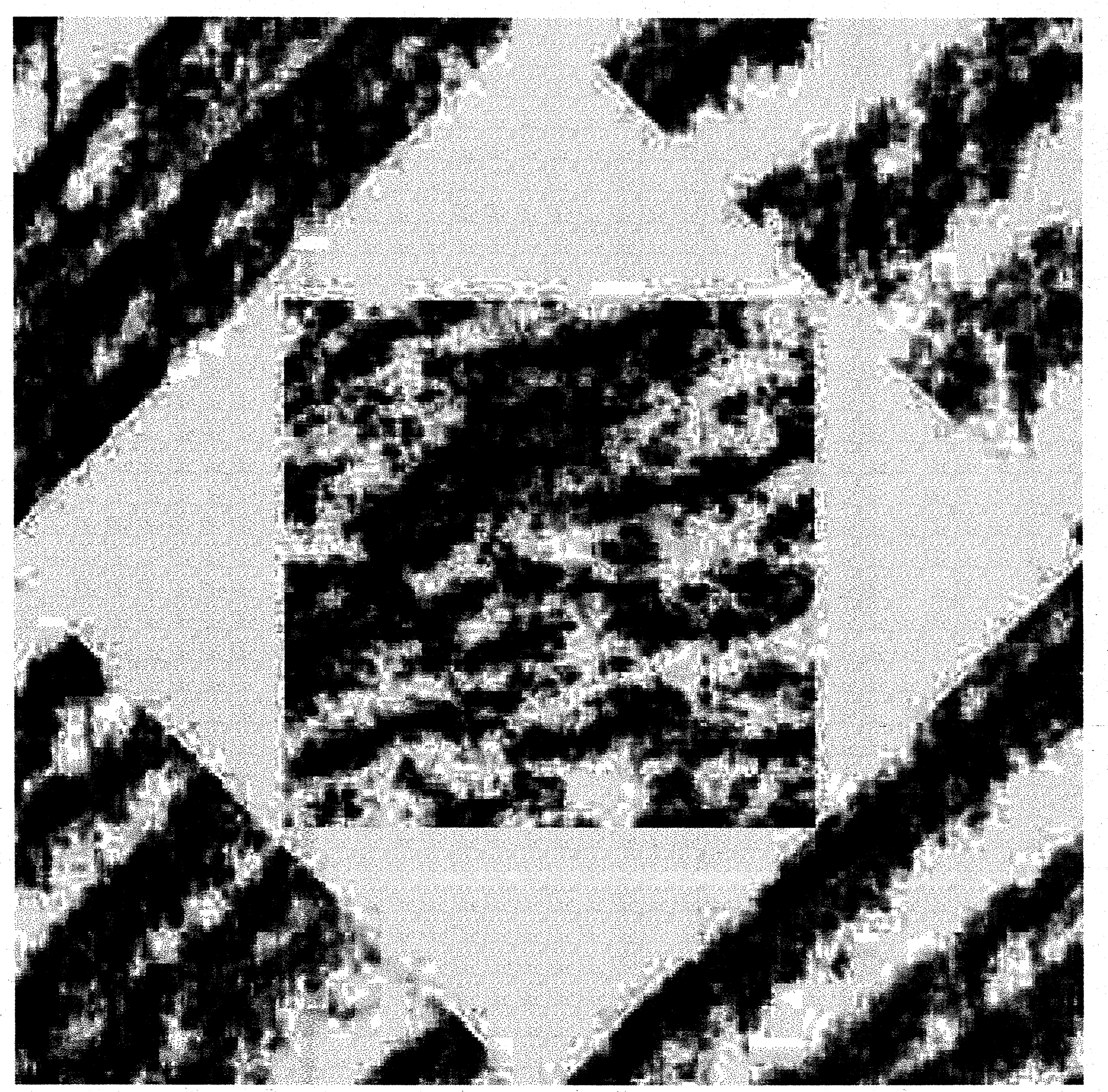

DWS - PROVO ENTRANCE REPLACEMENT & LANDSCAPING IMPROVEMENTS

DEPARTMENT OF WORKFORCE SERVICES
PROVO, UTAH

State of Utah–Department of Administrative Services

DIVISION OF FACILITIES CONSTRUCTION AND MANAGEMENT

4110 State Office Building/Salt Lake City, Utah 84114/538-3018

GENERAL ABBREVIATIONS

ACT.	ACCOUSTIC CEILING TILE
ALT.	ALTERNATE
ALUM.	ALUMINUM
A.B.	ANCHOR BOLT
&	AND
ARCH.	ARCHITECTURAL
@	AT OR AT THE
BM.	BEAM
BLK.	BLOCK
BLKG.	BLOCKING
BD.	BOARD
BOT.	BOTTOM
BLDG.	BUILDING
CPT.	CARPET
CLKG.	CAULKING
C.I.	CAST IRON
CLG.	CEILING
CEM.	CEMENT
CTR.	CENTER
CL	CENTER LINE
CER.	CERAMIC
C.T.	CERAMIC TILE
CLR.	CLEAR (ANCE)
CLO.	CLOSET
COL.	COLUMN
CONC.	CONCRETE
CMU	CONCRETE MASONRY UNIT
CMP	CORRUGATED METAL PIPE
CONN.	CONNECTION
CONSTR.	CONSTRUCTION
CONT.	CONTINUE/CONTINUOUS
CONTR.	CONTRACTOR
C.J.	CONTROL JOINT
CORR.	CORRIDOR
CNTR.	COUNTER
CTSK.	COUNTERSUNK
DET.	DETAIL
DEPT.	DEPARTMENT
DIA.	DIAMETER
DIM.	DIMENSION
DN.	DOWN
D.S.	DOWNSPOUT
DWG.	DRAWING
D.F.	DRINKING FOUNTAIN
E.	EAST
EA.	EACH
ELEC.	ELECTRIC (AL)
ELEV./EL.	

EXIST.	EXISTING
EQ.	EQUAL
EQUIP.	EQUIPMENT
EXIST.	EXISTING
EXP.	EXPANSION
EXT.	EXTERIOR
FIN.	FINISH
F.A.	FIRE ALARM
F.E.	FIRE EXTINGUISER
F.E.C.	F.E. CABINET
FLR./FL.	FLOOR
F.D.	FLOOR DRAIN
F.O.S.	FACE OF STUD
F.O.W.	FACE OF WALL
FTG.	FOOTING
FDN.	FOUNDATION
F.F.	FINISH FLOOR
GALV	GALVANIZED
G.I.	GALVANIZED IRON
GA.	GAUGE
GL.	GLASS
GR.	GRADE
GND.	GROUND
GYP. BD.	GYPSUM BOARD
GWB	GYPSUM WATERPROOF BOARD
H.D.P.E.	HIGH DENSITY POLYETHYLENE
HG#	HARDWARE GROUP #
HDWD.	HARDWOOD
HT.	HEIGHT
H.P.	HIGH POINT
HORIZ.	HORIZONTAL
H.B.	HOSE BIBB
H.M.	HOLLOW METAL
HR.	HOURS (FIRE RATING)
IN.	INCH
I.D.	INSIDE DIAMETER
INSUL.	INSULATION
INT.	INTERIOR
I.E.	INVERT ELEVATION
INV.	
JAN.	JANITOR
JT.	JOINT
J-BOX	JUNCTION BOX
KIT.	KITCHEN
LAM.	LAMINATE
LAV.	LAVATORY
LT.	LIGHT
LP.	LOW POINT

MATL.	MATERIAL
MAS.	MASONRY
MAINT.	MAINTENANCE
MFR.	MANUFACTURER
M.H.	MANHOLE
M.O.	MASONRY OPENING
MAX.	MAXIMUM
MECH.	MECHANICAL
MEMB.	MEMBRANE
MEN	MEN'S TOILET
MTL./MET.	METAL
MIN.	MINIMUM
MIR.	MIRROR
MISC.	MISCELLANEOUS
MTD.	MOUNTED
MUL.	MULLION
NOM.	NOMINAL
N.	NORTH
N.I.C.	NOT IN CONTRACT
N.T.S.	NOT TO SCALE
NO.	NUMBER
OR #	ORDINATE NUMBER
OFF.	OFFICE
O.C.	ON CENTER
OPNG.	OPENING
OPP.	OPPOSITE
OPP. H.	OPPOSITE HAND
O.D.	OUTSIDE DIAMETER
O.R.D.	OVERFLOW ROOF DRAIN
PTD. -E	PAINTED EPOXY
PTD.	PAINTED
PR.	PAIR
PART.	PARTITION
PED.	PEDESTRIAN
PLAS.	PLASTER
P. LAM.	PLASTIC LAMINATE
PL	PLATE
PLYWD.	PLYWOOD
PT.	POINT
Q.T.	QUARRY TILE
RAD.	RADIUS
R.B.	RUBBER BASE
R.W.L.	RAIN WATER LEADER
R.F.F.	REFERENCE FINISH FLOOR
REFL.	REFLECTED
REINF.	REINFORCING
REQ.	REQUIRED

RET.	RETAINING
REV.	REVISED
R.	RISER
R.D.	ROOF DRAIN
RM.	ROOM
R.O.	ROUGH OPENING
SCHED.	SCHEDULE
SEAL.	SEALANT
SECT.	SECTION
S.SK.	SERVICE SINK
SHT.	SHEET
SIM.	SIMILAR
SL/SLP.	SLOPE
S.C.	SOLID CORE
SPEC.	SPECIFICATIONS
SQ.	SQUARE
STD.	STANDARD
STL.	STEEL
STOR.	STORAGE
STRUCT	STRUCTURAL/STRUCTURE
SYM.	SYMMETRICAL
T.B.R.	TO BE REMOVED
TEL.	TELEPHONE
TEMP.	TEMPORARY / TEMPERED
THK.	THICK (NESS)
T & G	TONGUE AND GROOVE
T/CONC	TOP OF CONCRETE
T/CURB	TOP OF CURB
T.O. FTG.	TOP OF FOOTING
T.O.P.	TOP OF PLATE
T/WALL	TOP OF WALL
T.	TREAD
TYP.	TYPICAL
UNF.	UNFINISHED
U.N.O.	UNLESS NOTED OTHERWISE
VAR.	VARY OR VARIES
VERT.	VERTICAL
V.T.R.	VENT THROUGH ROOF
VCT	VINYL COMPOSITION TILE
W/	WITH
W.A.S.	WELDED ANCHOR STUD
WD.	WOOD
WP.	WATERPROOF
WSCT.	WAINSCOT
W/O	WITHOUT
W.P.	WORKING POINT
W.R.	WATER RESISTANT
W.I.	WROUGHT IRON

MATERIALS LEGEND

EARTH	GRAVEL
SAND	CONCRETE
CONCRETE W/ ARCH. FINISH	CAST STONE
CMU	MARBLE
BRICK	GRANITE
LIMESTONE	STONE
WOOD (BLOCKING)	PLYWOOD
WOOD FRAMING	WOOD FINISH
STEEL	ALUMINUM
RIGID INSULATION	BATT INSULATION
PLASTER	ACOUSTICAL TILE
COMPRESSIBLE FILLER	BACKER ROD AND FILLER
METAL LATH	GYPSUM BOARD
FENCE	TO REMOVE

CODE ANALYSIS

APPLICABLE CODES

	Year		Year
International Building Code	2006	National Electrical Code	2006
International Mechanical Code	2006	Uniform Code for Building Conservation	N.A.
International Plumbing Code	2006	ADA Accessibility Guidelines	1994
International Fire Code	2006		
International Energy Conservation Code	N.A.		

A. Occupancy and Group: B
Change in Use: Yes ___ No X Mixed Occupancy: Yes ___ No X
Special Use and Occupancy (e.g. High Rise, Covered Mall): N.A.

B. Seismic Design Category: N.A. Design Wind Speed: N.A. mph

C. Type of Construction (circle one):
I/A I/B II/A (II/B circled) III/A III/B IV/HT V/A V/B

D. Fire Resistance Rating Requirements for the Exterior Walls based on the fire separation distance (in hours):
North: N.A. South: N.A. East: N.A. West: N.A.

E. Mixed Occupancies: N.A. Nonseparated Uses: N.A.

F: Sprinklers:
Required: N.A. Provided: ___ Type of Sprinkler System: ___

G: Number of Stories: N.A. Building Height: N.A.

H: Actual Area per Floor (square feet): N.A.

I: Tabular Area: ___

J: Area Modifications:

a) $A_a = A_t + \left[\frac{A_t\, I_f}{100}\right] + \left[\frac{A_t\, I_s}{100}\right]$ $\quad I_f = 100\left[\frac{F}{P} - 0.25\right]\frac{W}{30}$

b) Sum of the Ratio Calculations for Mixed Occupancies:
$\frac{\text{Actual Area}}{\text{Allowable Area}} \leq 1$

c) Total Allowable Area for:
1) One Story: ___
2) Two Story: $A_a(2)$ ___
3) Three Story: $A_a(3)$ ___

d) Unlimited Area Building: Yes ___ No X Code Section: ___

K. Fire Resistance Rating Requirements for Building Elements (hours).

Element	Hours	Assembly Listing	Element	Hours	Assembly Listing
Exterior Bearing Walls	N.A.		Floors - Ceiling Floors	N.A.	
Interior Bearing Walls	N.A.		Roofs - Ceiling Roofs	N.A.	
Exterior Non-Bearing Walls	N.A.		Exterior Doors and Windows	N.A.	
Structural Frame	N.A.		Shaft Enclosures	N.A.	
Partitions - Permanent	N.A.		Fire Walls	N.A.	
Fire Barriers	N.A.		Fire Partitions	N.A.	
			Smoke Partitions		

L. Design Occupant Load: 0
Exit Width Required: 0 Exit Width Provided: 0

M. Minimum Number of Required Plumbing Facilities:
a) Water Closets - Required (m) 0 (f) 0 Provided (m) 0 (f) 0
b) Lavatories - Required (m) 0 (f) 0 Provided (m) 0 (f) 0
c) Bath Tubs or Showers: 0
d) Drinking Fountains: 0 Service Sinks: 0

FOOTNOTES:
1) In case of conflict with the U.S. Department of Justice Federal Registers Parts I - V - ADA Guidelines and specific reference to the International Building Code Accessibility Chapters, the more restrictive requirement shall govern.
2) Additional Code Information shall be provided at the discretion of the Building Official for Complex Buildings. Including, but not limited to:
a) High Rise Requirements. e) Fire Assembly Locator Sheet.
b) Atriums. f) Exterior and Interior Accessibility Route.
c) Performance Based Criteria. g) Fire Stopping, Including Tested Design Number.
d) Means or Egress Analysis.

GRAPHIC SYMBOLS

DTL# / SHT#	BLDG. ELEV. SYMB.
DTL# / SHT#	BLDG. SECT. SYMB.
DTL# / SHT#	WALL SECT. SYMB.
#	KEYED NOTE SYMB.
T.O. ELEV	DATUM PT. SYMB.
ELEV	SPOT ELEV. SYMB.
RM NAME / RM# / F/B W/W	RM. NAME/NUMB RM. FINISH SYMB.
FIN ELEV	CEIL. FINISH/ELEV. SYMB.
DTL# / SHT#	DETAIL REF. SYMB.
DTL# / SHT#	INTR. ELEV. SYMB.
DR# / HW#	DOOR/HDWR. SYMB.
EF#	WINDOW SYMB.

DRAWING INDEX

ARCHITECTURAL DRAWINGS

GI001	TITLE SHEET, GENERAL INFO, & SHEET INDEX
AS101	EXISTING SITE UTILITY PLAN
AS102	SITE DEMOLITION PLAN
AS103	LANDSCAPE DEMOLITION PLAN
AS104	SITE PLAN
AS105	LANDSCAPE PLAN
AS501	SITE SECTIONS & DETAILS

STRUCTURAL DRAWINGS

SE101	GENERAL NOTES (GSN)
SE201	FOOTING AND FOUNDATION PLAN
SE501	SECTIONS AND DETAILS
SE502	SECTIONS AND DETAILS

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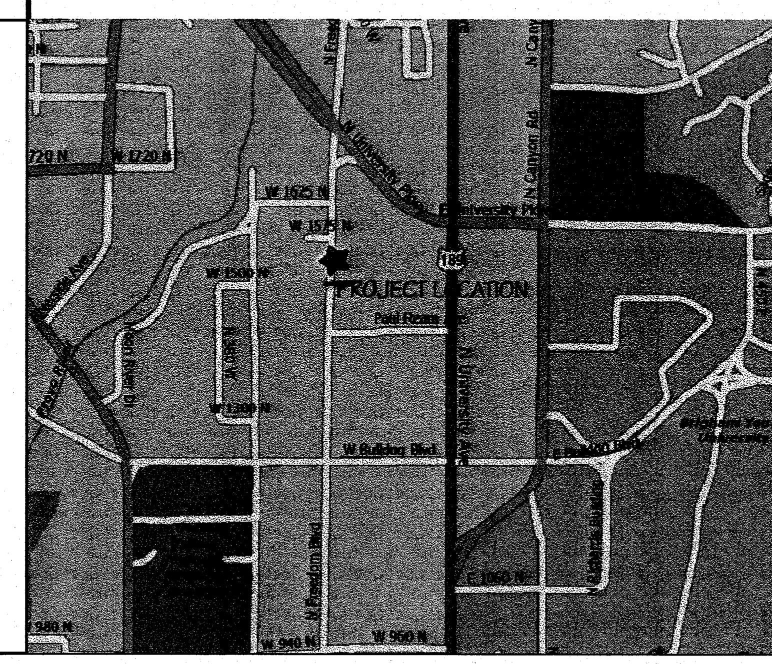
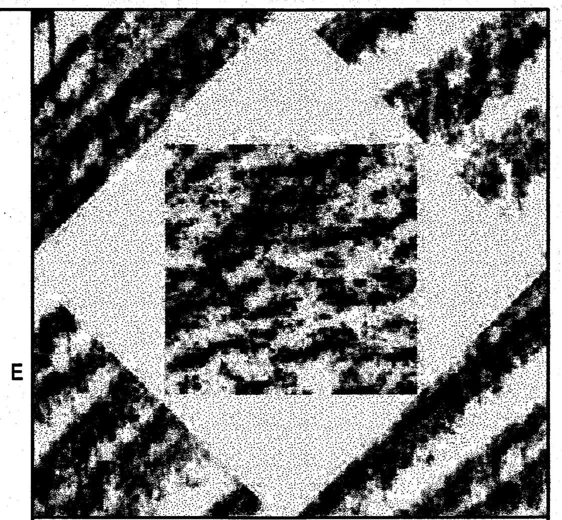
VICINITY MAP





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CONSULTANT

STATE OF UTAH – BARRY W. SMITH – 123799 – 19 MAY 2009 – LICENSED ARCHITECT

DWS - PROVO ENTRANCE REPLACEMENT & LANDSCAPING IMPROVEMENTS

Department of Workforce Services
1550 North Freedom Boulevard
Provo, Utah

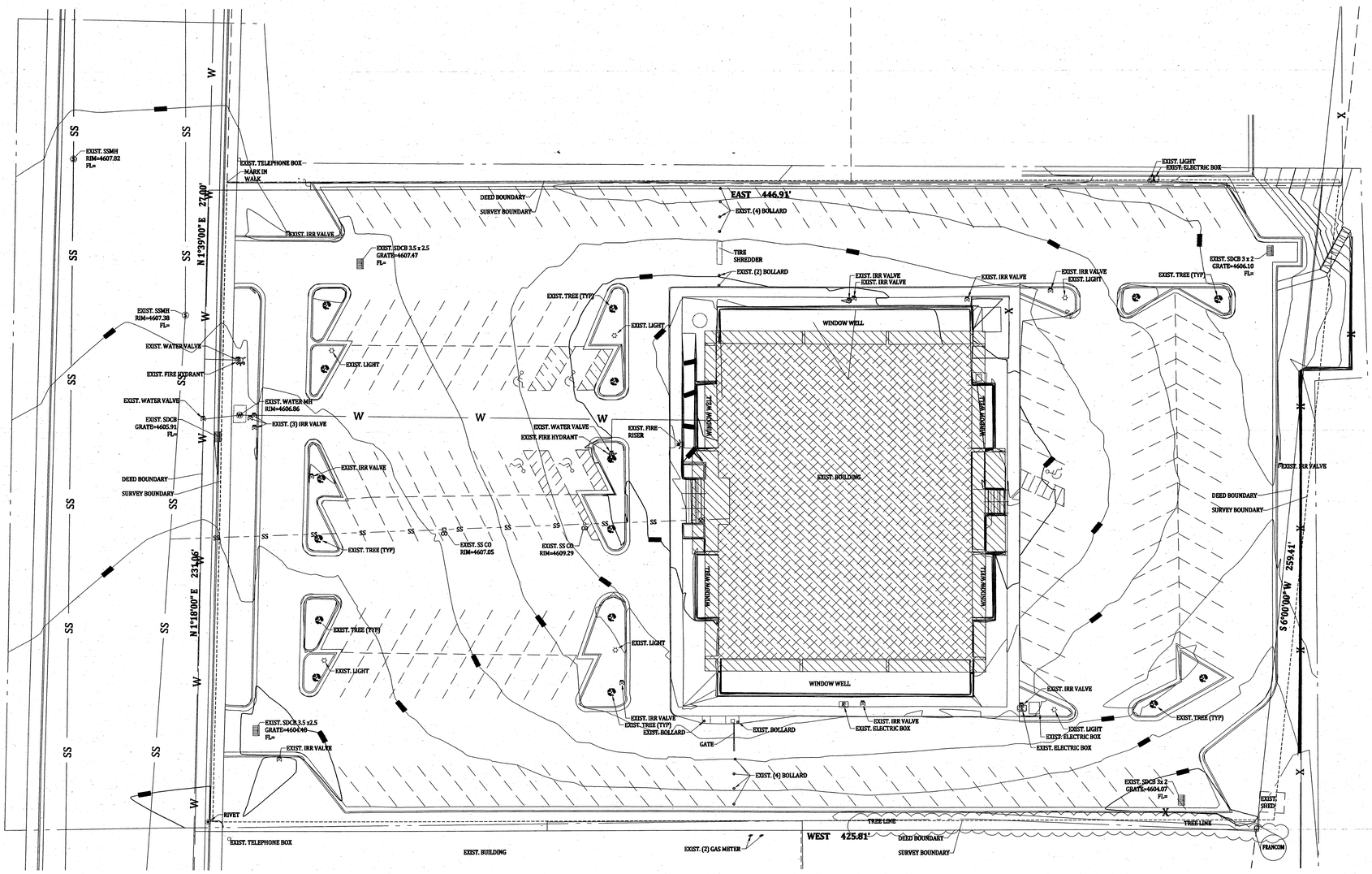
MARK	DATE	DESCRIPTION
	5/20/09	Add #3 ISSUE

DATE:	29 APRIL 2009
DFCM PROJECT NO:	08071920
HFSA PROJECT NO:	0821.01
CAD DWG FILE NO:	
DRAWN BY:	RLS
CHECKED BY:	BWS
DESIGNED BY:	RLS
DWG TYPE:	ARCHITECTURAL
ARCHITECTURAL PHASE:	CONSTRUCTION BID DOCUMENTS

SHEET TITLE

TITLE SHEET, GENERAL INFO, & SHEET INDEX

GI001



A3 EXISTING SITE UTILITY PLAN
1"=20'-0"

GENERAL NOTES

1. ALL DIMENSIONS & EXISTING CONDITIONS IN AREAS OF WORK ARE TO BE FIELD VERIFIED PRIOR TO COMMENCING WORK - ANY DISCREPANCIES ARE TO BE REPORTED TO THE ARCHITECT OR ENGINEER OF RECORD PRIOR TO COMMENCING WORK.

2. PROTECT ALL AREAS & SURFACES ADJACENT TO DEMOLITION & CONSTRUCTION. PATCH & REPAIR ANY DAMAGE RESULTING FROM THE DEMOLITION OF EXISTING ITEMS OR THE CONSTRUCTION OF NEW ITEMS.

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7. PROVIDE TEMPORARY 6'-0" HIGH CHAIN LINK FENCE @ PROJECT LIMIT LINE, COMPLETE W/GATES AS REQUIRED, UNLESS NOTED OTHERWISE.

8. EXISTING ROADS, PARKING, SERVICE DRIVES, & SIDEWALKS SHALL REMAIN ACCESSIBLE & KEPT CLEAR OF CONSTRUCTION EQUIPMENT, MATERIALS, DIRT AND DEBRIS.

SITE PLAN LEGEND

- IRR — EXISTING IRRIGATION VALVE
- WV — EXISTING WATER VALVE
- EXISTING FIRE RISER
- EXISTING FIRE HYDRANT
- W — EXISTING WATER MAN HOLE
- S — EXISTING SANITARY SEWER MAN HOLE
- CO — EXISTING SANITARY SEWER CLEAN OUT
- EXISTING STORM DRAIN CATCH BASIN & GRATE
- EXISTING LIGHT POLE
- E — EXISTING ELECTRICAL BOX
- T — EXISTING TELEPHONE BOX
- EXISTING PARKING SECURITY GATE
- EXISTING BOLLARD
- EXISTING PARKING SECURITY EXIT - TIRE SHREDDER
- EXISTING TREE
- EXISTING BURIED SANITARY SEWER LINE
- EXISTING BURIED WATER LINE
- EXISTING FENCE
- PROPERTY LINE DEED BOUNDARY
- PROPERTY LINE SURVEY BOUNDARY

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123789
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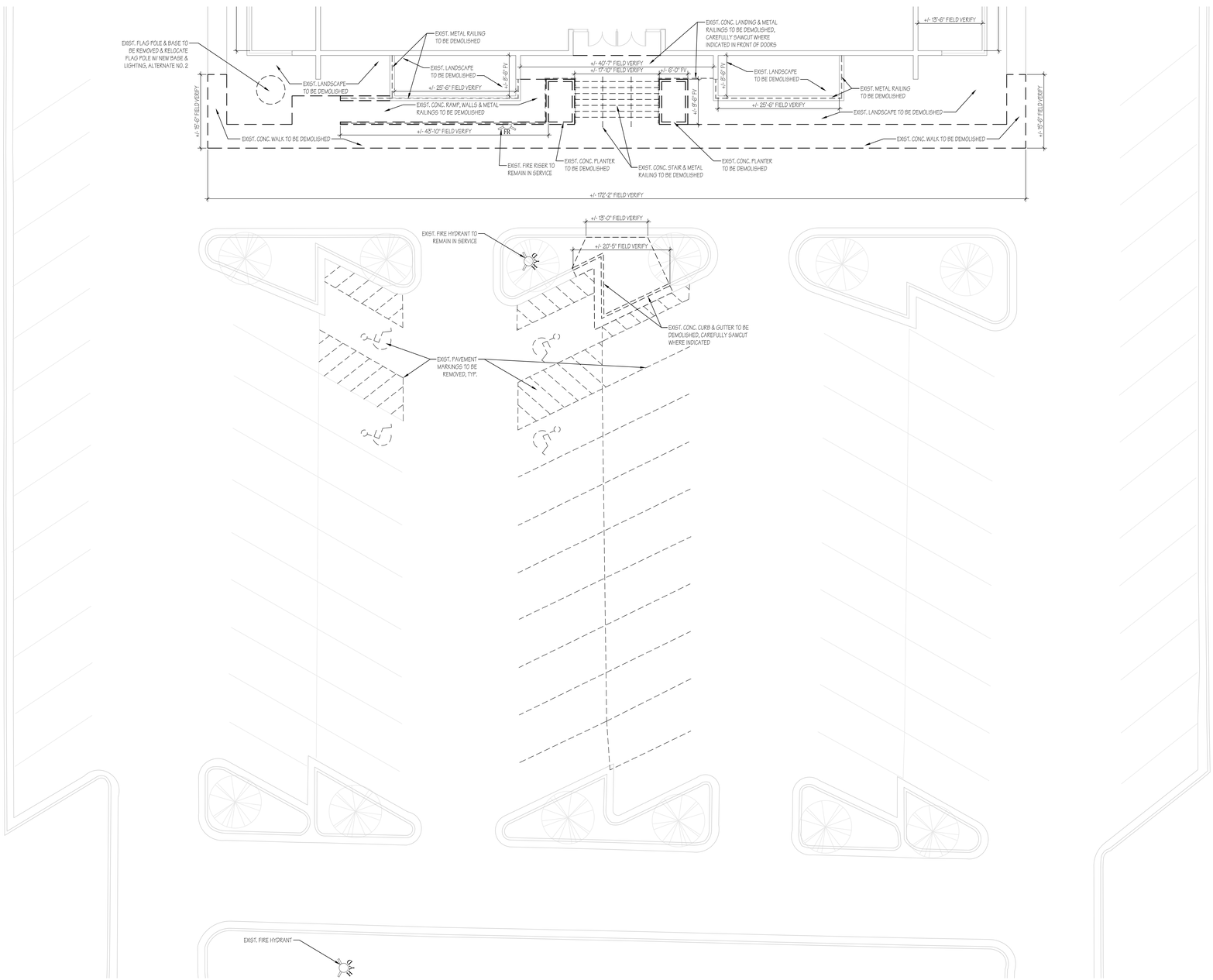
SHEET TITLE

EXISTING SITE UTILITY PLAN

AS101

SHEET 2 OF 11

E3 PHOTO OF EXISTING CONDITIONS
N.T.S.



A3 SITE DEMOLITION PLAN
1"=10'-0"

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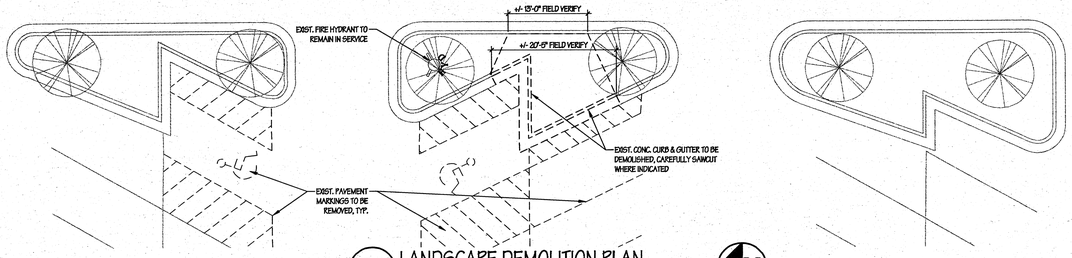
SHEET TITLE

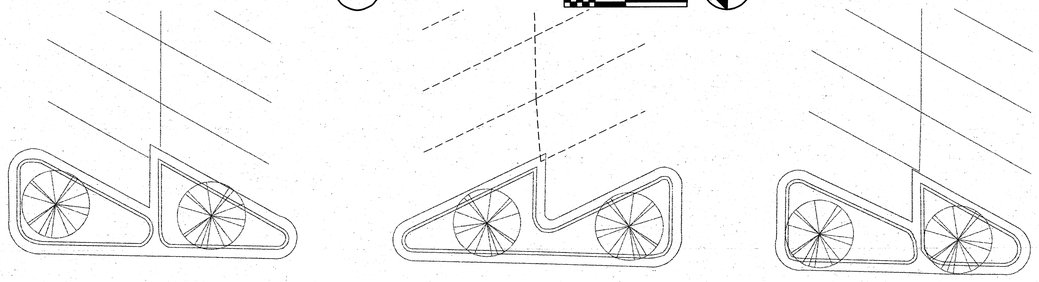
SITE DEMO PLAN

AS102

SHEET 3 OF 11



E4 LANDSCAPE DEMOLITION PLAN
1"=10'-0"



D4 LANDSCAPE DEMOLITION PLAN
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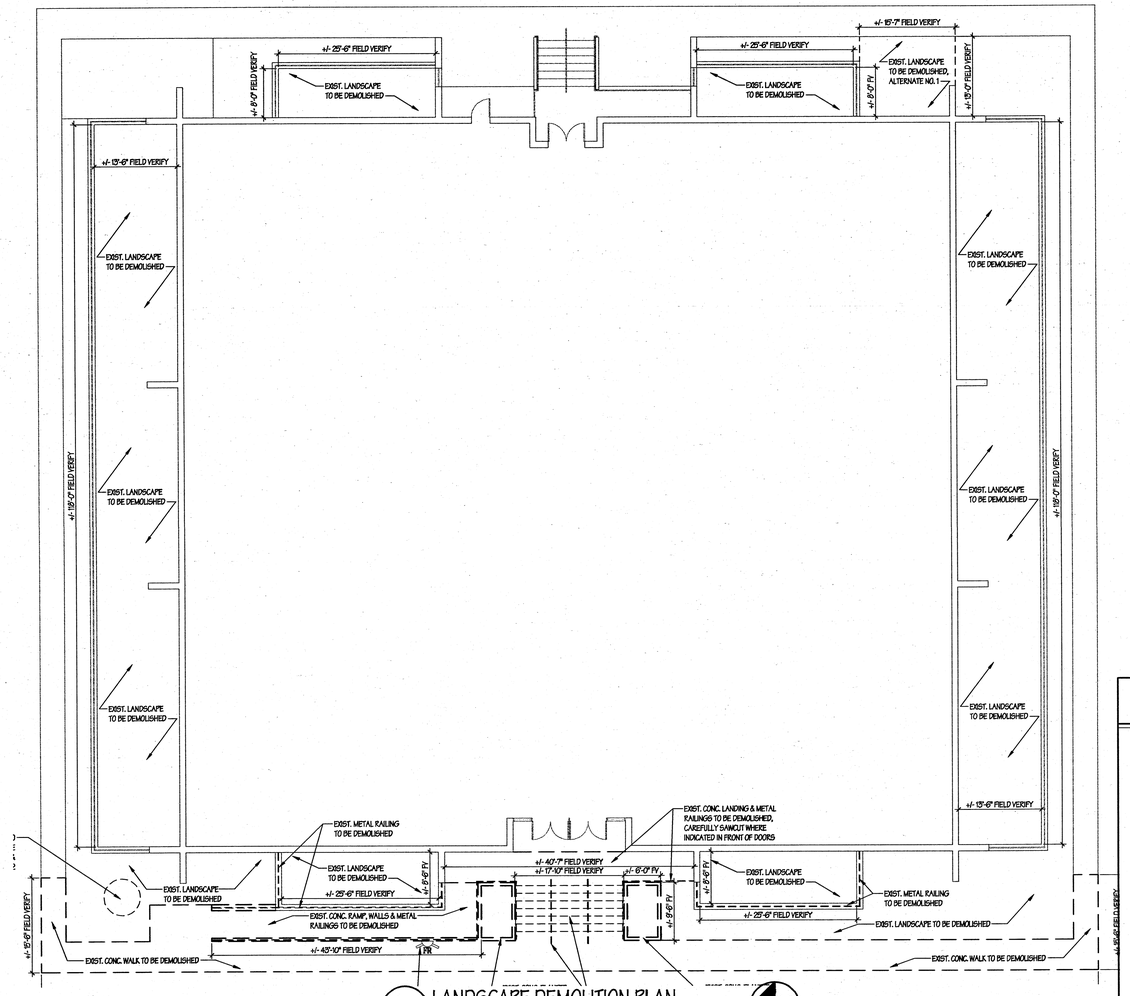

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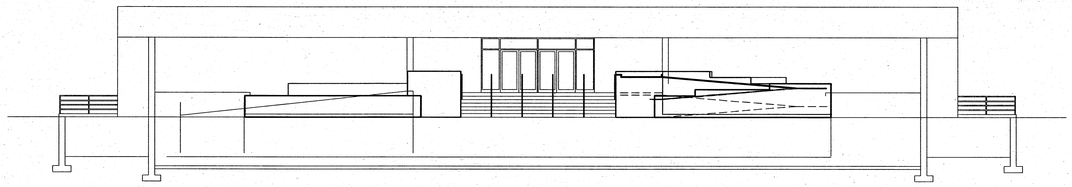
SHEET TITLE

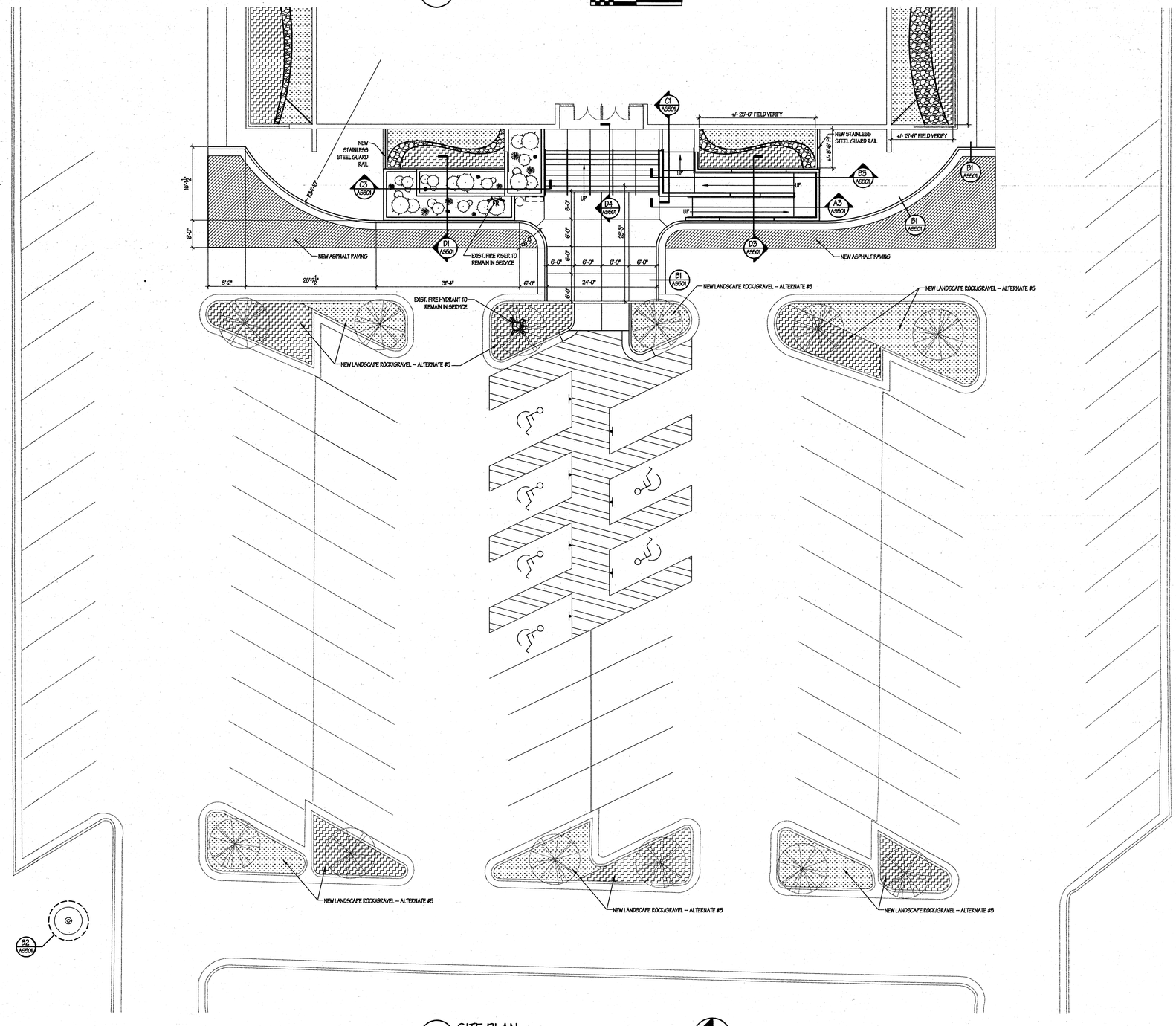
LANDSCAPE DEMO PLAN

AS103

SHEET 4 OF 11



E3 BUILDING ELEVATION
1"=10'-0"



A3 SITE PLAN
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3. NOTED AREAS INDICATED THE GENERAL EXTENT OF DEMOLITION. THE CONTRACTOR'S CHOICE OF MEANS & METHODS OF CONSTRUCTION MAY REQUIRE MORE OR LESS DEMOLITION. THE MEANS & METHODS OF DEMOLITION & CONSTRUCTION MUST BE ACCOUNTED FOR IN THE CONTRACTORS BID. ANY DEMOLITION & REPAIR TO ADJACENT SURFACES BEYOND THE AREAS INDICATED IN THE CONTRACT DOCUMENTS WILL NOT BE COMPENSATED FOR AFTER THE BID OPENING.

4. 72-HOUR NOTICE IS REQUIRED FOR ANY UTILITY SHUT DOWN.

5. CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING ALL EXISTING SITE IMPROVEMENTS WHICH ARE TO REMAIN FREE FROM DAMAGE DURING CONSTRUCTION BOTH INSIDE AND OUTSIDE THE CONTRACT LIMIT LINE.

6. CONTRACTOR SHALL VERIFY EXACT LOCATION OF ALL UTILITIES & SITE IMPROVEMENTS WHICH ARE AFFECTED BY OR WHICH TIE IN WITH NEW CONSTRUCTION.

7. PROVIDE TEMPORARY 6'-0" HIGH CHAIN LINK FENCE @ PROJECT LIMIT LINE, COMPLETE WIGATES AS REQUIRED, UNLESS NOTED OTHERWISE.

8. EXISTING ROADS, PARKING, SERVICE DRIVES, & SIDEWALKS SHALL REMAIN ACCESSIBLE & KEPT CLEAR OF CONSTRUCTION EQUIPMENT, MATERIALS, DIRT AND DEBRIS.

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CONSULTANT

STATE OF UTAH
BARRY W. SMITH
123796
19 March 2009
LICENSED ARCHITECT

DWS - PROVO ENTRANCE REPLACEMENT & LAND-SCAPING IMPROVEMENTS

Department of Workforce Services
1550 North Freedom Boulevard
Provo, Utah

MARK	DATE	DESCRIPTION
	5/20/09	Add #3 Issue

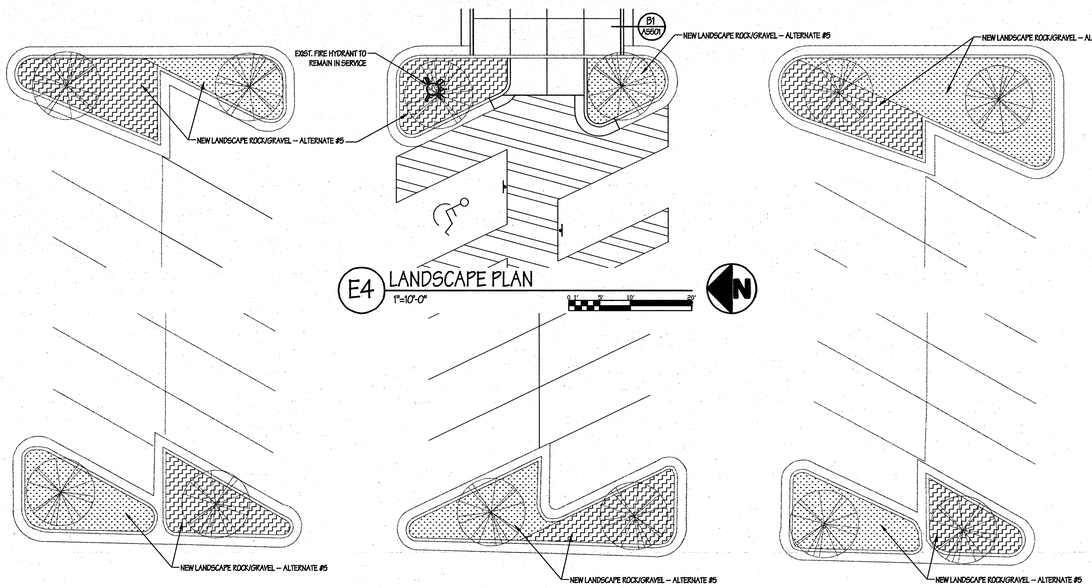
DATE:	29 APRIL 2009
DFCM PROJECT NO:	08071920
HFSA PROJECT NO:	0821.01
CAD DWG FILE NO:	
DRAWN BY:	RLS
CHECKED BY:	BWS
DESIGNED BY:	RLS
DWG TYPE:	ARCHITECTURAL
ARCHITECTURAL PHASE:	CONSTRUCTION BID DOCUMENTS

SHEET TITLE

SITE PLAN

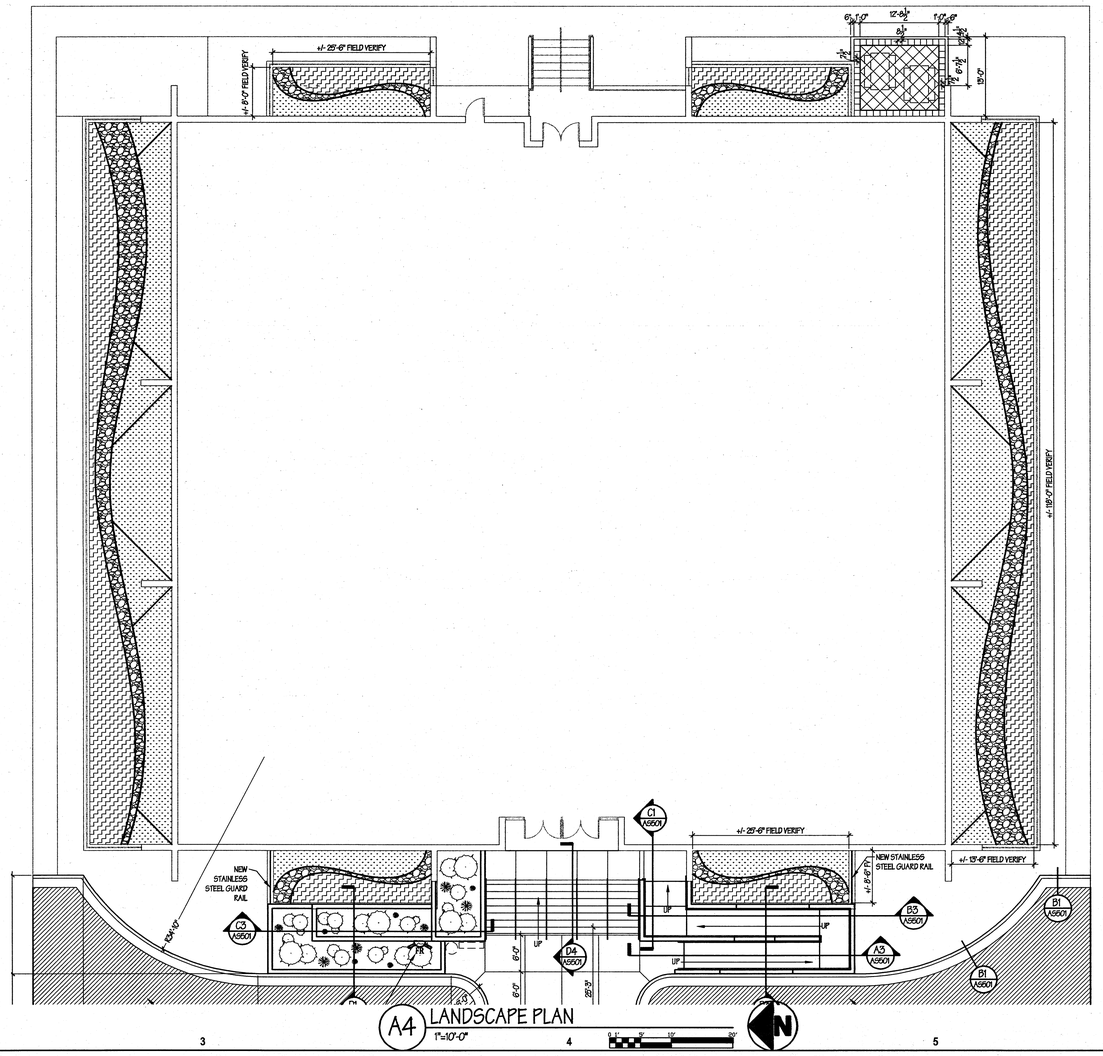
AS104

SHEET 5 OF 11



E4 LANDSCAPE PLAN
1"=10'-0"

D4 LANDSCAPE PLAN
1"=10'-0"



A4 LANDSCAPE PLAN
1"=10'-0"

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LANDSCAPE PLAN LEGEND

ALTERNATE NO. 1: NEW PATIO WITH 6" CONCRETE MOWSTRIP (SEE DTL. A2/AS501), NEW 12"x12" CONCRETE PAVER BORDER (SEE DTL. A1/AS501), NEW 18"x18" CONCRETE PAVER FIELD SET ON DIAGONAL (SEE DTL. A1/AS501), & NEW PICNIC TABLE (NIC).

NEW DRAUGHT RESISTANT FOUNTAIN GRASS

NEW GRAVEL 1

NEW RIVER ROCK

NEW GRAVEL 2

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STATE OF UTAH
BARRY W. SMITH
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19 MAY 2007

DWS - PROVO ENTRANCE REPLACEMENT & LANDSCAPING IMPROVEMENTS

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1550 North Freedom Boulevard
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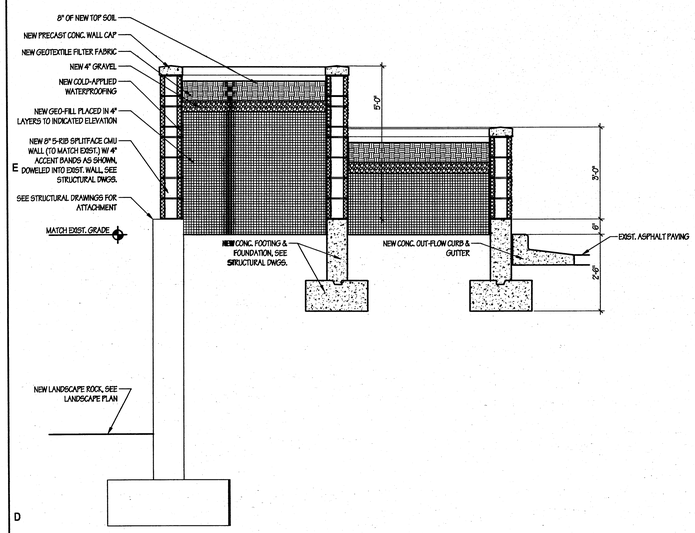
CHECKED BY:	BWS
DESIGNED BY:	RLS
DWG TYPE:	ARCHITECTURAL
ARCHITECTURAL PHASE:	CONSTRUCTION BID DOCUMENTS

SHEET TITLE

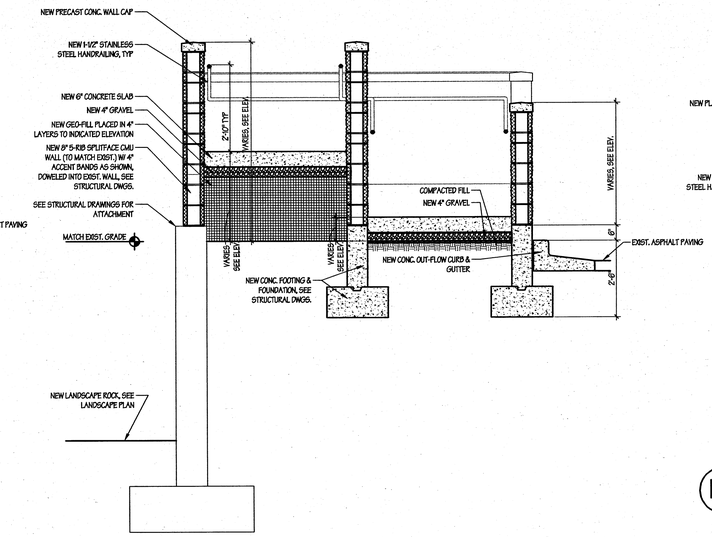
LANDSCAPE PLAN

AS105

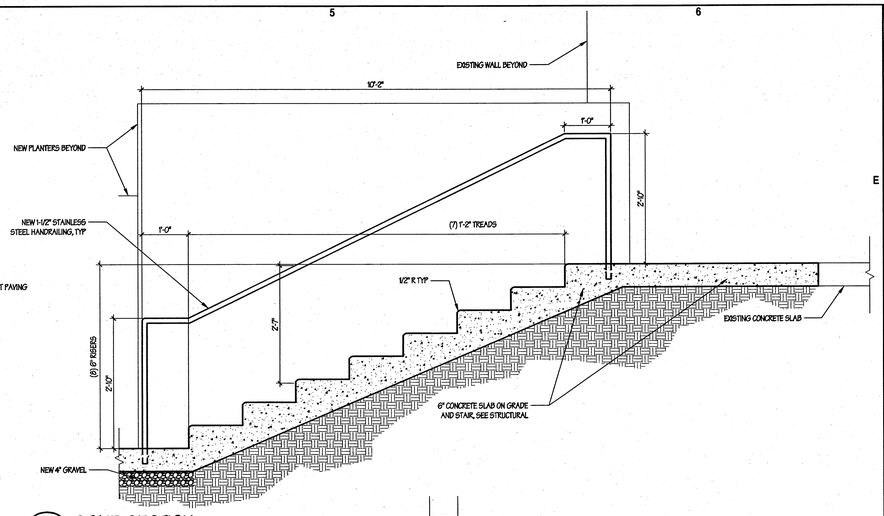
SHEET 6 OF 11



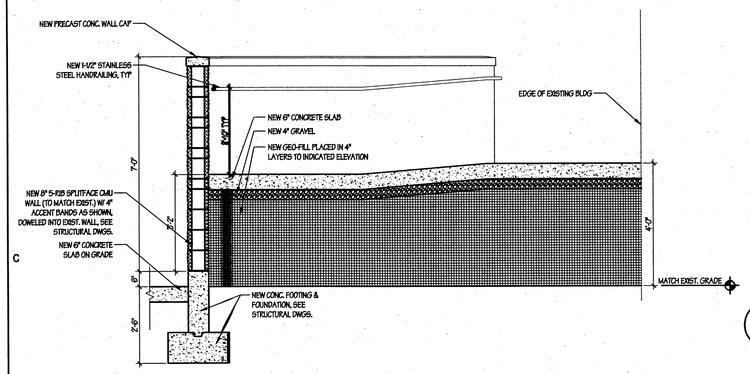
D1 SECTION
1/2"=1'-0"



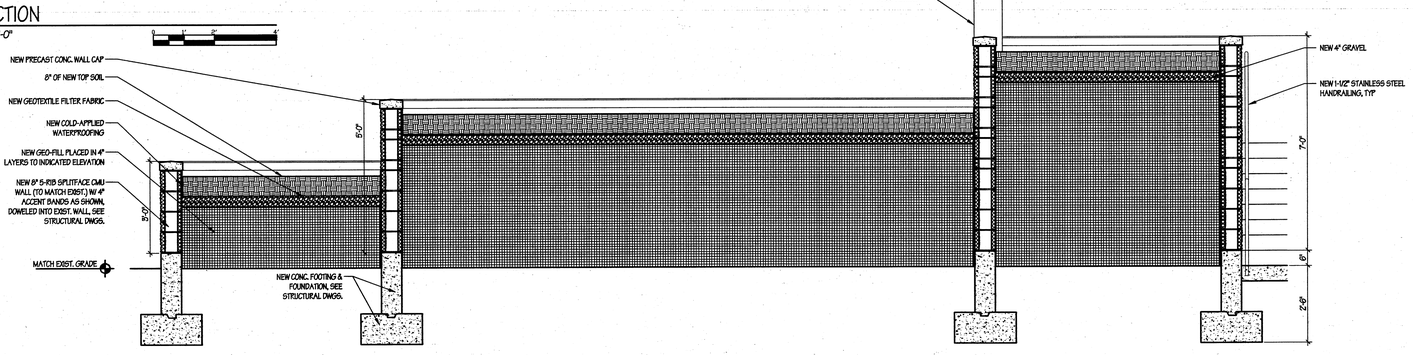
D3 SECTION
1/2"=1'-0"



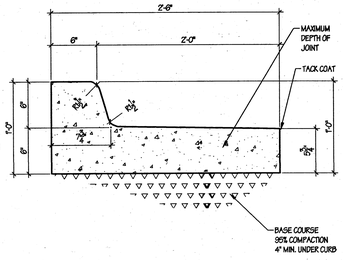
D4 STAIR SECTION
3/4"=1'-0"



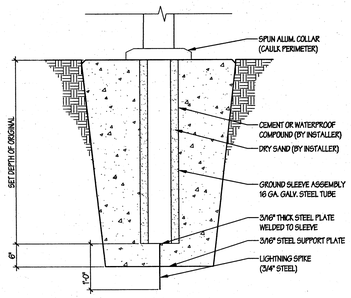
C1 SECTION
1/2"=1'-0"



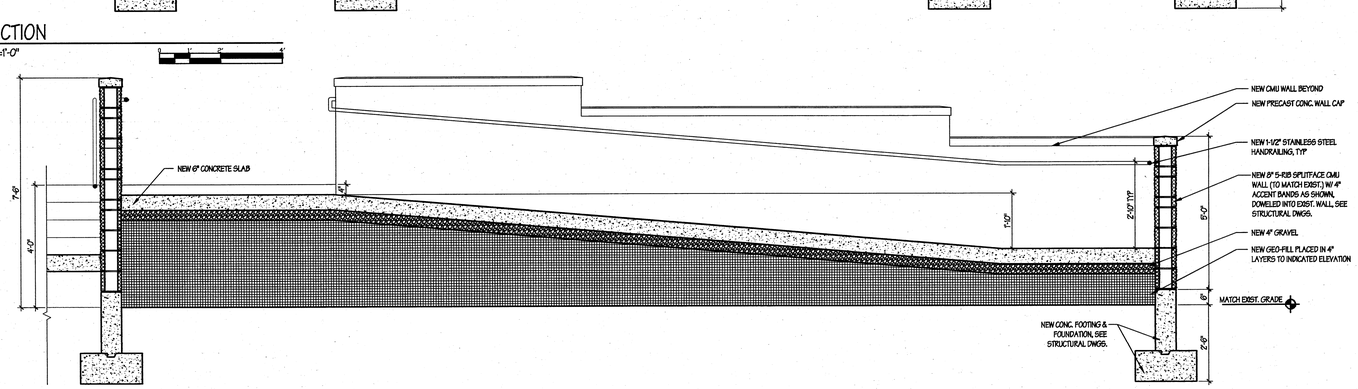
C3 SECTION
1/2"=1'-0"



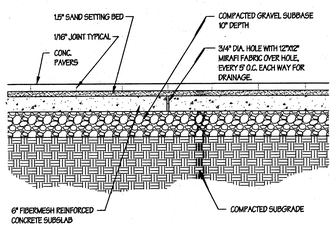
B1 OUT FLOW GUTTER
1-1/2"=1'-0"



B2 FLAG POLE BASE
3/4"=1'-0"

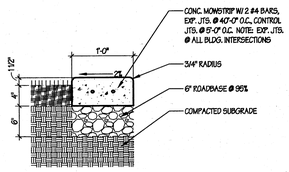


B3 SECTION
1/2"=1'-0"

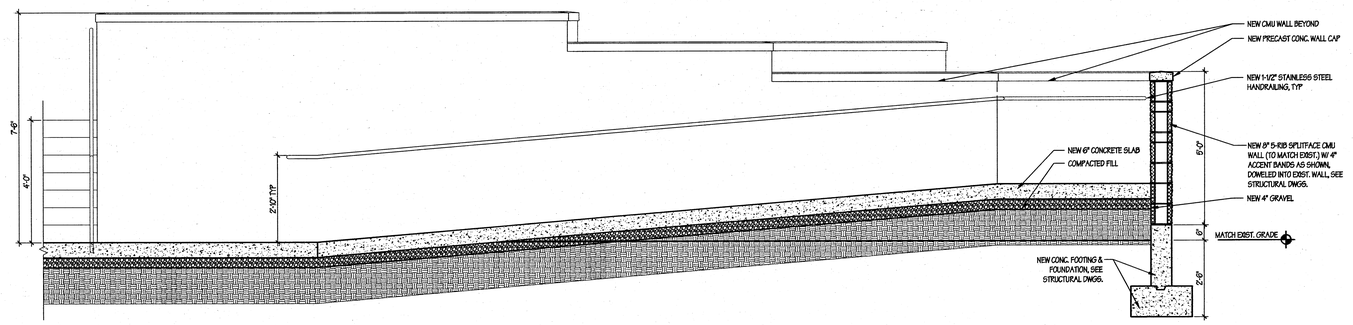


A1 PAVER DETAIL
1/2"=1'-0"

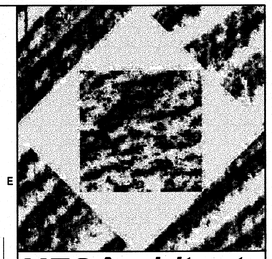
NOTE:
WHEN INSTALLED ADJACENT TO BUILDING, PROVIDE EX. JOINT & SLOPE @ 2% FOR DRAINAGE.



A2 CONCRETE MOW STRIP
1"=1'-0"



A3 SECTION
1/2"=1'-0"



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DESIGNED BY:	RLS
DWG TYPE:	ARCHITECTURAL
ARCHITECTURAL PHASE:	CONSTRUCTION BID DOCUMENTS

SHEET TITLE

SITE SECTIONS & DETAILS

AS501

SHEET 7 OF 11

GENERAL NOTES FOR STRUCTURAL SHEETS

BASIS OF DESIGN

1. BUILDING CODE ..... 2006 IBC
2. OCCUPANCY CATEGORY II
3. GRAVITY DESIGN:
   DEAD LOADS:
   Planter box's ..... 150 psf
   LIVE LOADS:
   Corridors ..... 100 psf
   SNOW LOADS:
   Snow load on ground, (Drifting not included here) Pg ..... 42.8 psf
   Snow load on flat roof, Pf ..... 30 psf
   Exposure factor, Ce 1.0
   Importance factor, Is 1.0
   Thermal factor, Ct 1.0
4. WIND DESIGN:
   Basic wind speed ..... 90 mph
   Importance factor, Iw 1.0
   Exposure ..... B
   Wind Design Pressure ..... 15 psf
SEISMIC DESIGN:
   Importance factor, Ie 1.0
   Mapped Spectral response accelerations:
   Ss & S1 ..... 1.24 & 0.53
   Site class ..... D
   Spectral response coefficients:
   SDs & SD1 ..... 0.83 & 0.53
   Seismic Design Category ..... D
   Basic Seismic-Force-Resisting System:
   Bearing wall systems, Special reinforced masonry shear walls
   Seismic response coefficient, Cs 0.12
   Response modification factor, R 5
   Analysis procedure ..... Equivalent Lateral Force
5. SOILS:
   Soil bearing pressure ..... 1500 psf
   Lateral earth pressure ..... 61 psf
   Minimum frost cover ..... 30 inches
6. ABBREVIATIONS:
   EOR = Engineer of record. See professional stamp this page.
   UNO = Unless noted otherwise
   (E) = Existing condition
   (N) = New construction

GENERAL

1. THE GENERAL CONTRACTOR SHALL:
   A. Be familiar with the contract documents and insure that subcontractors are familiar with their portion of the work. Submit a written request to the Arch/EOR for approval before proceeding with any changes.
   B. Verifiee site conditions and dimensions at the site. If they differ from the contract documents, notify the Arch/EOR prior to fabrication/construction of affected elements. Existing condition information on the drawings is based on best knowledge acquired during the design phase and may differ from actual conditions. Affected details may require redesign.
   C. Report to the Arch/EOR modifications made to the structure.
   D. Be responsible for safety and protection on and around the job site and adjacent properties.
2. THE GENERAL CONTRACTOR SHALL COORDINATE:
   A. And verify locations, weights and sizes of mechanical units, equipment, etc. prior to the fabrication and erecting of structural supporting elements. Report sizes and locations that differ from those shown on the drawings to the Arch/EOR for review. Additional framing maybe required.
   B. Floor, and wall openings required for mechanical, etc. which are not shown on the structural drawings with the Arch/EOR.
   C. Any structural situation not covered by the drawings with the Arch/EOR.
   D. Walls, elevations, slopes, stairs, curbs, drains, recesses, depressions, railings, waterproofing, finishes, chamfers, kerfs, landscape walls, trenches in slabs, etc. with the structural work.
   E. Inspections, testing, and structural observations as work proceeds. Notify the EOR 48 hours prior to any required structural observations.
3. CONTRACT DOCUMENTS & DRAWINGS:
   A. These structural notes complement the specifications and the drawings.
   B. Specific details, sections and notes shown on the drawings govern over these general notes and typical details.
   C. Contract documents take precedence over shop drawings, UNO.
   D. Apply typical or similar details, sections and notes to similar situations on the drawings where specific details are not referenced.
   E. Drawings and details have been prepared to visually represent information provided in scaled form. However, DO NOT scale plans or details for dimensional information.
   F. Refer to architectural drawings for dimensions.
4. BUILDING CODE COMPLIANCE: Construction, inspection, materials, testing, and workmanship shall conform to the requirements of the governing building code.
5. CONSTRUCTION SEQUENCE, SHORING, AND BRACING REQUIREMENTS: The general contractor is responsible for the method, means, and sequence of structural erection, UNO. He shall provide adequate temporary shoring or bracing for all structural elements until the entire structural system is completed. Design of shoring and bracing is by others at no additional cost to the owner.
6. OMISSIONS, CONFLICTS & DISCREPANCIES:
   A. Bring omissions, conflicts or discrepancies between the elements of the contract documents to the attention of the Arch/EOR before proceeding with work involved.
   B. In case of conflicts or discrepancies, follow the most stringent requirements as directed by the Arch/EOR.
7. MISCELLANEOUS:
   A. During and after construction, builder and/owner shall keep loads on the structure within the limits of this design. See Basis of Design.
   B. Site observations by WCA's field representative shall neither be construed as Inspection nor approval of construction.
8. SUBMITTALS:
   A. Make submittals in a timely manner. WCA's review is for general compliance only and is not intended as approval. Contractor is responsible for verifying sizes, dimensions and elevations on submittals as related to the contract documents.
   B. Submit the following items for review prior to proceeding with the work:
      Concrete material Certifications & mix designs.
      Masonry material Certifications, grout & mortar designs.
      Shop Drawings:
      Reinforcing steel
      Steel deck
      Structural steel
      Wall openings not shown on the drawings.
      Welding procedures and certifications.
   C. Allow two weeks for the review of submittals by the EOR.
   D. Have EOR approved shop drawings & materials on site before construction of those components begins.
   E. Substitutions are not allowed unless approved by the EOR. Submit requests for structural substitutions to the Arch/EOR.

SPECIAL INSPECTION AND TESTING

1. INSPECTIONS: Provide special inspection by an independent agency in accordance with IBC Chapter 17 and as outlined below:
   Anchor bolts and concrete & masonry embedments.
   Concrete: during pours, rebar placement, and taking of test specimens. Inspections shall be ACI-II or ICC certified.
   Masonry: during placement, grouting, reinforcing, and taking of specimens. Inspectors shall be ICC certified.
   Reinforcing steel: in concrete and masonry.
   Soils: bearing materials and placement of structural fill.
   Steel Deck: (periodic)
   Welding: all field welds. Inspector shall be AWS-QC1 certified.
   Drill and epoxy anchors: (continuous)
2. TESTING: The owner will provide testing by qualified testing personnel for the following types of construction:
   Concrete: strength, slump, air, and temperature.
   Masonry: strength of mortar, grout, block, and prisms.
   Soils: compaction.
   Welding: type, size, length, and quality of shop and all field welds by approved methods. Ultrasonically test complete penetration welds.
   Drill and epoxy anchors: load test 10% of anchors, with a minimum of (2) anchors tested
3. THE CONTRACTOR SHALL:
   A. Coordinate testing. DO NOT proceed with subsequent work until inspections and testing has been approved.
   B. Copy inspection reports/testing results to the Arch/EOR and owner before work proceeds.
   C. Correct deficient work at no additional cost to the owner.

FOUNDATIONS

1. SOILS REPORT:
   A. Foundations and retaining wall systems have been designed following the recommendations contained in a soils report prepared by:
      _____
      Dated: ___.
      Obtain a copy from the architect. This design assumes the recommendations contained in this report are being followed.
   B. If soil conditions vary from the report or if the report has been amended, etc., the contractor shall immediately inform the EOR. Foundations as shown on the drawings may require revision.
2. Soil preparation under footings and slabs-on-grade shall be in accordance with the soils report.
3. The general contractor is responsible for soil excavation, back fill and support of adjacent properties during earthwork.
4. All walls (except cantilevered retaining walls) shall be adequately brace against lateral movement prior to backfilling. Design and erection of bracing/shoring is the general contractor's responsibility. Bracing shall remain in place until supporting structural elements are in place and have attained full strength. If walls are not braced, DO NOT backfill until suspended floor is in place and concrete has attained full strength.
5. Requirements:
   A. ____ (engineer – add site specific info)

REINFORCING STEEL

1. CODES AND STANDARDS. Comply with:
   A. CRSI "Manual of Standard Practice".
   B. ACI "Detailing Manual", ACI 315 (or SP-66).
2. MATERIALS:
   A. New stock deformed rebar: ASTM A615, Grade 60, except as noted.
      a. Field bent dowels: ASTM A615, Grade 40 or ASTM A706, Grade 60, Low-Alloy Steel. Reduce spacing of grade 40 dowels by 1/3.
      b. Welded rebar: ASTM A706, Grade 60, Low-Alloy Steel.
      c. Deformed bar anchors (DBA's): See materials under section 'Structural Steel'
   B. Masonry joint wire: ASTM A82.
3. CONSTRUCTION:
   A. Detail, bolster, and support all rebar. Tie bars securely with proper clearances before casting concrete.
   B. Use rebar free of loose flaky rust, scale, grease, oil, dirt, and other materials, which affect or impair bond.
   C. Place rebar continuous in walls, beams, columns, slabs, footings, etc.
   D. Minimum lap splices (inches), for normal weight concrete and masonry with strengths specified per 'General Structural Notes', UNO:

	#3	#4	#5	#6	#7	#8	#9	#10	#11
Concrete:	24"	24"	24"	26"	36"	44"	48"	56"	62"
Masonry:	24"	24"	30"	36"	42"	48"	54"	*	*

*Use mechanical splice couplers

In suspended slabs and beams, make top bar splices at mid-spans and bottom bar splices at supports, UNO.

   E. Make cold bends. DO NOT use heat. Bends in the fabricator's shop, UNO. DO NOT unbend or rebend a previously bent bar.
   F. Minimum concrete cover: (securely position and anchor rebar prior to pour)
      Cast against and permanently exposed to earth ..... 3"
      Exposed to earth or weather:
      #6 and larger ..... 2"
      #5 and smaller ..... 1-1/2"
      NOT exposed to earth or weather:
      Slabs, walls and joist, #11 & smaller ..... 3/4"
      Beams, columns: Main reinforcing or ties ..... 1-1/2"
      Slabs-On-Grade (SOG) ..... Center of slab, UNO
   G. In masonry, place and position rebar according to the structural drawings while laying units. Secure against displacement at intervals not to exceed the following:
      #4 and smaller ..... 6'-0"
      #5 and larger ..... 10'-0"
   H. DO NOT weld reinforcing unless specifically noted. Use E-90XX electrodes and ASTM A706 reinforcing. Comply with AWS requirements.
   I. Use epoxy coated reinforcing when specifically noted. Increase lap splice lengths by a factor of 1.2.

CONCRETE

1. CODES AND STANDARDS. Comply with the following Codes:
   A. ACI 301, "Specifications for Structural Concrete for Buildings".
   B. ACI 318, "Building Code Requirements for Reinforced Concrete".
   C. ACI 347, "Recommended Practice for Concrete Form Work".
2. MATERIALS shall conform to the following:
   A. Cement: ASTM C150, Type I, Portland Cement.
   B. Hard rock aggregates: ASTM C33
   C. Water shall be potable.
   D. Air entrainment: ASTM C260
   E. Fly ash: ASTM C618
   F. Calcium chloride SHALL NOT be used.
3. MIX DESIGNS:
   A. Place only one type of concrete at any given time.
   B. The maximum slump shall be 4" w/o plasticizer added.
   C. Limit fly ash to 20% of the total cement.
   D. Concrete mixes shall conform to the following:

TYPE OF CONCRETE MEMBER	28 DAY STRENGTH (psi)	MAX. W/C (ratio)	MAX. DRY WEIGHT (pcf)	MAX. AGGREGATE SIZE (inches)	AIR (%)	COEF. of SHRINKAGE (in./ft.)
Footings:	3000	0.50	150	3/4	3 ±1	
Foundation walls :	4000	0.45	150	3/4	3 ±1	.003
Slabs on grade: Exterior	4000	0.47	150	1.5"	6 ±1	.003

*Well-graded Aggregates required (1.5"=> course >3/8", medium, and fines <#8) Follow ACI 302 for sand gradation.

4. CONSTRUCTION:
   A. Mechanically vibrate concrete during placement.
   B. Prior to placing concrete, check with trades to insure proper placement of openings, block outs, sleeves, curbs, conduits, bolts, inserts, embeds, dowels, etc. Place anchor bolts and dowels prior to casting concrete, UNO.
   C. Form construction joints and bulkheads with a key way. Intentionally roughen contact surfaces (new or existing) at construction joints prior to casting adjacent pours, UNO.
   D. Add additional reinforcing too sides of floor and wall opening, equivalent to the bars cut by the opening with half to each side of the opening or (2) #5 bars, whichever is greater, UNO. Bars parallel to the principal reinforcing shall run full length of the span. End bars in the other direction with a standard hook. Add (2) #5 x 5'-0" diagonal bars at every corner.
   E. DO NOT allow penetrations through any beam, joist, column, pier, footing, or jamb without the EOR's approval. Otherwise, re-rout the penetration.
5. FOOTINGS:
   A. Bear footings on properly prepared materials.
   B. Center footings on the wall or column above, UNO.
   C. Bear exterior footings below the effects of frost. See Basis of Design.
   D. Provide 2x4 beveled key in continuous wall footings.
   E. Stagger footing construction joints from wall construction joints above by at least 6 feet.
   F. Provide corner bars in continuous footings at corners and intersections.
   G. Add (2) #6 or (3) #5 lengthwise too bars in addition to footing schedule reinforcement at continuous footings without concrete foundation walls directly above (door openings, etc.).
   H. DO NOT allow penetrations through any concrete footing. At utilities, step the footing down below the conflict and add a concrete wall, pier or column that extends to the footing. Consult with the EOR.
   I. Backfill bearing surfaces that are undermined during construction with a lean concrete mix (1000 psi min.).
6. SLABS ON GRADE (SOG):
   A. Minimum Interior slabs on grade requirements:
      4 inches thickness.
      4 inch layer of free-draining gravel base.
      #4 bars at 18" o.c. both ways, UNO. Chair rebar for proper placement.
   B. Place large areas of interior slabs-on-grade in strips not to exceed 120 feet in length nor 20 feet in width. Subdivide by construction or contraction (control) joints into roughly squares whose sides DO NOT exceed 10 feet in either direction.
   C. See Architectural for exterior slabs on grade, UNO.
7. WALLS:
   A. Place vertical reinforcing in the center of walls (UNO) unless each face (E.F.) is specified. When each face is specified, splice the horizontal reinforcing of each curtain at different locations.
   B. Dowel vertical reinforcing to the structure below and above with the same bar size and spacing, UNO.
   C. Terminate horizontal reinforcing at the ends of walls or openings with a standard hook or corner type bars. Provide corner bars of the size and spacing as the horizontal reinforcing at intersections and corners.
   D. Build penetrations into the wall before pouring concrete. Have the penetrations reviewed by the EOR prior to installation unless detailed on the plans.
   E. Provide drains at the base of retaining and basement walls.
   F. Concrete walls not specially noted on the plans shall be reinforced as follows:

WALL THICKNESS	VERTICAL REINFORCING	HORIZONTAL REINFORCING	TOP & BOT. REINFORCING HORZ.
8"	#5 @ 18" o.c.	#5 @ 12" o.c.	(2) #5

See plans, schedules, and details for other reinforcing.

MASONRY

1. CODES AND STANDARDS: Comply with ACI 530, "Building Code Requirements for Masonry Structures".
2. MATERIALS:
   A. Minimum net area compressive strength of masonry, fm = 2000 psi.
      1. Determine the compressive strength of masonry by either of the two following methods:
         a. Unit strength method.
            1. Lightweight concrete masonry units. ASTM C90, Grade N, Type 1. Minimum compressive strength = 2800 psi at 28 days (net area).
            2. Grout shall conform to ASTM C476 with a minimum compressive strength of 3000 psi at 28 days. Use a fluid consistent grout, which may contain additional pea gravel if grout spaces are 4" or more in every direction. Limit fly ash to 20% of the total cement.
         b. Prism test method.
            1. Minimum compressive strength of masonry prism tests at 28 days, f'm as specified above.
      B. Type "S" Portland cement-lime mortar. Minimum compressive strength = 1800 psi at 28 days. No additives.
1. CONSTRUCTION:
   A. Store masonry under cover at the job site.
   B. Fully bed face shells.
   C. Tool Mortar joints concave.
   D. DO NOT use mortar for grout.
   E. DO NOT use any frozen materials.
   F. Use either low or high lift grouting procedures.
   G. Consolidate grout by mechanical vibration during placing and reconsolidated after excess moisture has been absorbed but before workability is lost (45 minutes max.).
   H. Grout solid cells containing rebar, bolts, anchors, etc.
   I. Grout steel joist and steel beam pockets solid, UNO.
   J. Provide 1" of grout around bolts in side shells.
   K. DO NOT allow penetration through any beam, column, pier, or jamb without the EOR's approval. Otherwise, re-route penetrations at those locations.
   L. Prior to placing masonry, check with trades to insure proper placement of openings, block outs, sleeves, conduits, bolts, inserts, embeds, dowels, etc.
4. WALLS:
   A. Use running bond. Build corners and intersections as an integral unit.
   B. Dowel vertical reinforcing to the structure below and above with the same size bar and spacing, UNO.
   C. Place vertical reinforcing at the centerline of the wall unless each face (E.F.) is specified, UNO.
   D. Provide vertical reinforcing in grouted cells at corners and intersections.
   E. Terminate horizontal reinforcing at wall ends or openings with standard hooks or corner type bars. Provide corner bars of the same size bar and spacing as the horizontal reinforcing at corners and intersections.
   F. Make horizontal bars continuous where concrete walls, columns, or pilasters interfaces. Provide a lap between the masonry and concrete. Grout lay solid.
   G. Construct bond beams at the top course and at floor and roof diaphragm interfaces.
   H. Construct penetrations thru walls as they are being laid. Add 2-#5 bars in grouted cells on all sides of opening which exceed 24 inches in either direction, UNO. Extend vertical edge bars the full height of the wall between floor or roof support. Extend horizontal edge bars 24 inches beyond the opening edges.
   I. DO NOT place construction or expansion joints in beams, headers, columns or supports, UNO.
   J. Reinforce masonry walls as follows, UNO.

WALL THICKNESS (with joint reinforcing)	VERTICAL REINFORCING	HORIZONTAL REINFORCING
8"	1-#5 at 32" o.c.	2-#4 at 48" o.c.

Add ladder-type joint reinforcing of 2-#9 wires (3-#9 wires with veneer) at 16" o.c. horizontally in all masonry walls. See plans, schedules, and details for other reinforcing.

See plans, schedules, and details for other reinforcing. Modify these reinforcing spacing as necessary such that they are less than one third the height or one third the length of the wall, whichever is smaller (see ACI 530 1.14.2.2.5(a)

EPOXY

1. PRODUCT:
   A. Epoxy for Concrete Connections shall be:
      1. "Hilti RE 500" by Hilti Corporation
      2. "Set-XP" by Simpson Strong Tie
   B. Epoxy for Masonry Connections shall be:
      1. "Hilti RE 500" or "Hilti HY 150 Max" by Hilti Corporation (grouted cells)
      2. "Hilti HY 20" by Hilti Corporation (un-grouted cells, with screen tubes)
      3. "Set Epoxy-Tet" by Simpson Strong Tie
      4. "Powers-Fast" by Powers Fasteners
2. INSTALLATION:
   A. All drilled holes for anchors rods 1" in diameter and smaller shall be 1/8" larger than the anchor rod being installed. All drilled holes for anchor rods greater than 1" in diameter shall be 1/4" larger than the bar or anchor rod being installed.
   B. After drilling the proper size hole, clean the walls and bottom of the drilled hole of all dust and debris using a nylon brush in conjunction with oil free compressed air. The hole shall be free of dust, debris and standing water.
   C. Follow all manufacturers' recommendations for epoxy installation.

STRUCTURAL STEEL

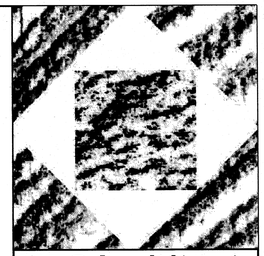
1. CODES AND STANDARDS: Comply with:
   A. AISC "Specification for Structural Steel Buildings & Commentary".
   B. AISC "Code of Standard Practice" excluding sections 7.5.4, and 7.11.5.
   C. AWS "Structural Welding Code", exclude items conflicting with AISC.
2. MATERIALS SHALL CONFORM AS FOLLOWS:
   A. Misc. shapes & plates: ASTM A53, Fy = 36 ksi.
   B. Hollow Structural Sections: ASTM A500, Fy = 46 ksi, Gr. B.
   C. Pipe: ASTM A53, Fy = 35 ksi, Gr. B.
   D. Other bolts: ASTM A307 or better.
3. CONSTRUCTION:
   A. Fabricate in an approved fabricator's shop.
   B. Fabricate beams with incidental camber up, UNO.
   C. Use 6000 psi (minimum at 28-day) non-shrink liquid grout beneath bearing plates. Place grout per manufacturer's recommendations prior to loading member.
4. BOLTED CONNECTIONS:
   A. Use 3/4" diameter bolts in Std. holes (bolt diameter + 1/16"), UNO.
   B. Connections: Use ASTM A307 bolts or better, UNO.
   C. Use hardened washers beneath the turned element of the bolt or nut. Use beveled hardened washers where the outer face of bolted parts has a slope greater than one in twenty with respect to the plane normal to the bolts axis. At oversized holes, use hardened washers or plates at least 5/16" thick conforming to ASTM F436.
   D. Tighten bolts until all plies of the joint are in firm contact. Snug tight condition, UNO.
   E. Enlarge bolt holes by reaming. DO NOT torch cut.
5. WELDED CONNECTIONS:
   A. Perform welding and cutting by AWS certified welders in accordance with ANSI/AWS D1.1 (latest edition).
   B. For typical shop & field welds, use filler metals with nominal 70 ksi tensile strength having:
      a. Matching material for multiple pass welds.
      b. A diffusible hydrogen limit of H16 or less.
      c. A CVN toughness of 20 ft-lbs at 0 deg. F.
   C. Use pre-qualified procedures.
   D. Weld intersecting steel shapes together, which are not connected with bolts, with all-around fillet welds, UNO.
   E. Weld studs and DBA's according to Manufacture's specs.
   F. Whenever possible use shop welds. The contractor shall coordinate field and shop welds between shop fabrication and the steel erector.
   G. Remove slag from welds.

STEEL DECK

1. PRODUCT:
   A. See plans for deck size and gauge, etc.
   B. Manufacture deck and accessories from cold rolled steel conforming to ASTM A653 SS Grade 33 (galvanized G60), ASTM A1008 SS Grade 33 (painted), or ASTM A653 SS Grade 80 (Form Deck) , and conform to the Steel Deck Institute and AISC Standards.
2. CONSTRUCTION:
   A. Install deck with a minimum of 3 spans, UNO. Where such layout is impossible, decking must meet design criteria for simple span condition.
   B. Provide 2" minimum deck bearing and 4" lap at splice locations. Provide structural support (angles, etc.) around the perimeter of all deck openings.
   C. DO NOT bend or mar deck.
   D. Store decking off the ground with one end elevated. Cover with waterproof material and ventilate to avoid condensation.
3. DECK WELDING REQUIREMENTS:
   A. See plans for welding pattern.
   B. Weld to framing members with E60XX or E70XX electrodes.
   C. Use 3/4" diameter puddle welds. Penetrate all deck layers with weld metal and have proper fusion to the supporting steel.
   D. Crimp side seams before welding side laps or use Verco's Punchlok system w/ connections @ 12" o.c. max. Engage all layers of the deck material. (A standard button punch can NOT be used in place of Verco's Punchlok.)
   E. Overlap end laps at least 2" directly over a single steel support. Place welds 1" from the edge of the deck or more.
   F. Welds 3/8" x 1.5" long may replace 3/4" diameter welds.

GEOFOAM

1. PRODUCT:
   A. Geofoam under ramp Slab-On-Grade: TYPE EPS15 Geofoam.
2. MATERIAL: Conform to ASTM D6817.
   Density 0.9 pcf
   Flexural Strength 25.0 psi
   Min. Compressive Resistance 3.6 psi @ 1% deformation
3. CONSTRUCTION:
   A. Subgrade Preparation:
      a. Compact top 12" of (E) soil materials to 95% .
      b. Place a 2" thick sand layer (minimum) over the prepared surface.
   B. Geofoam Installation:
      a. Verify density of Geofoam delivered to the site. See MATERIAL section above. Have (2) tests made. Test must meet the MATERIAL requirements.
      b. Tightly place blocks of Geofoam on the prepared sand layer and adjacent to foundation walls.
      c. Multiple Block Layers: Orient successive layers of blocks at 90 deg. to the previous layer. Offset block joints between layers.
      d. A minimum gravel layer of 6" is required between the Geofoam and Slab-On-Grade.
      e. Cut geofoam to fit ramp configuration. Ramp & landings next to (E) window well, UNO.



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LICENSED CERTIFIED STRUCTURAL ENGINEER No.160916 KENNETH PAUL WILLMORE
5/20/09

DWS - PROVO ENTRANCE REPLACEMENT & LAND-SCAPING IMPROVEMENTS

Department of Workforce Services
1550 North Freedom Boulevard
Provo, Utah

MARK	DATE	DESCRIPTION
	5/20/09	Add #3 Issue.

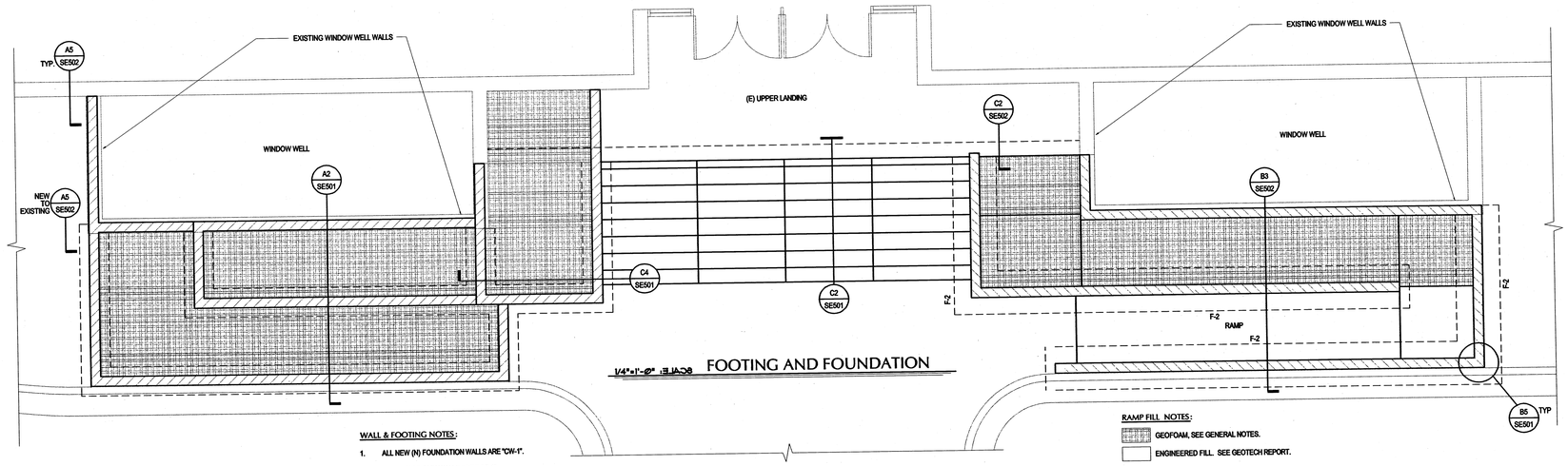
DATE:	29 APRIL 2009
DFCM PROJECT NO:	08071920
HFSA PROJECT NO:	0821.01
CAD DWG FILE NO:	WCA file # 09035
DRAWN BY:	
CHECKED BY:	DW
DESIGNED BY:	KPW
DWG TYPE:	ARCHITECTURAL
ARCHITECTURAL PHASE:	CONSTRUCTION BID DOCUMENTS

SHEET TITLE

GENERAL NOTES (GSN)

SE101

SHEET OF XX



FOOTING AND FOUNDATION
1/4"=1'-0" SCALE

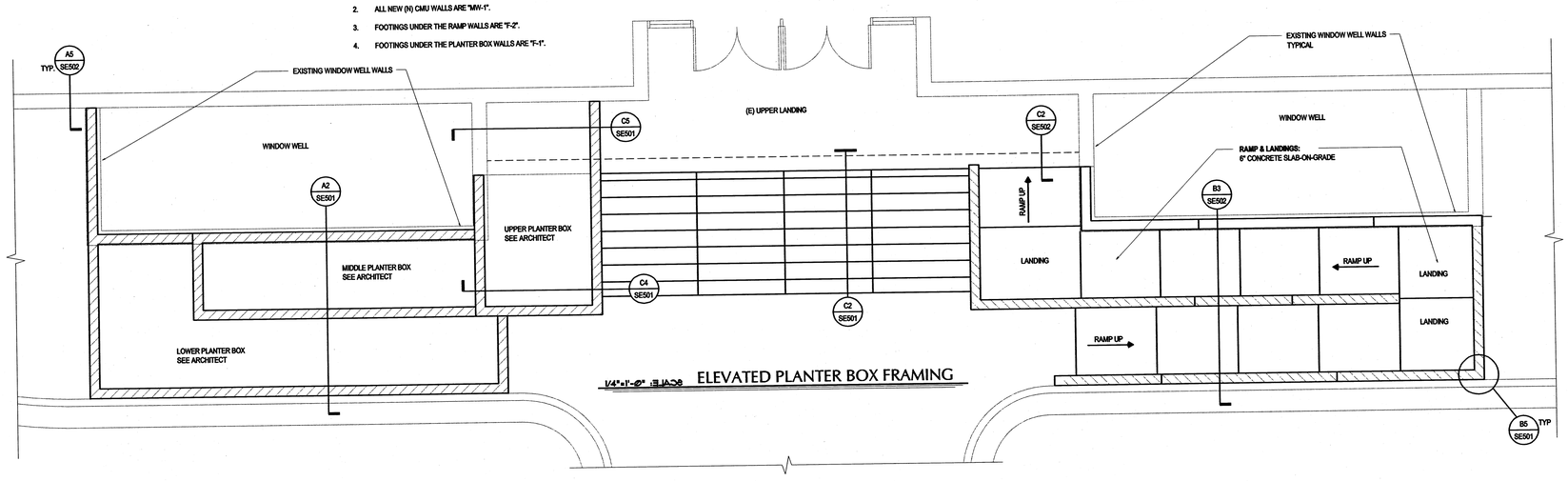
WALL & FOOTING NOTES:

1. ALL NEW (N) FOUNDATION WALLS ARE "CW-1".
2. ALL NEW (N) CMU WALLS ARE "MW-1".
3. FOOTINGS UNDER THE RAMP WALLS ARE "F-2".
4. FOOTINGS UNDER THE PLANTER BOX WALLS ARE "F-1".

RAMP FILL NOTES:

GEOFOAM, SEE GENERAL NOTES.

ENGINEERED FILL. SEE GEOTECH REPORT.

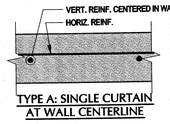


ELEVATED PLANTER BOX FRAMING
1/4"=1'-0" SCALE

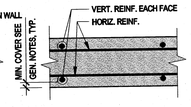
CONCRETE WALL SCHEDULE

MARK	THICKNESS	VERTICAL REINF.	HORIZONTAL REINF.	NOTES
CW-1	8"	#5 @ 18" O.C.	#5 @ 12" O.C.	(2) #5 TOP & BOTTOM, TYPE 'A'

1. ALL WALLS ARE MARK CW-1, U.N.O..
2. CONTRACTOR TO VERIFY ALL WALL THICKNESS W/ ARCHITECT.
3. PROVIDE CORNER BARS AT ALL CORNERS AND INTERSECTING WALLS, SEE DETAIL B5/SE501.
4. WHEN A SINGLE CURTAIN OF REINFORCING IS SPECIFIED, PLACE THE VERTICAL REINFORCING IN THE CENTER OF THE WALL, TYPICAL, U.N.O..
5. WHEN A DOUBLE CURTAIN OF REINFORCING IS SPECIFIED, PLACE EACH CURTAIN OF STEEL AT THE FACE OF THE WALL WITH MINIMUM COVER AS SPECIFIED IN THE GENERAL NOTES. PLACE THE VERTICAL REINFORCING CLOSEST TO THE FORMS, TYPICAL, U.N.O..
6. PROVIDE DOWELS WITH STANDARD HOOKS TO THE STRUCTURE BELOW WITH SIZE AND SPACING TO ATTACH THE VERTICAL REINFORCING IN THE WALL ABOVE.
7. SPLICE VERTICAL REINFORCING AT FLOOR LEVELS ONLY, TYPICAL U.N.O..
8. SPLICES IN HORIZONTAL REINFORCING IN ONE CURTAIN SHALL BE STAGGERED FROM SPLICES IN THE OPPOSITE CURTAIN A MINIMUM OF FOUR FEET.



TYPE A: SINGLE CURTAIN AT WALL CENTERLINE



TYPE B: DOUBLE CURTAIN

FOOTING SCHEDULE

MARK	SIZE			BOTTOM REINFORCING	TOP REINFORCING	NOTES
	WIDTH	LENGTH	THICK			
F-1	2'-0"	CONT.	12"	(2) #5 CONTINUOUS	NONE	
F-2	3'-0"	CONT.	12"	(3) #5 CONTINUOUS	NONE	

C.W. = CROSSWISE E.W. = EACH WAY L.W. = LENGTHWISE

1. ALL FOOTINGS ARE MARK F-1, U.N.O..
2. BEAR FOOTINGS ON PROPERLY PREPARED MATERIAL.
3. BEAR EXTERIOR FOOTINGS BELOW THE EFFECTS OF FROST.
4. CENTER FOOTINGS BELOW THE WALL AND/OR COLUMN ABOVE, TYPICAL UNO.
5. PROVIDE 2x4 BEVELED KEYWAYS IN CONTINUOUS WALL FOOTINGS. UNO
6. PROVIDE 3" CLEAR CONCRETE COVER AT BOTTOM BARS, UNO.
7. PROVIDE DOWELS WITH STANDARD HOOK FROM FOOTINGS TO ANY REINFORCED ELEMENT ABOVE. DOWEL SIZE TO MATCH VERTICAL REINFORCING IN ELEMENT ABOVE. UNO
8. ANY INCREASE IN THE SIZE OF FOOTINGS FOR CONSTRUCTION CONVENIENCE, MAY REQUIRE ADDITIONAL REINFORCING. COORDINATE WITH THE EOR
9. ALL FOOTINGS TO BEAR ON ENGINEERED FILL. SEE SOILS REPORT.

MASONRY WALL SCHEDULE

MARK	THICK	VERTICAL REINF.	HORIZONTAL REINF.	WALL CONSTRUCTION NOTES
MW-1	8"	(1) #5 @ 32" O.C.	(2) #4 @ 32" O.C.	RUNNING BOND

1. ALL MASONRY WALLS ARE MW-1, UNO.
2. TERMINATE ALL REINFORCING (BOTH VERTICAL AND HORIZONTAL) WITH A STANDARD HOOK, OR APPROVED EQUAL.
3. DO NOT SOLID GROUT WALLS UNLESS SPECIFICALLY NOTED ON PLANS.
4. SOLID GROUT ALL CELLS WITH REINFORCING AND/OR EMBEDS, TYPICAL.
5. LAY BLOCK IN RUNNING BOND, TYPICAL, UNO.
6. TOOL MORTAR JOINTS CONCAVE, TYPICAL, UNO.
7. PROVIDE MINIMUM COVER FOR REINFORCING PER THE GENERAL NOTES.
8. PROVIDE CORNER BARS AT CORNERS AND INTERSECTING WALLS.
9. DOWEL TO THE STRUCTURE BELOW WITH BARS MATCHING THE VERTICAL WALL REINFORCING, UNO.
10. PLACE VERTICAL REINFORCING IN THE CENTER OF THE WALL UNLESS EACH FACE IS SPECIFIED.
11. REMOVE BACK-TO-BACK FACE SHELLS AT INTERSECTING WALLS. ALTERNATE REMOVED FACE SHELLS, COURSE BY COURSE.
12. ADD LADDER-TYPE JOINT REINFORCING OF 2-#9 WIRES (3-#9 WIRES WITH VENEER) AT 16" O.C. HORIZONTALLY IN ALL MASONRY WALLS.

PLANTER BOX DECKS

1. SEE GENERAL NOTES: "STEEL DECK" FOR ADDITIONAL INFORMATION.
2. CONNECTIONS: WELD DECK TO SUPPORTING FRAMING MEMBERS W/ EITHER E60 or E70 ELECTRODES W/ PUDDLE WELDS HAVING A FUSION AREA NOT LESS THAN 3/4" EFFECTIVE DIAMETER AS FOLLOWS: (WELDS SHALL PENETRATE ALL LAYERS OF DECK AND HAVE PROPER FUSION TO THE SUPPORTING MEMBERS.)
   A. @ EACH FLUTE & TWO AT PANEL LAPS (ONE EACH SIDE OF SEAM).
   B. 6" O.C. @ ALL SUPPORTS PARALLEL TO DECK CORRUGATIONS.
   C. FASTEN SIDE LAPS BY ONE OF THE FOLLOWING:

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LICENSED CERTIFIED STRUCTURAL ENGINEER No.160916 KENNETH PAUL WILLMORE
5/18/09

DWS - PROVO ENTRANCE REPLACEMENT & LAND-SCAPING IMPROVEMENTS

Department of Workforce Services
1550 North Freedom Boulevard
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MARK	DATE	DESCRIPTION
	5/20/09	Add #3 Issue

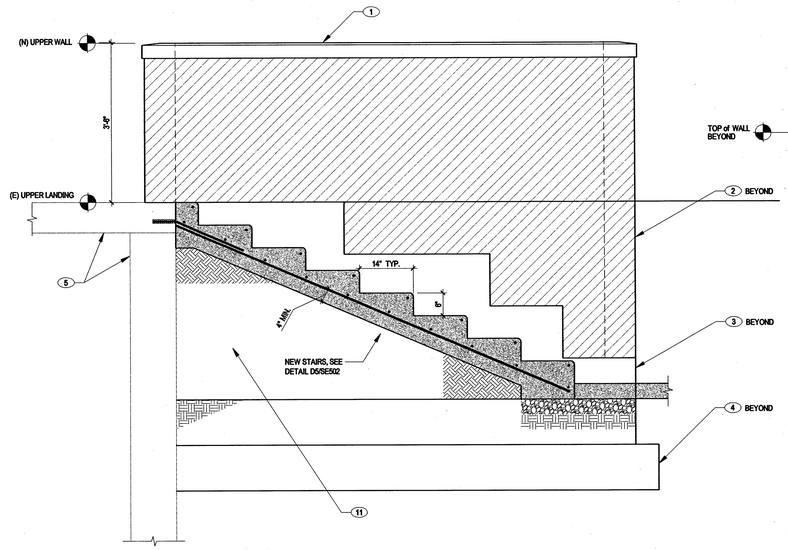
DATE:	29 APRIL 2009
DFCM PROJECT NO:	08071920
HFSA PROJECT NO:	0821.01
CAD DWG FILE NO:	WCA file # 09035
DRAWN BY:	
CHECKED BY:	DW
DESIGNED BY:	KPW
DWG TYPE:	ARCHITECTURAL
ARCHITECTURAL PHASE:	CONSTRUCTION BID DOCUMENTS

SHEET TITLE

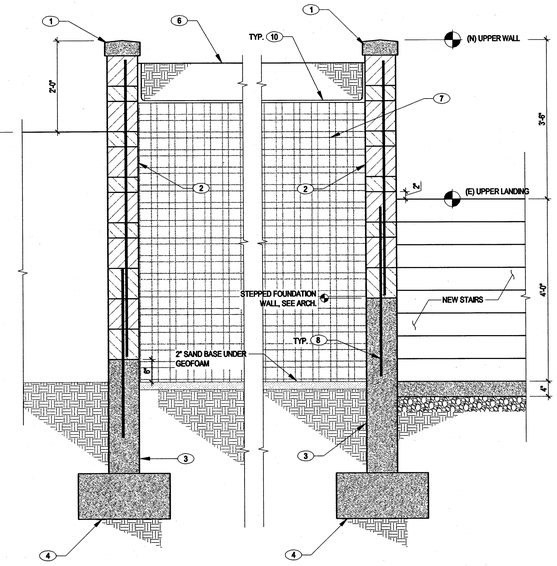
FOOTING AND FOUNDATION PLAN

SE201

SHEET OF XX



C2 STAIR SECTION
SE501 NO SCALE



C4 UPPER PLANTER BOX SECTION
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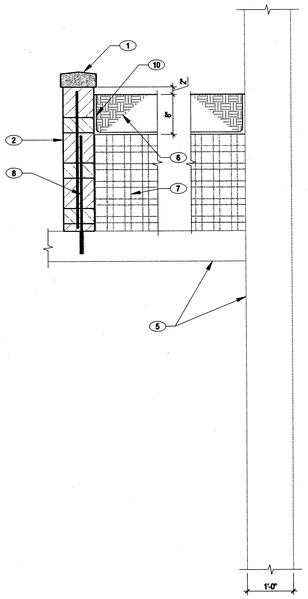
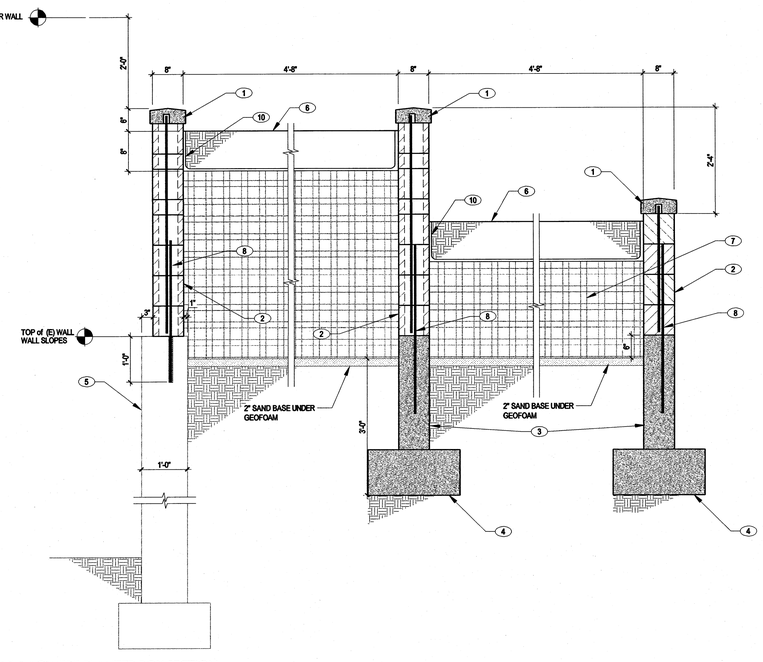


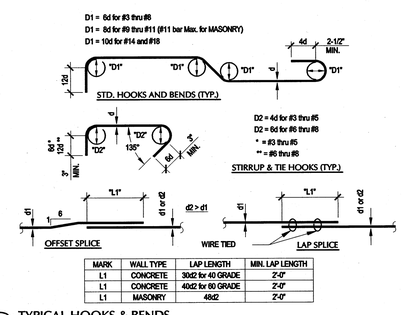
C5 UPPER PLANTER BOX SECTION
SE501 NO SCALE

KEYNOTES for SECTIONS (Not for Details) :

1. NEW PRE-CAST CONCRETE CAP 4x8. SEE ARCHITECT. EITHER DOWELED or GLUED TO WALL.
2. NEW 8" CMU WALL, SEE PLANS, GENERAL NOTES & MASONRY WALL SCHEDULE FOR REINFORCEMENT.
3. NEW 8" CONCRETE WALL, SEE PLANS, GENERAL NOTES AND CONCRETE WALL SCHEDULE & DETAIL A4/SE501.
4. NEW CONC. FOOTINGS, SEE PLANS AND FOOTING SCHEDULE
5. (E) CONC. STRUCTURE
6. 8" DEEP POTTING SOIL, SEE ARCHITECT.
7. NEW GEOFOAM, SEE PLAN AND GENERAL NOTES
8. NEW #5 DOWELS @ 32"O.C. & @ ALL CORNERS. 12" EPOXY EMBEDMENT INTO (E) CONCRETE WITH 24" LAP INTO NEW CMU. 24" LAP INTO NEW CONCRETE AND NEW CMU, TYP.
9. 6" CONC. SLAB-ON-GRADE OVER 6" COMPACTED BASE.
10. MEMBRANE LINER, SEE ARCHITECT.
11. ENGINEERED FILL

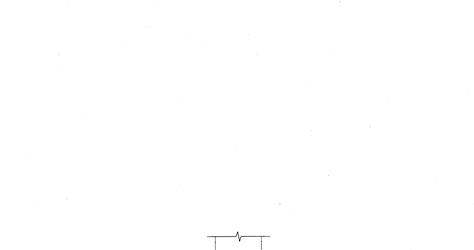


A2 MIDDLE & LOWERE PLANTER BOX SECTION
SE501 NO SCALE

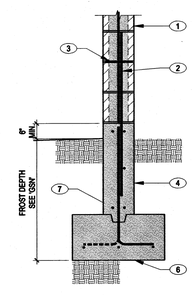


MARK	WALL TYPE	LAP LENGTH	MIN. LAP LENGTH
L1	CONCRETE	30d2 for 40 GRADE	2'-0"
L1	CONCRETE	40d2 for 60 GRADE	2'-0"
L1	MASONRY	48d2	2'-0"

B4 TYPICAL HOOKS & BENDS
SE501 NO SCALE



B5 CONCRETE OR MASONRY WALL CORNER REINF.
SE501 NO SCALE



KEYNOTES:

1. MASONRY WALL, SEE PLAN
2. DOWELS TO MATCH VERT. REINF. IN MASONRY WALL
3. LADDER-TYPE JOINT REINFORCING, MASORNY WALL SCHEDULE
4. CONCRETE WALL, SEE PLAN
5. (2) #5 x CONT. TOP & BOTTOM.
6. FOOTING, SEE PLAN AND SCHEDULE FOR SIZE AND REINF.

A4 TYPICAL FOOTING DETAIL
SE501 NO SCALE

EMBEDMENT OF ADHESIVE ANCHORS

BASE MATERIAL	REBAR DOWELS	THREADED ROD Ø	EMBEDMENT LENGTH	SCREEN LENGTH
CONCRETE	-	-	-	--
	#4	1/2"	8"	--
	#5	5/8"	12"	--
	#6	3/4"	12"	--
CMU (GROUTED)	-	-	-	--
	-	-	-	--
	#4	1/2"	-	--
	#5	5/8"	-	--
	#6	3/4"	-	--
CMU (HOLLOW)	-	--	--	-
	#4	1/2"	--	-
	#5	5/8"	--	-
	#6	3/4"	--	-

NOTES:
1. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS.
2. EMBEDMENT LENGTH IS INTO STRUCTURE AND NOT VENEER, UNO.
3. REBAR SHALL BE DEFORMED.
4. MINIMUM WALL THICKNESS TO BE EMBEDMENT LENGTH PLUS 1-1/2" OTHERWISE SEE STRUCTURAL ENGINEER.
5. URM WALL TO USE SCREENS AND TO BE EMBEDED IN MORTAR JOINT UNO.
6. USE HILTI HY150 EPOXY FOR BOLTS INTO UN-REINFORCED MASONRY FACE SHELLS
7. USE HILTI HY150 EPOXY FOR BOLTS INTO CONCRETE AND GROUT FILLED BLOCK.
8. ONLY USE CMU HOLLOW IF CALLED OUT SPECIFICALLY IN DETAIL.

A5 EMBEDMENT SCHEDULE
SE501 NO SCALE

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LICENSED CERTIFIED STRUCTURAL ENGINEER
No. 160916
KENNETH PAUL WILLMORE
5/20/09

DWS - PROVO ENTRANCE REPLACEMENT & LAND-SCAPING IMPROVEMENTS

Department of Workforce Services
1550 North Freedom Boulevard
Provo, Utah

MARK	DATE	DESCRIPTION
	5/20/09	Add #5 Issue.

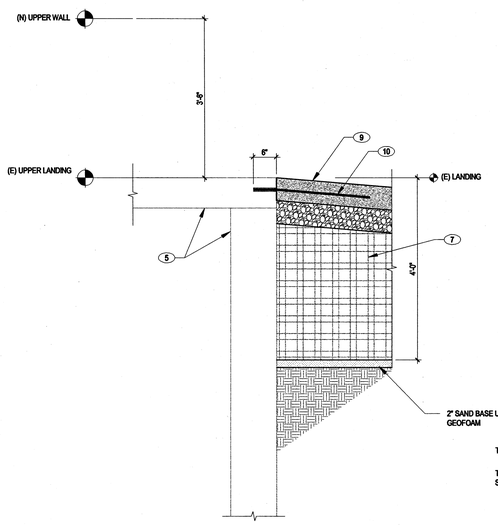
DATE:	29 APRIL 2009
DFCM PROJECT NO:	08071920
HFSA PROJECT NO:	0821.01
CAD DWG FILE NO:	WCA file # 09035
DRAWN BY:	
CHECKED BY:	DW
DESIGNED BY:	KPW
DWG TYPE:	ARCHITECTURAL
ARCHITECTURAL PHASE:	CONSTRUCTION BID DOCUMENTS

SHEET TITLE

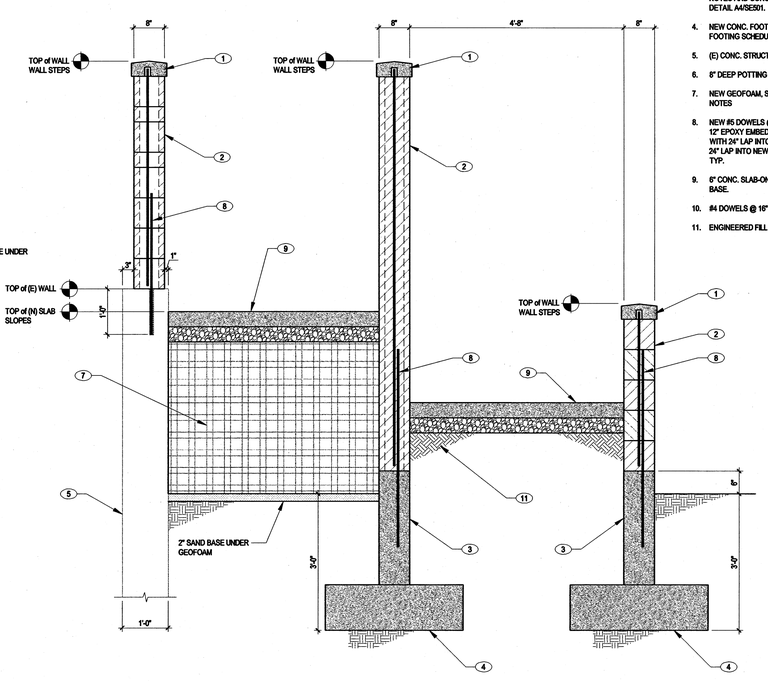
SECTIONS AND DETAILS

SE501

SHEET OF XX



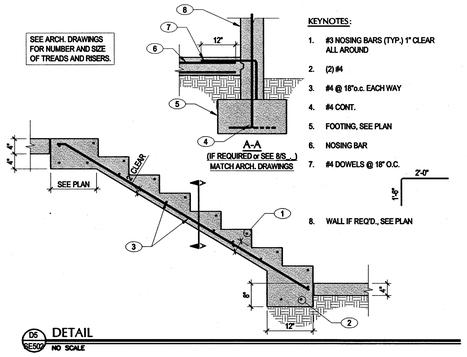
C2 RAMP - (E) LANDING SECTION
NO SCALE



B3 RAMP SECTION
NO SCALE

KEYNOTES for SECTIONS (Not for Details) :

1. NEW PRE-CAST CONCRETE CAP 4x8, SEE ARCHITECT.
2. NEW 8" CMU WALL, SEE PLANS, GENERAL NOTES & MASONRY WALL SCHEDULE FOR REINFORCEMENT.
3. NEW 8" CONCRETE WALL, SEE PLANS, GENERAL NOTES AND CONCRETE WALL SCHEDULE & DETAIL A4/SE501.
4. NEW CONC. FOOTINGS, SEE PLANS AND FOOTING SCHEDULE
5. (E) CONC. STRUCTURE
6. 8" DEEP POTTING SOIL, SEE ARCHITECT.
7. NEW GEOFOAM, SEE PLAN AND GENERAL NOTES
8. NEW #5 DOWELS @ 32"O.C. & @ ALL CORNERS. 12" EPOXY EMBEEDMENT INTO (E) CONCRETE WITH 24" LAP INTO NEW CMU. 24" LAP INTO NEW CONCRETE AND NEW CMU, TYP.
9. 6" CONC. SLAB-ON-GRADE OVER 6" COMPACTED BASE.
10. #4 DOWELS @ 16" O.C. (EPOXY COATED REBAR)
11. ENGINEERED FILL



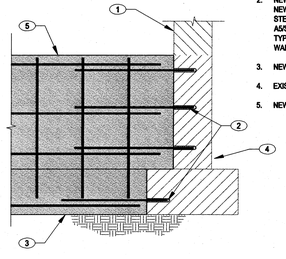
KEYNOTES:

1. #3 NOSING BARS (TYP.) 1" CLEAR ALL AROUND
2. (2) #4
3. #4 @ 18"o.c. EACH WAY
4. #4 CONT.
5. FOOTING, SEE PLAN
6. NOSING BAR
7. #4 DOWELS @ 18" O.C.
8. WALL IF REQ'D., SEE PLAN

D5 DETAIL
NO SCALE

C5 NOT USED
NO SCALE

B5 NOT USED
NO SCALE



KEYNOTES:

1. EXISTING WALL
2. NEW 24" HORIZ DOWELS, TO MATCH NEW FOUNDATION AND STEM WALL STEEL. DRILL & EPOXY PER DETAIL A5/SE501 INTO EXISTING WALLS. TYPICAL for both CONCRETE & CMU WALLS.
3. NEW FOOTING, SEE PLAN
4. EXISTING FOUNDATION
5. NEW WALLS, SEE PLAN

A5 DETAIL
NO SCALE

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5/18/09

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SECTIONS AND DETAILS

SE502

SHEET OF XX