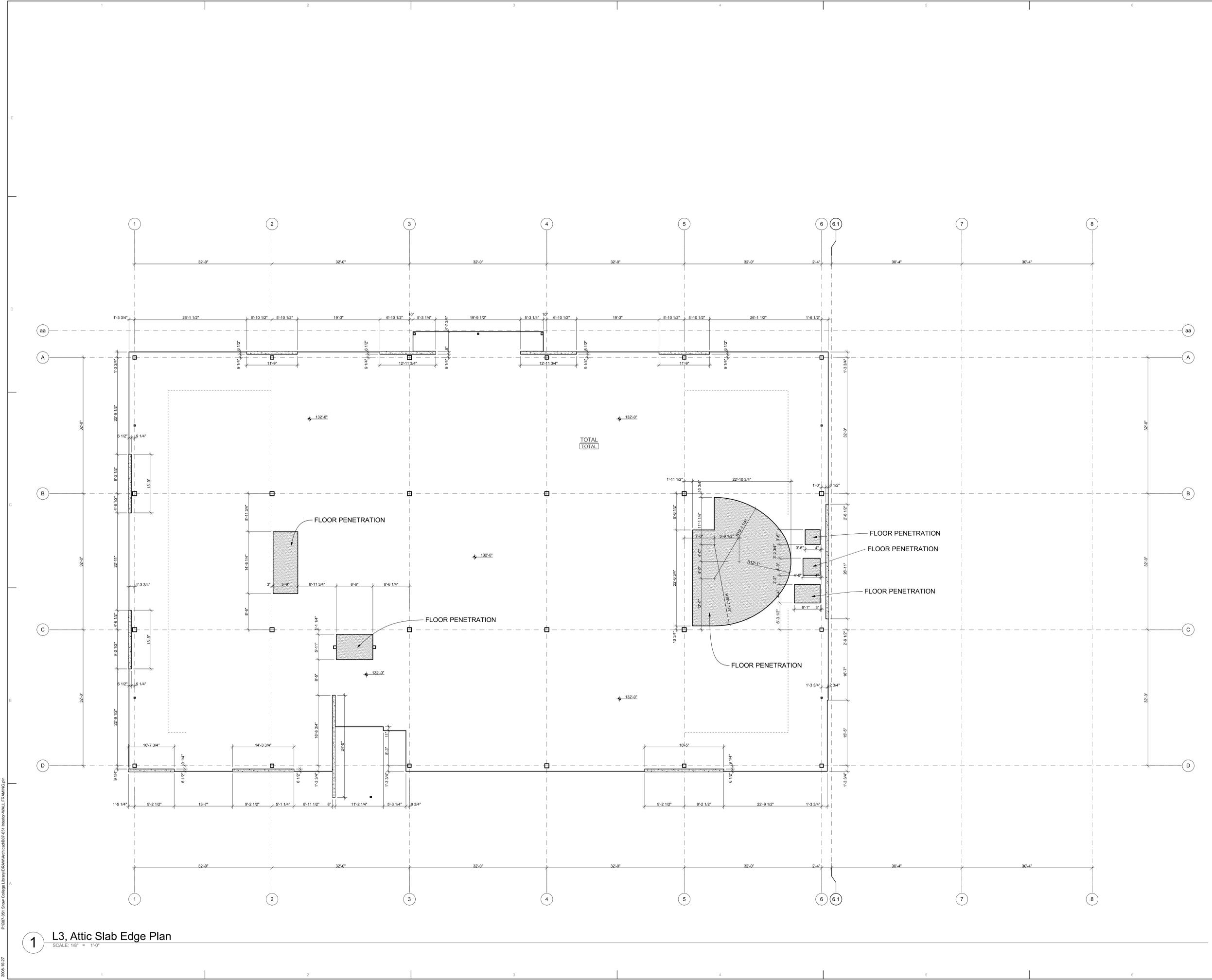
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ARCHITECT'S PROJECT NUMBER:
B07-051
DFCM PROJECT NUMBER:
07258700
SHEET TITLE:
Level 3 Slab Edge Plan
SHEET NUMBER:
AE104



1 L3, Attic Slab Edge Plan
SCALE: 1/8" = 1'-0"

FLOOR PLAN KEYED NOTES

1. ROLL DOWN FIRE GATE
2. SECTIONAL PARTITION. SEE DETAIL B5, B6/AE604
3. SEMI-PERMANENT PARTITIONS
4. 7" TALL GYP. BOARD WALL
5. GLASS GUARD RAIL
6. HIGH DENSITY COLLECTIONS
7. FUTURE HIGH DENSITY COLLECTION LOCATION
8. SLIDING DOOR.
9. BOOK RETURN SEE DETAIL E1/AE506.
10. TEMPORARY GYP. BOARD WALL
11. AIR HANDLER. SEE MECHANICAL
12. BENCH SEE DETAIL D1/AE408
13. WINDOW WELL
14. WALK OFF MATS, FLUSH
15. ART WALL SEE DETAILS A4, A6, B4, B5, B6, C5/AE507
16. ALTERNATING TREAD, 6" X 3" SEE DETAIL B4/AE506
17. THEFT DETERRENT DEVICES
18. FIRE EXTINGUISHER CABINET. SEE DETAIL B4, B5/ AE502
19. EXTERIOR BOOK DROP LOCATION. (BY OTHERS.)
20. DISCHARGE IDENTIFICATION BARRIER. (MAG. HOLD OPEN)
21. ROLL DOWN SECURITY GATE.
22. 90 MIN. RATED 24"X36" ACCESS HATCH.

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No. 122478

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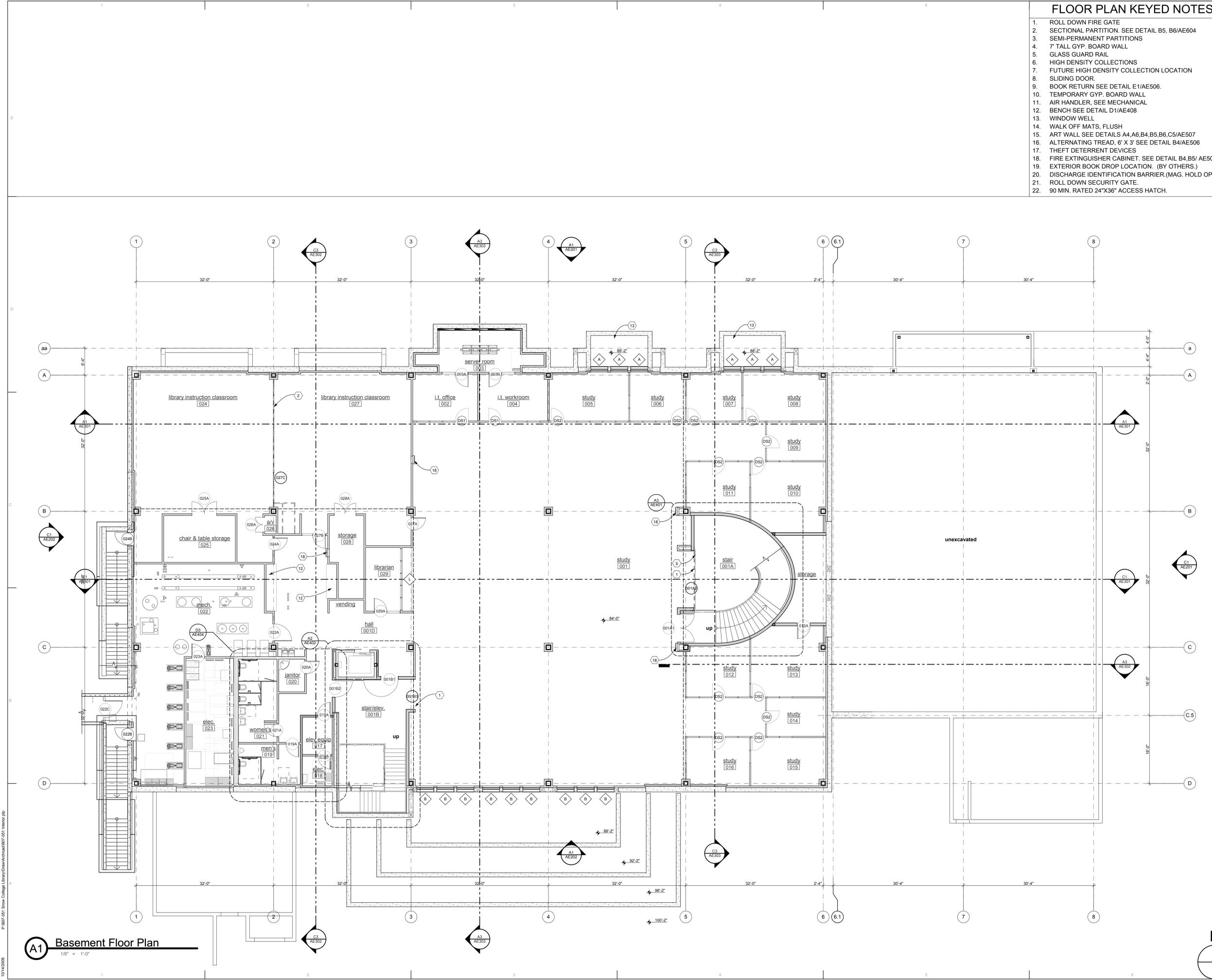
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ISSUE DATE:
SEPTEMBER 15 2008, 100% CD

ARCHITECT'S PROJECT NUMBER:
B07-051
DFCM PROJECT NUMBER:
07258700

SHEET TITLE:
Basement Level, Floor Plan

SHEET NUMBER:
AE105



A1 Basement Floor Plan
1/8" = 1'-0"

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FLOOR PLAN KEYED NOTES

1. ROLL DOWN FIRE GATE
2. SECTIONAL PARTITION. SEE DETAIL B5, B6/AE604
3. SEMI-PERMANENT PARTITIONS
4. 7" TALL GYP. BOARD WALL
5. GLASS GUARD RAIL
6. HIGH DENSITY COLLECTIONS
7. FUTURE HIGH DENSITY COLLECTION LOCATION
8. SLIDING DOOR.
9. BOOK RETURN SEE DETAIL E1/AE506.
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15. ART WALL SEE DETAILS A4, A6, B4, B5, B6, C5/AE507
16. ALTERNATING TREAD, 6" X 3" SEE DETAIL B4/AE506
17. THEFT DETERRENT DEVICES
18. FIRE EXTINGUISHER CABINET. SEE DETAIL B4, B5/ AE502
19. EXTERIOR BOOK DROP LOCATION. (BY OTHERS.)
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PROJECT NAME:
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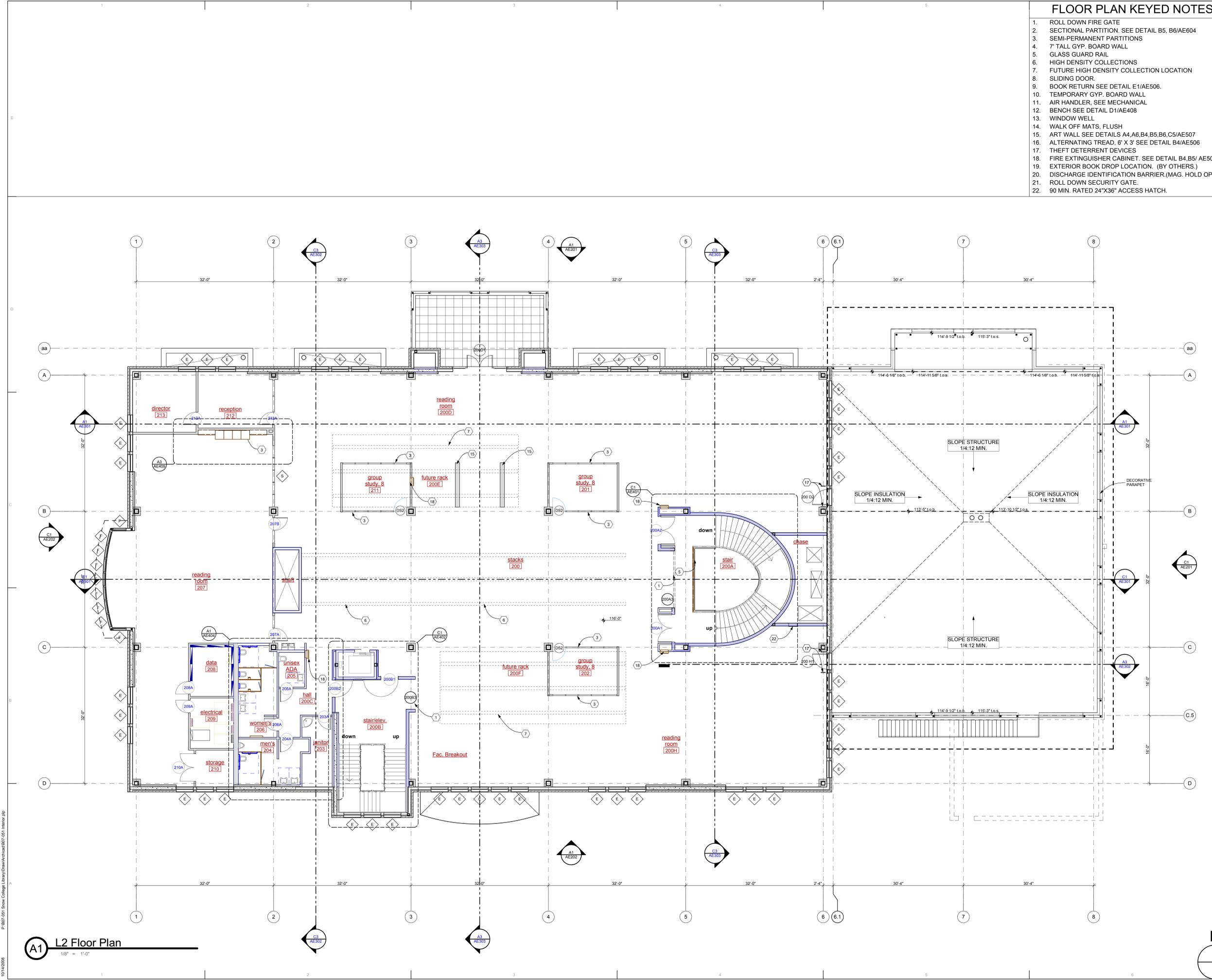
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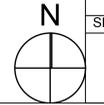
DFCM PROJECT NUMBER:
07258700

SHEET TITLE:
Level 2, Floor Plan

SHEET NUMBER:
AE107



A1 L2 Floor Plan
1/8" = 1'-0"



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FLOOR PLAN KEYED NOTES

1. ROLL DOWN FIRE GATE
2. SECTIONAL PARTITION. SEE DETAIL B5, B6/AE604
3. SEMI-PERMANENT PARTITIONS
4. 7" TALL GYP. BOARD WALL
5. GLASS GUARD RAIL
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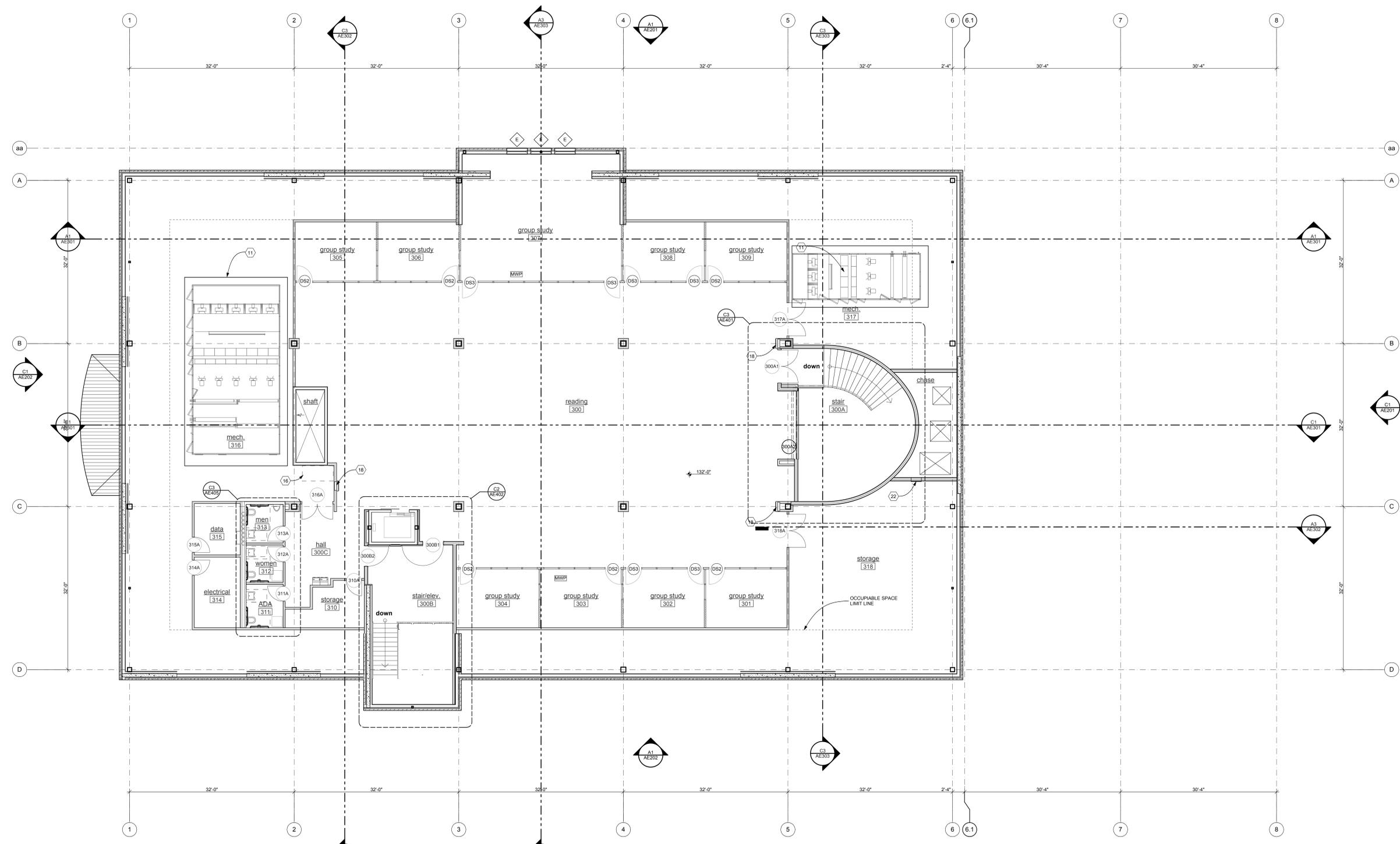
PROJECT NAME:
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REVISIONS:
100% CD, September 15, 2008
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ISSUE DATE:
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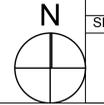
ARCHITECT'S PROJECT NUMBER:
B07-051
DFCM PROJECT NUMBER:
07258700

SHEET TITLE:
Level 3, Attic Add Alt, Floor Plan

SHEET NUMBER:
AE108a



A1 L3 Add Alt Floor Plan
1/8" = 1'-0"



10/14/2008 P:\B07-051 Snow College Library\Draw\Arch\A108\A108a.rvt Interior.rvt



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STAMP:



PROJECT NAME:

Snow College Library

**150 College Avenue
Ephraim, Utah 84627**

REVISIONS:

NO.	DATE	DESCRIPTION

100% CD, September 15, 2008
100% CD Review, August 4, 2008

ISSUE DATE:

SEPTEMBER 15 2008, 100% CD

ARCHITECT'S PROJECT NUMBER:

B07-051

DFCM PROJECT NUMBER:

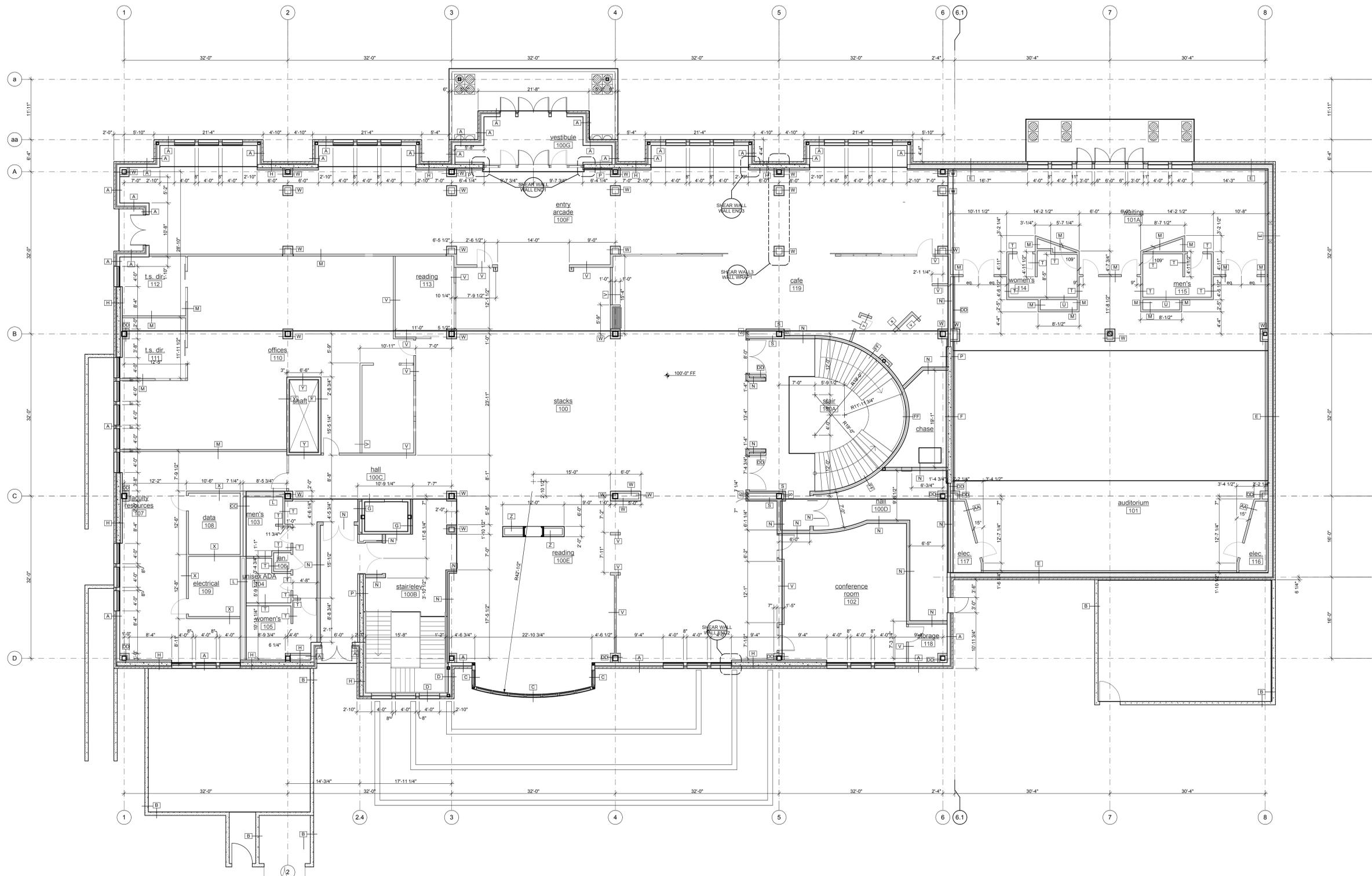
07258700

SHEET TITLE:

Level 1, Main Level Wall Framing

SHEET NUMBER:

AE110



1 LEVEL 1, MAIN LEVEL WALL FRAMING
SCALE: 1/8" = 1'-0"



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PROJECT NAME:
Snow College Library
150 College Avenue
Ephraim, Utah 84627

REVISIONS:

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100% CD Review, August 4, 2008
ISSUE DATE:

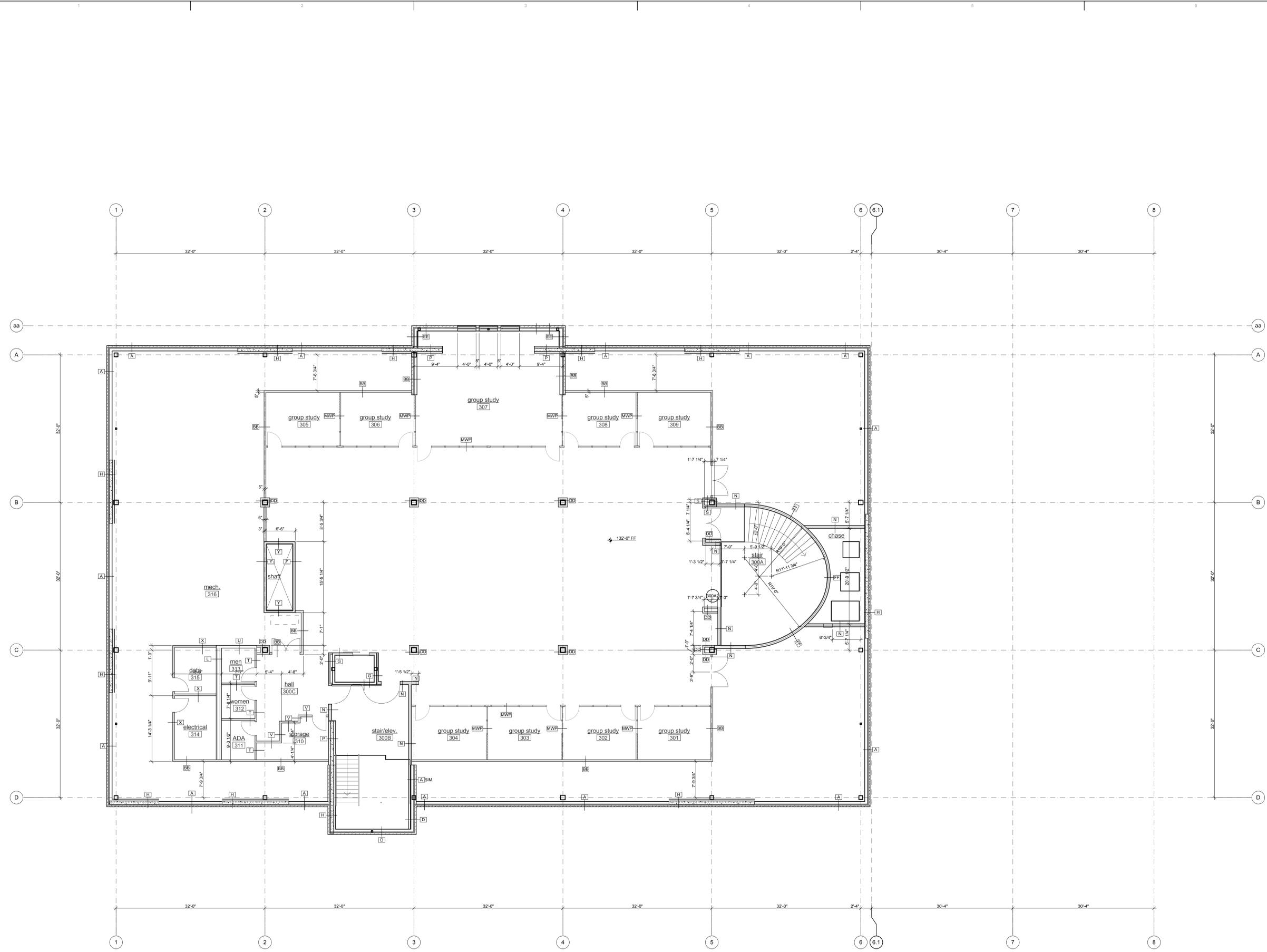
SEPTEMBER 15 2008, 100% CD

ARCHITECT'S PROJECT NUMBER:
B07-051

DFCM PROJECT NUMBER:
07258700

SHEET TITLE:
Level 3 Wall Framing

SHEET NUMBER:
AE112



1 LEVEL 3 WALL FRAMING
SCALE: 1/8" = 1'-0"



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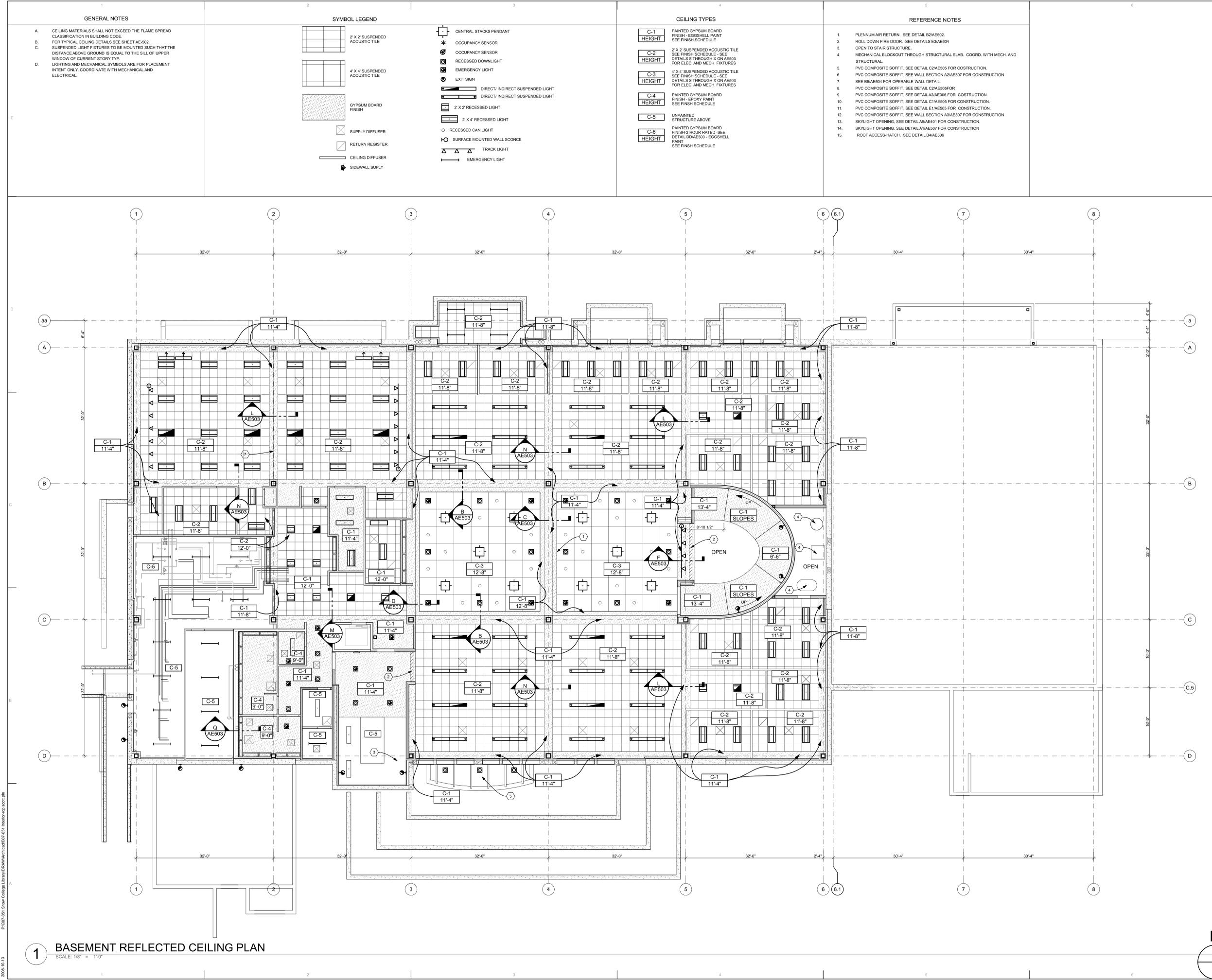
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Snow College Library
150 College Avenue
Ephraim, Utah 84627

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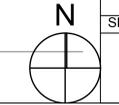
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100% CD Review, August 4, 2008
ISSUE DATE:
SEPTEMBER 15, 2008, 100% CD
ARCHITECT'S PROJECT NUMBER:
B07-051
DFCM PROJECT NUMBER:
07258700
SHEET TITLE:

Basement Level R C P

SHEET NUMBER:
AE113



1 BASEMENT REFLECTED CEILING PLAN
SCALE: 1/8" = 1'-0"





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PROJECT NAME:

Snow College Library
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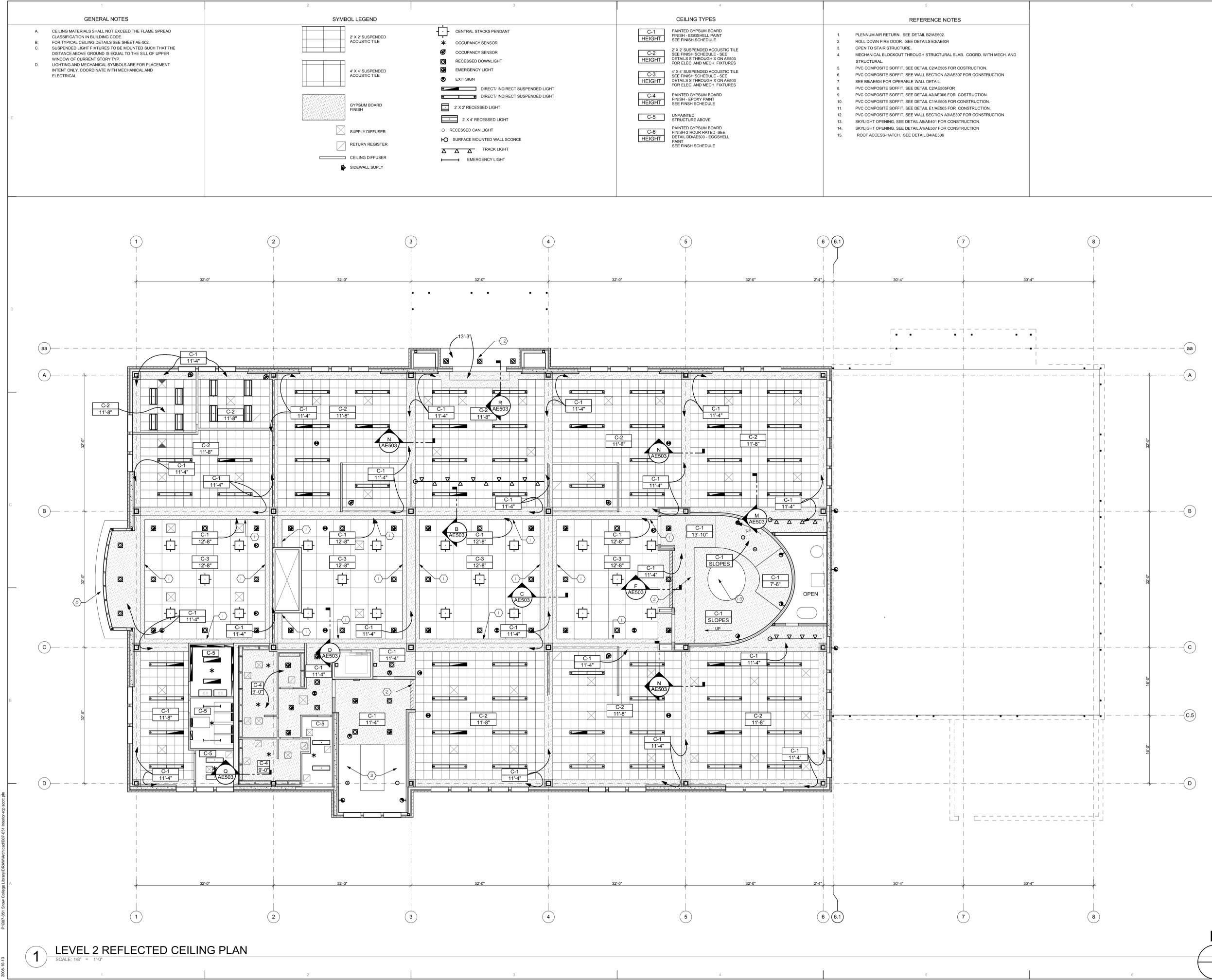
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100% CD Review, August 4, 2008
ISSUE DATE:
SEPTEMBER 15, 2008, 100% CD

ARCHITECT'S PROJECT NUMBER:
B07-051
DFCM PROJECT NUMBER:
07258700
SHEET TITLE:

Level 2, R C P
SHEET NUMBER:

AE115



1 LEVEL 2 REFLECTED CEILING PLAN
SCALE: 1/8" = 1'-0"

P:\007\001 Snow College Library\DRAWING\Architect\007_001 Interior.rvt north.plt
2008-10-13



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DIVISION OF FACILITIES CONSTRUCTION
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410 State Office Building Salt Lake City, Utah 84143-2000

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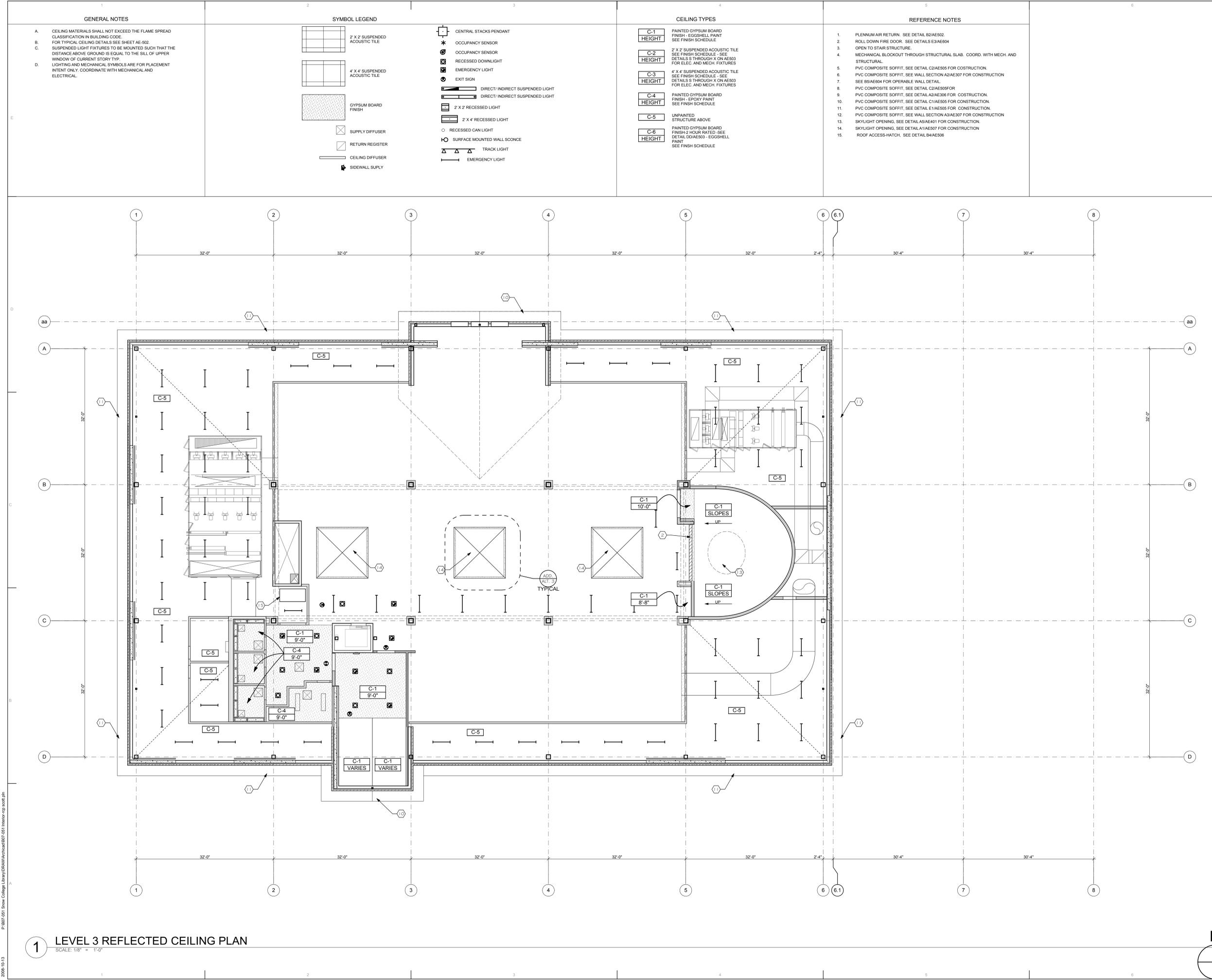
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ISSUE DATE:
SEPTEMBER 15, 2008, 100% CD

ARCHITECT'S PROJECT NUMBER:
B07-051
DFCM PROJECT NUMBER:
07258700
SHEET TITLE:

Level 3, Attic, R C P
SHEET NUMBER:

AE116



1 LEVEL 3 REFLECTED CEILING PLAN
SCALE: 1/8" = 1'-0"



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PROJECT NAME:
Snow College Library
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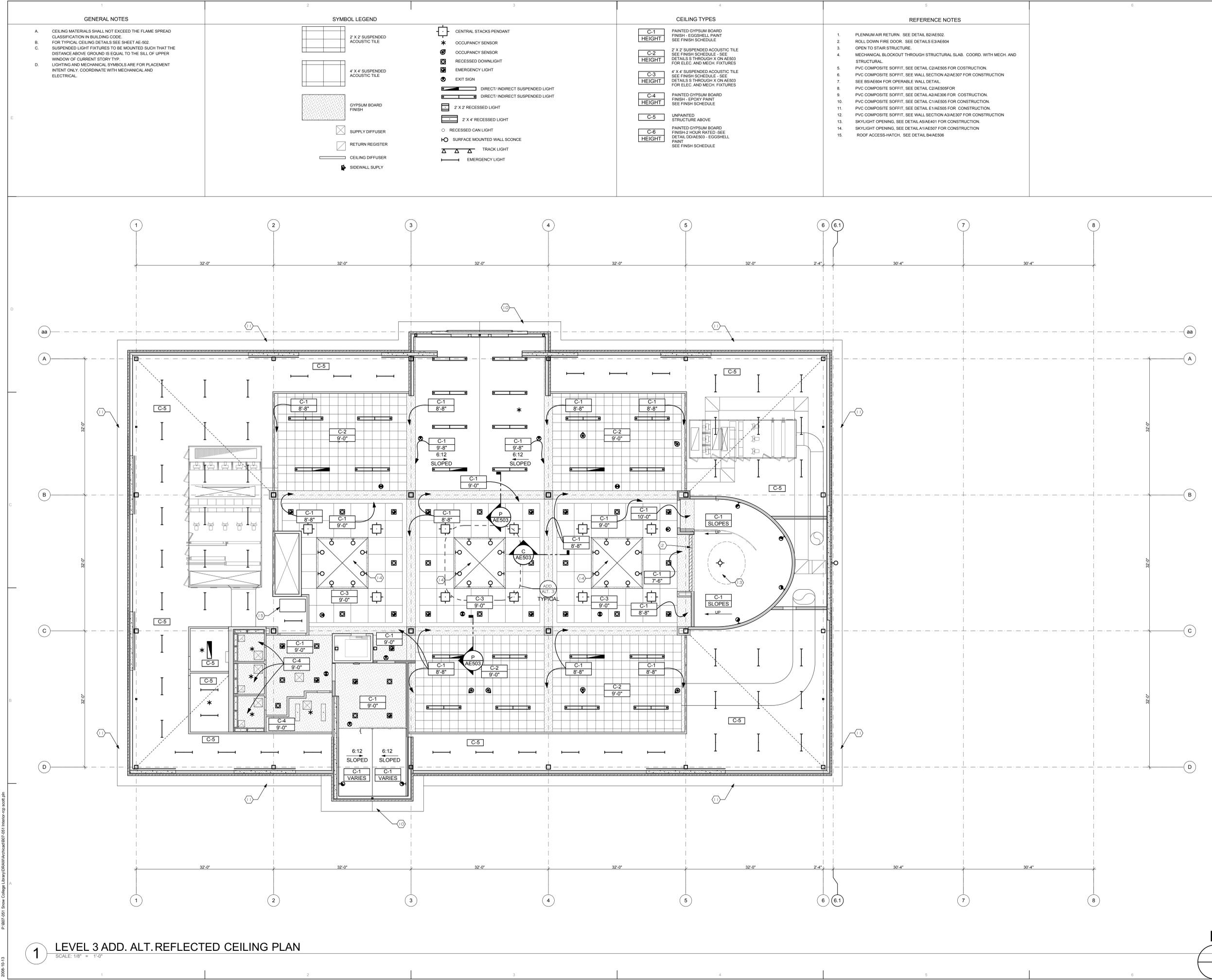
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100% CD, September 15, 2008
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ISSUE DATE:
SEPTEMBER 15, 2008, 100% CD

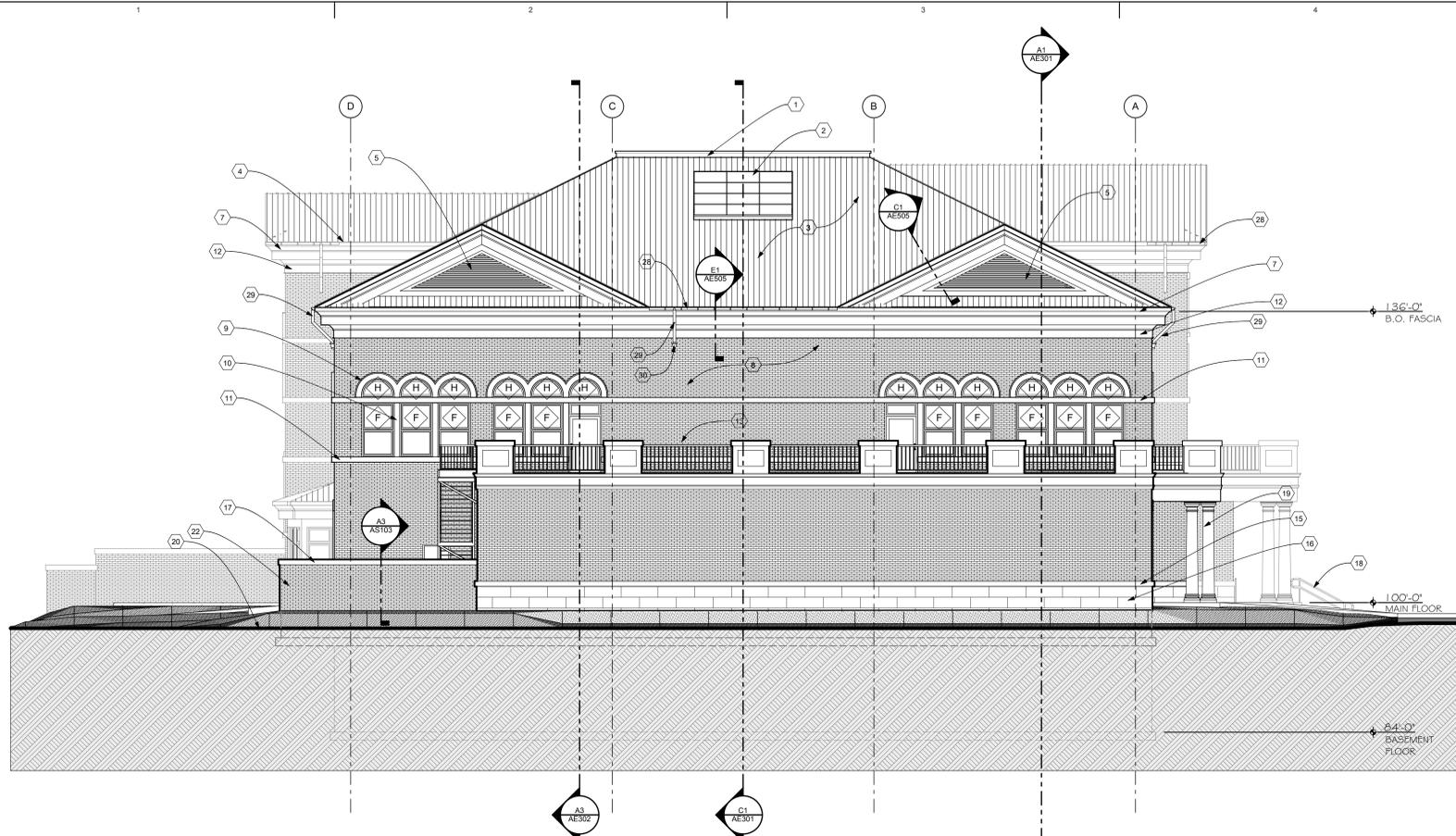
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B07-051
DFCM PROJECT NUMBER:
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SHEET TITLE:

Level 3, Attic RCP Add Alt
SHEET NUMBER:

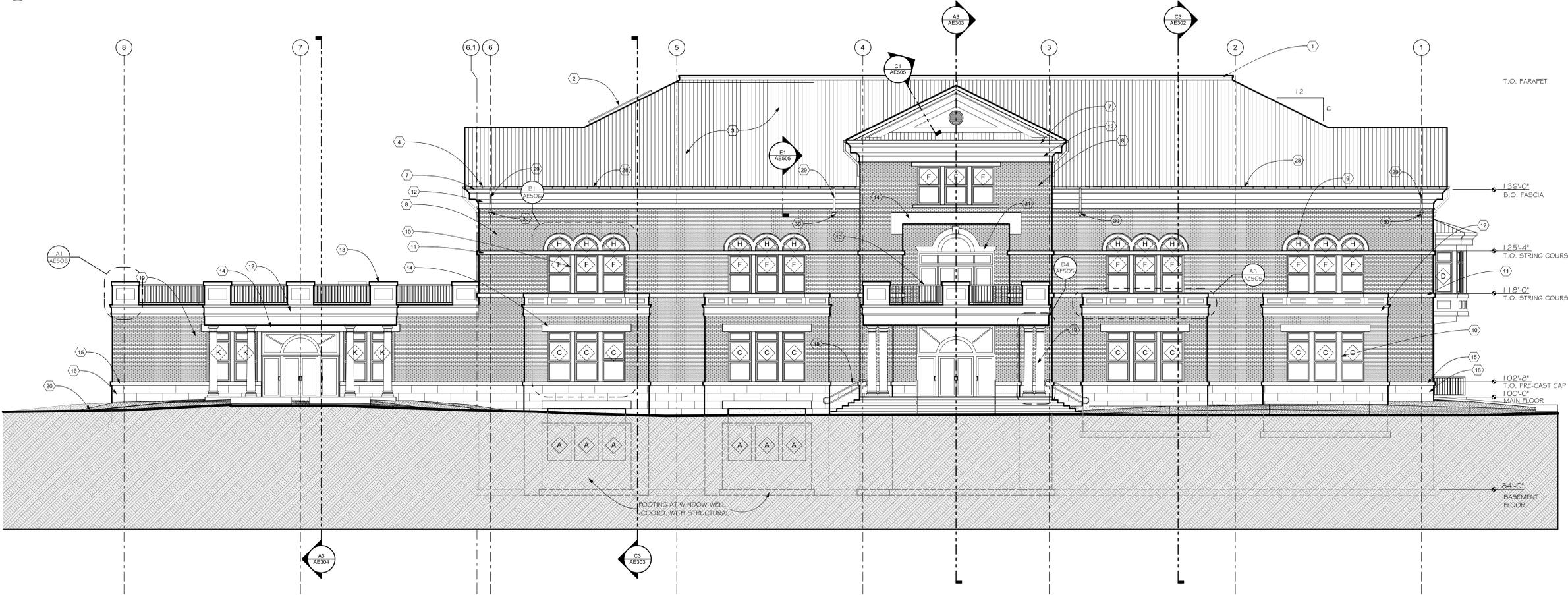
AE116a



1 LEVEL 3 ADD. ALT. REFLECTED CEILING PLAN
SCALE: 1/8" = 1'-0"



C1 EAST ELEVATION
SCALE: 1/8" = 1'-0"



A1 NORTH ELEVATION
SCALE: 1/8" = 1'-0"

REFERENCE NOTES

1. 1" PARAPET WALL WITH CAP
2. SKYLIGHT
3. STANDING SEAM METAL ROOF
4. PRE-FINISHED METAL DRIP EDGE
5. RELIEF AIR VENT - COORD. WITH MECHANICAL SHEET MH104. FIELD VERIFY EXACT SIZE OF MECHANICAL VENT WITH MECHANICAL CONTRACTOR.
6. PRE-FINISHED RAIN GUTTER
7. COMPOSITE FASCIA & SOFFIT
8. 4" BRICK VENEER
9. 8" STONE ARCHES
10. 8" STONE MULLIONS
11. 8" STONE STRING COURSE
12. COMPOSITE FRIEZE BOARD
13. GUARD RAIL
14. STONE LINTEL
15. 8" STONE WATERTABLE
16. STONE PLINTH
17. 8" STONE CAP
18. METAL HANDRAIL - SEE DETAIL A2/A5103
19. EXTERIOR COLUMN - SEE DETAIL XXX
20. FINISH GRADE - COORD WITH CIVIL
21. CONCRETE WALL
22. MECHANICAL ENCLOSURE WALL
23. MECHANICAL UNIT
24. METAL GATE - SEE DETAIL D5/AE505
25. EXTERIOR LIGHT FIXTURE
26. CONCRETE STAIRWELL
27. 4X12 FAUX BEAM - COMPOSITE WRAPPED
28. SMACNA RAIN GUTTER - STYLE F
29. 4" ROUND DOWN SPOUT
30. RECEIVER - SEE DETAIL E1/AE507
31. STONE LINTEL - SEE DETAIL E4/AE506

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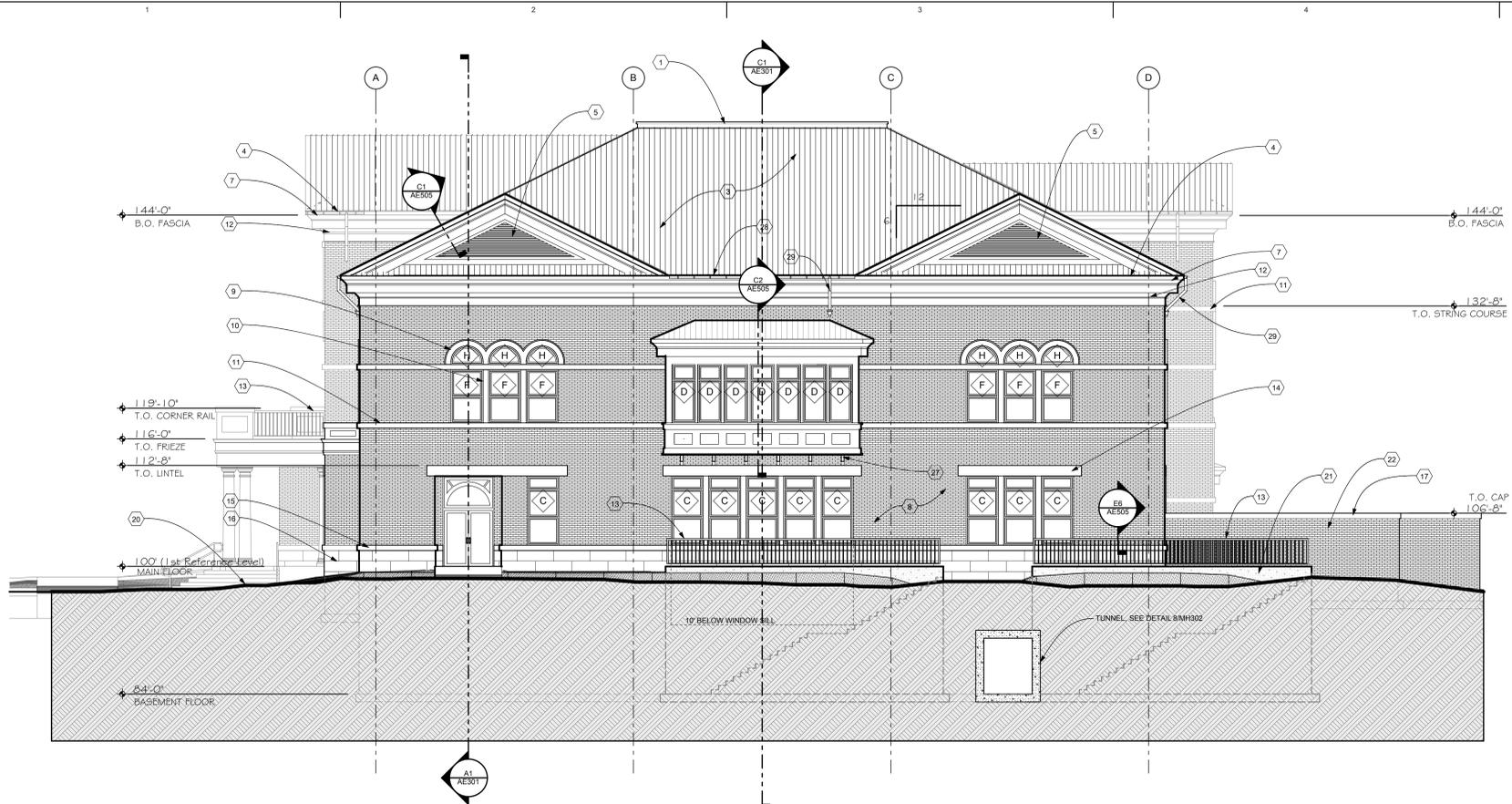
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100% CD Review, AUGUST 4, 2008
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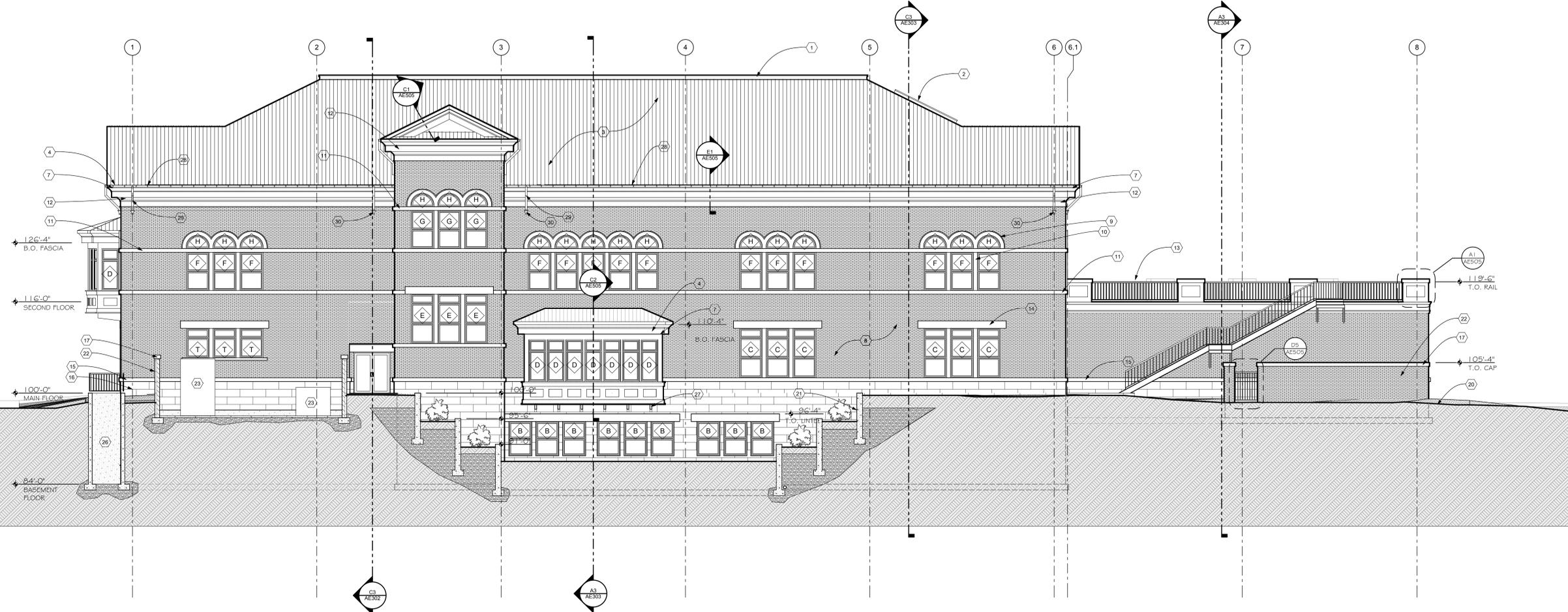
ARCHITECT'S PROJECT NUMBER:
B07-051
DFCM PROJECT NUMBER:
07258700

SHEET TITLE:
ELEVATIONS

SHEET NUMBER:
AE201



C1 WEST ELEVATION
SCALE: 1/8" = 1'-0"



A1 SOUTH ELEVATION
SCALE: 1/8" = 1'-0"

REFERENCE NOTES

1. 1" PARAPET WALL WITH CAP
2. SKYLIGHT
3. STANDING SEAM METAL ROOF
4. PRE-FINISHED METAL DRIP EDGE
5. RELIEF AIR VENT - COORD. WITH MECHANICAL SHEET MH1.04. FIELD VERIFY EXACT SIZE OF MECHANICAL VENT WITH MECHANICAL CONTRACTOR.
6. PRE-FINISHED RAIN GUTTER
7. COMPOSITE FASCIA & SOFFIT
8. 4" BRICK VENEER
9. 8" STONE ARCHES
10. 8" STONE MULLIONS
11. 8" STONE STRING COURSE
12. COMPOSITE FRIEZE BOARD
13. GUARD RAIL
14. STONE LINTEL
15. 8" STONE WATERTABLE
16. STONE PLINTH
17. 8" STONE CAP
18. METAL HANDRAIL - SEE DETAIL A2/A5103
19. EXTERIOR COLUMN - SEE DETAIL XXX
20. FINISH GRADE - COORD WITH CIVIL
21. CONCRETE WALL
22. MECHANICAL ENCLOSURE WALL
23. MECHANICAL UNIT
24. METAL GATE - SEE DETAIL D5/AE505
25. EXTERIOR LIGHT FIXTURE
26. CONCRETE STAIRWELL
27. 4X12 FAUX BEAM - COMPOSITE WRAPPED
28. SMACNA RAIN GUTTER - STYLE F
29. 4" ROUND DOWN SPOUT
30. RECEIVER - SEE DETAIL E1/AE507
31. STONE LINTEL - SEE DETAIL E4/AE506

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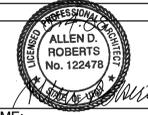
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STAMP:



PROJECT NAME:

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REVISIONS:

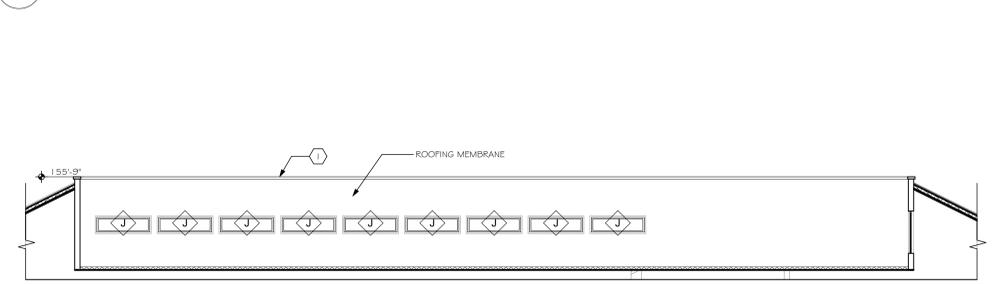
100% CD, September 15, 2008
100% CD Review, AUGUST 4, 2008
ISSUE DATE:
SEPTEMBER 15, 2008, 100% CD
ARCHITECT'S PROJECT NUMBER:
B07-051
DFCM PROJECT NUMBER:
07258700
SHEET TITLE:

ELEVATIONS

SHEET NUMBER:

AE202

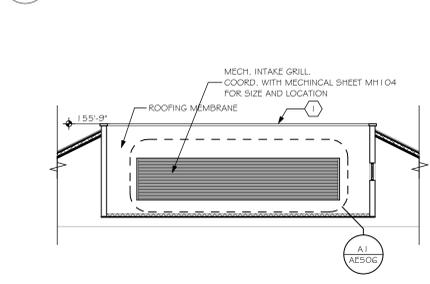
E1 NORTH CLERESTORY ELEVATION
SCALE: 1/8" = 1'-0"



D1 SOUTH CLERESTORY ELEVATION
SCALE: 1/8" = 1'-0"



E3 EAST CLERESTORY ELEVATION
SCALE: 1/8" = 1'-0"



D4 WEST CLERESTORY ELEVATION
SCALE: 1/8" = 1'-0"



REFERENCE NOTES

1. 1" PARAPET WALL WITH CAP
2. SKYLIGHT
3. STANDING SEAM METAL ROOF
4. PRE-FINISHED METAL DRIP EDGE
5. RELIEF AIR VENT - COORD. WITH MECHANICAL SHEET M11.04. FIELD VERIFY EXACT SIZE OF MECHANICAL VENT WITH MECHANICAL CONTRACTOR.
6. PRE-FINISHED RAIN GUTTER
7. COMPOSITE FASCIA & SOFFIT
8. 4" BRICK VENEER
9. 8" STONE ARCHES
10. 8" STONE MULLIONS
11. 8" STONE STRING COURSE
12. COMPOSITE FRIEZE BOARD
13. GUARD RAIL
14. STONE LINTEL
15. 8" STONE WATERTABLE
16. STONE PLINTH
17. 8" STONE CAP
18. METAL HANDRAIL - SEE DETAIL A2/A5.103
19. EXTERIOR COLUMN - SEE DETAIL XXX
20. FINISH GRADE - COORD WITH CIVIL
21. CONCRETE WALL
22. MECHANICAL ENCLOSURE WALL
23. MECHANICAL UNIT
24. METAL GATE - SEE DETAIL D5/AE505
25. EXTERIOR LIGHT FIXTURE
26. CONCRETE STAIRWELL
27. 4X12 FAUX BEAM - COMPOSITE WRAPPED
28. 5MACHNA RAIN GUTTER - STYLE F
29. 4" ROUND DOWN SPOUT
30. RECEIVER - SEE DETAIL E1/AE507
31. STONE LINTEL - SEE DETAIL E4/AE506

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**SNOW
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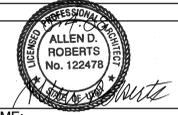
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STAMP:



PROJECT NAME:

Snow College Library
150 College Avenue
Ephraim, Utah 84627

REVISIONS:

NO.	DATE	DESCRIPTION

100% CD, September 15, 2008
100% CD Review, AUGUST 4, 2008

ISSUE DATE:

SEPTEMBER 15, 2008, 100% CD

ARCHITECT'S PROJECT NUMBER:

B07-051

DFCM PROJECT NUMBER:

07258700

SHEET TITLE:

ELEVATIONS

SHEET NUMBER:

AE203



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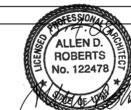
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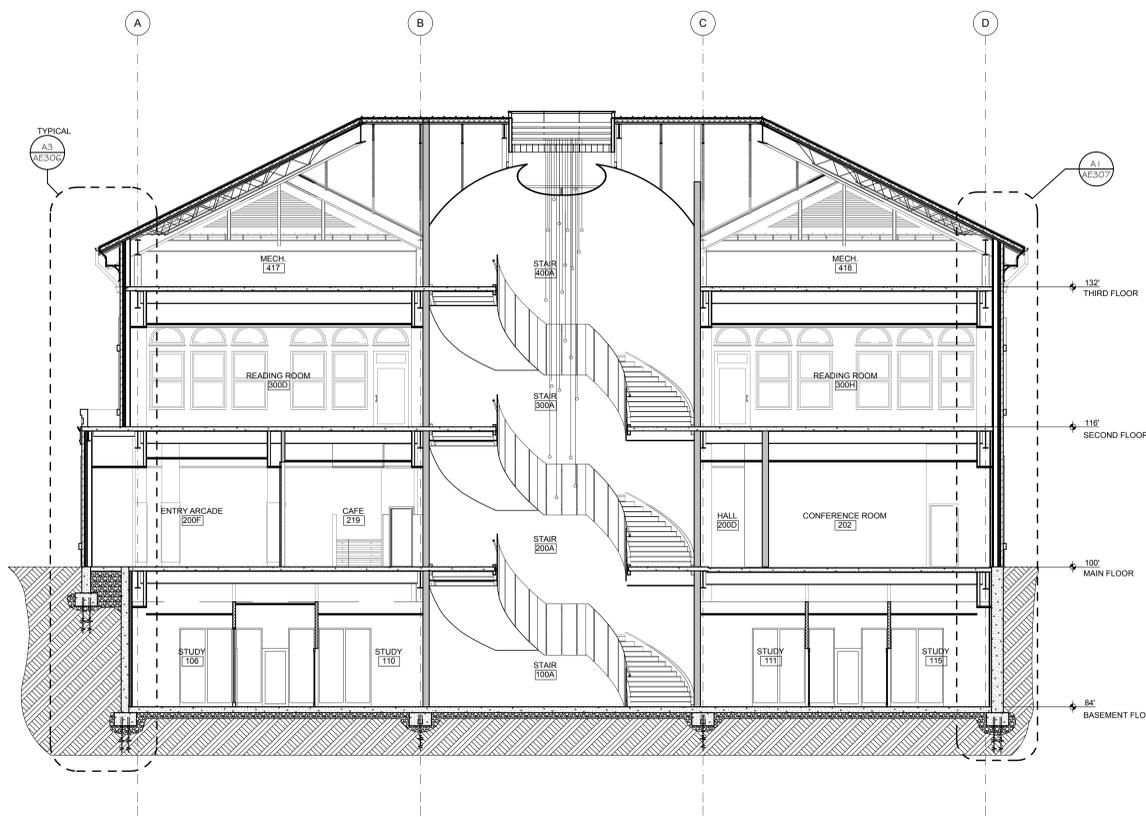
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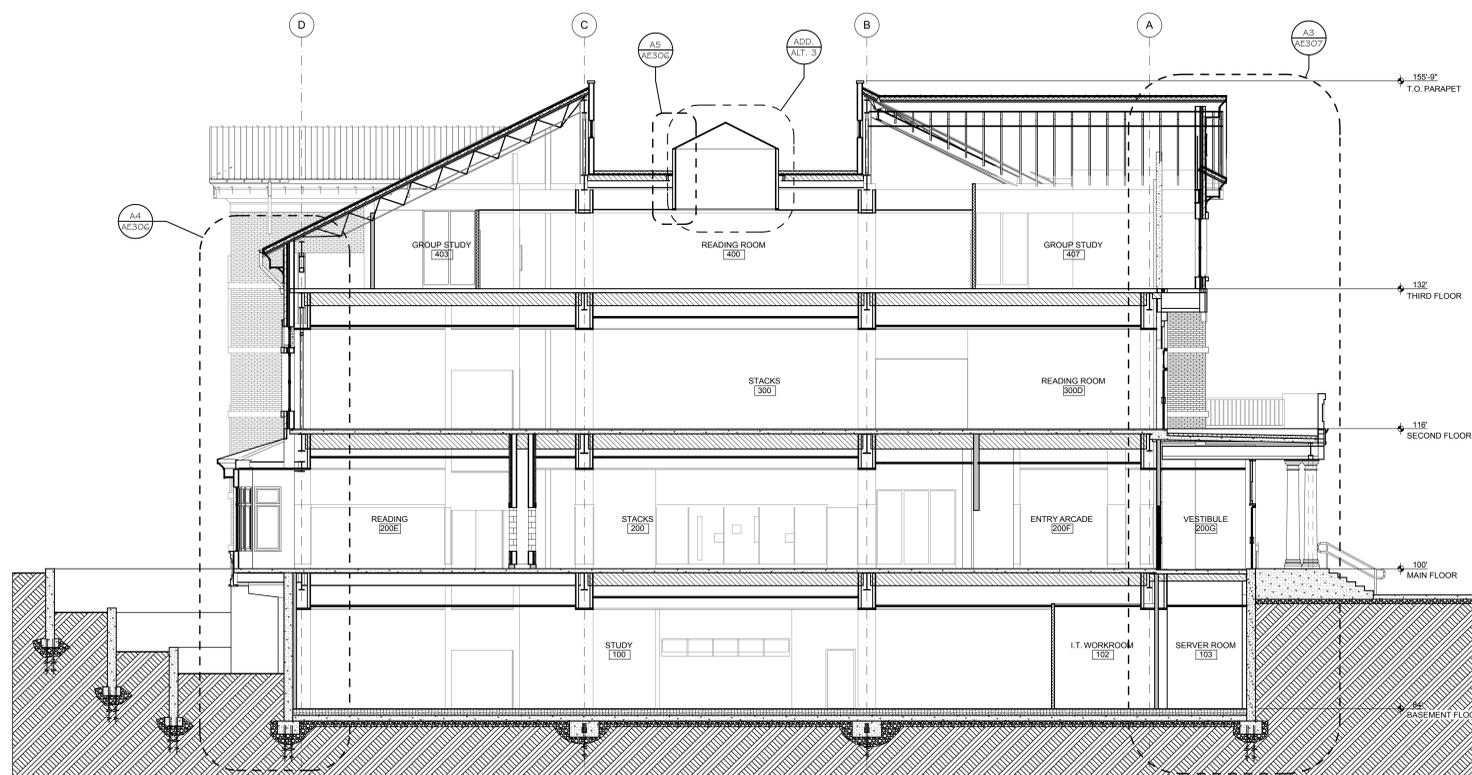
Building Sections

SHEET NUMBER:

AE303



C3 Cross Section 3
1/8" = 1'-0"



A3 Cross Section 2
1/8" = 1'-0"

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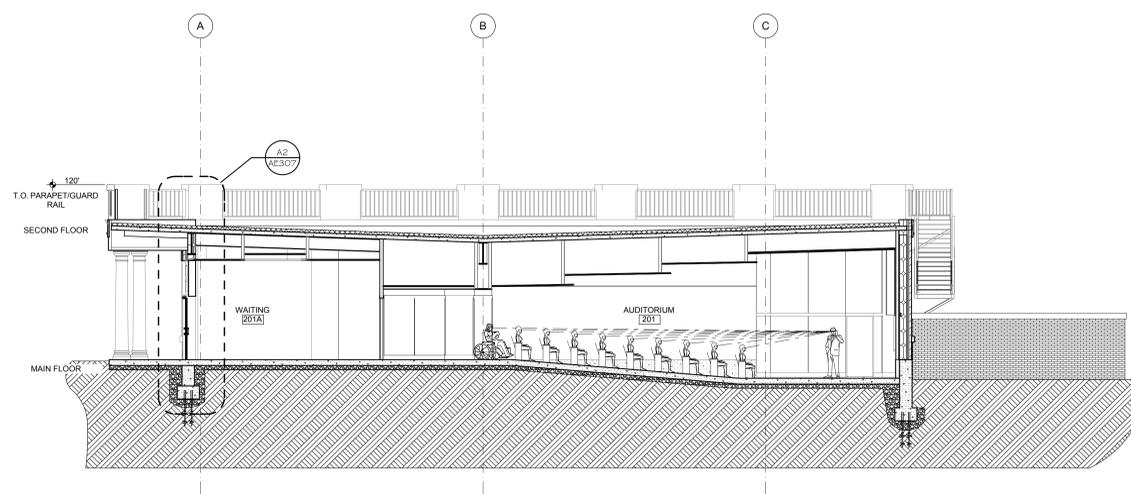
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SHEET TITLE:

Building Sections

SHEET NUMBER:

AE304



A3 Cross Section 4
1/8" = 1'-0"

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REFERENCE NOTES

- 02 SITEWORK**
 2A - FINISH GRADE - PROVIDE SLOPE AWAY FROM BUILDING
 2B - GRAVEL COVER ENGINEER FILL
 2C - UNDISTURBED EARTH
- 02 CONCRETE**
 3A - CONCRETE FOOTING
 3B - CONCRETE FOUNDATION WALL
 3C - CONCRETE FLOOR SLAB
 3D - CONCRETE SIDEWALK
 3E - EXPANSION FIBER JOINT
 3F - CONCRETE FOUNDATION WALL
 3G - CONCRETE SLAB SLOPE AWAY FROM BUILDING
 3H - LEIGHTHEIGHT CONCRETE FLOORING AND DECK
 3J - CONCRETE HAUNCH
 3K - CONCRETE STAIRS
 3L - 1/2" ADDED LEIGHTHEIGHT CONCRETE FLOORING
 3M - CONCRETE SHEAR WALL
 3N - ARCHITECTURAL CONCRETE
 3P - RETAINING WALL
- 04 MASONRY**
 4A - STONE VENEER
 4B - BRICK VENEER
 4C - MANUFACTURED STONE SILL/MATERIAL TABLE
 4D - MANUFACTURED STONE HEADER
 4E - MANUFACTURED STONE ARCH, COORDINATE W EXT. ELEVATIONS
 4F - MANUFACTURED STONE ACCENT
 4G - 4" C/1/2" MASONRY WALL
 4H - BOND BEAM
 4J - MASONRY TIE
 4K - STONE COURSE
 4L - FREE DRAINAGE
- 02 METAL**
 5A - STEEL MEMBER
 5B - STEEL STUD FRAMING
 5C - METAL DECK
 5D - 3" STEEL STUD FRAMING @ 16" O.C.
 5E - METAL FURRING, SEE WALL TYPES
 5F - 2" STEEL STUD FRAMING SPACED 16" O.C.
 5G - 1/2" DIA. STEEL HANDRAIL
 5H - 1/2" DIA. STEEL GUARDRAIL
 5J - STRUCTURAL STEEL JOIST
 5K - STRUCTURAL STEEL COLUMN
 5L - STRUCTURAL STEEL BEAM
 5M - HELICAL PIERS
 5N - STEEL BEAM BRACING AS PER STRUCTURAL
 5P - 6" STEEL STUD FRAMING SPACED 16" O.C.
 5Q - SLIP JOINT
 5R - 1/2" HAT CHANNEL
 5S - OPEN WEB STEEL JOIST
 5T - METAL ROOF
 5U - STEEL SHELF ANGLE
- 06 WOOD & PLASTICS**
 6A - WOOD BLOCKING
 6B - EXTERIOR SHEATHING
 6C - 3/4" PVC COMPOSITE
 6D - PVC SHEATHING
 6E - 1" PVC COMPOSITE
 6F - 1/2" PVC COMPOSITE
 6G - ROOF FRAMING
 6H - PVC CROWN HOLDING
 6J - 1/2" PVC COMPOSITE
 6K - EXTERIOR GYPSUM BOARD
 6L - PLTWOOD VENEER PANEL SYSTEM
- 07 THERMAL & MOISTURE PROTECTION**
 7A - ALUMINUM FACED BATT INSULATION
 7B - BUILDING WRAP
 7C - SPECIFIED MOISTURE BARRIER
 7D - ASPHALT ROOF SINKLES
 7E - MECHANICAL INTAKE GRILL
 7F - RIGID INSULATION BOARD
 7G - CEILING INSULATION
 7H - PREFINISHED METAL FLASHING
 7I - PREFINISHED PARAPET CAP
 7J - PREFINISHED RAIN GUTTER
 7K - 1" AIR VOID
 7L - BUILT UP CRICKET
 7M - ADHERED MEMBRANE ROOFING SYSTEM
 7N - JOINT SEALANTS
 7O - ROOF DRAIN
 7P - PROTECTION BOARD
 7Q - VAPOR BARRIER
- 08 DOORS & WINDOWS**
 8A - ALUM. GLAZ. WOOD WINDOW SYSTEM
 8B - SKYLIGHT
 8C - SCHEDULED STOREFRONT ENTRY SYSTEM
- 09 FINISHES**
 9A - SUSPENDED CEILING SYSTEM, TYPE VARIES - SEE REFLECTED CEILING PLANS
 9B - 3/4" GYP BOARD - FINISHED AS PER SCHEDULE
 9C - EXTERIOR GYP BOARD
 9D - HOOD PANEL SYSTEM
- 10 SPECIALTIES**
 10A - PAVER PEDESTAL FLOORING SYSTEM
 10B - LIVE ROOF SYSTEM
 10C - RAISED FLOOR SYSTEM
 10D - FAUX VENT
 10E - ARCHITECTURAL EXTERIOR COLUMN
 10F - SKY LIGHT PANELS

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DFCM PROJECT NUMBER:
 07258700
SHEET TITLE:

WALL SECTIONS

SHEET NUMBER:
AE305

